PLANNING & ZONING COMMISSION

ROBERT C. WHEATLEY, CHAIRMAN KIM HOEY STEVENSON, VICE-CHAIRMAN R. KELLER HOPKINS J. BRUCE MEARS HOLLY J. WINGATE





DELAWARE sussexcountyde.gov 302-855-7878 T 302-854-5079 F JANELLE CORNWELL, AICP DIRECTOR

PLANNING AND ZONING AND COUNTY COUNCIL INFORMATION SHEET Planning Commission Public Hearing Date February 13, 2020.

Application: (CU 2211) Indian River School District

Applicant: Indian River School District C/O Joseph Booth

31 West Hosier Street Selbyville, DE 19975

Owner: Indian River School District

31 West Hosier Street Selbyville, DE 19975

Site Location: Located on the west side of Patriots Way (S.C.R. 318) approximately

0.74 miles south of Zoar Road (S.C.R. 48).

Current Zoning: Agricultural Residential (AR-1)

Proposed Use: Special Needs School

Comprehensive Land

Use Plan Reference: Low Density Area

Councilmatic

District: Mr. Wilson

School District: Indian River School District

Fire District: Millsboro Fire District

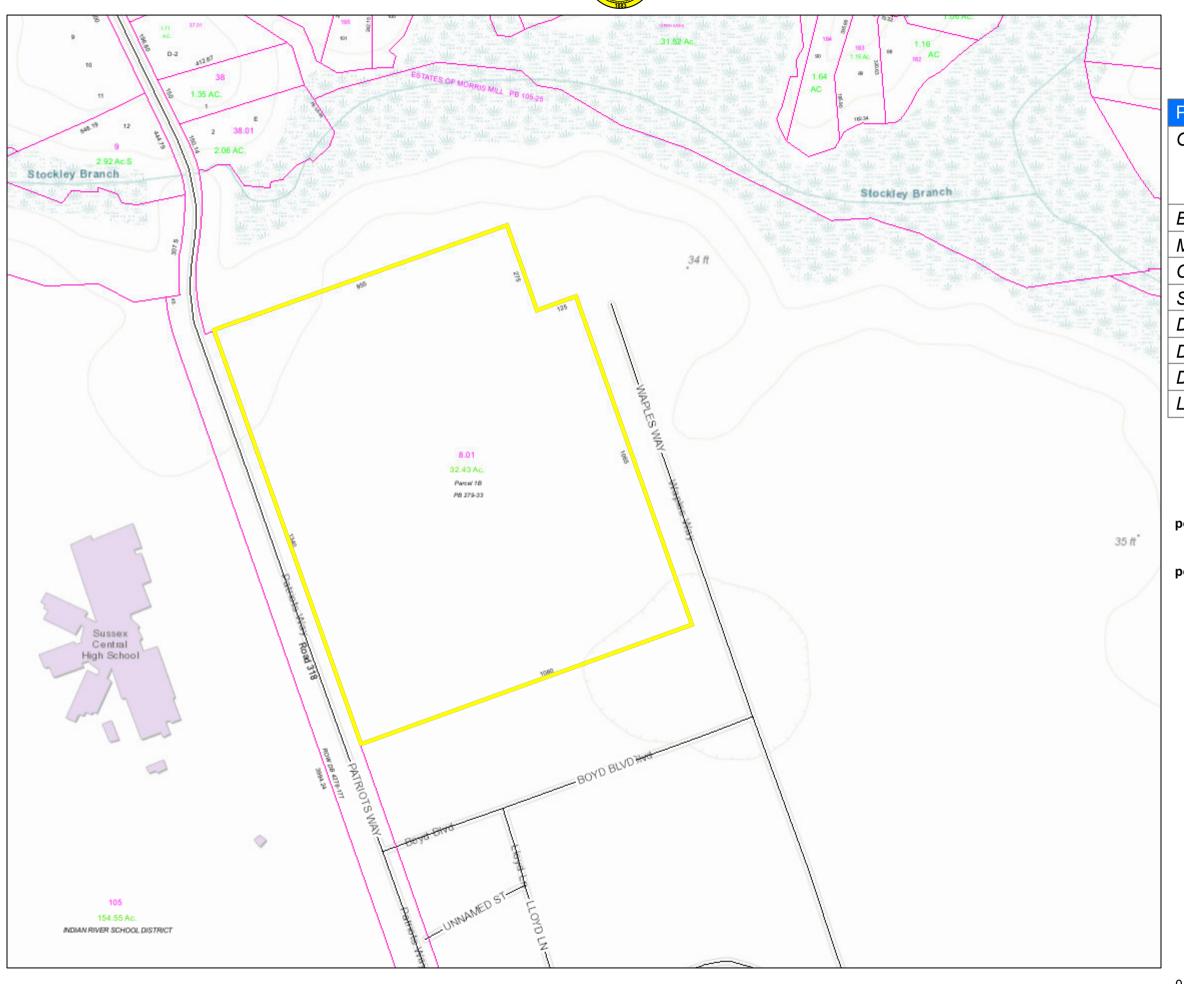
Sewer: Town of Georgetown

Water: Private, On-Site

Site Area: 32.43 acres +/-

Tax Map ID.: 133-7.00-8.01





PIN:	133-7.00-8.01
Owner Name	INDIAN RIVER SCHOOL DISTRICT
Book	5076
Mailing Address	31 W HOSIER ST
City	SELBYVILLE
State	DE
Description	188000
Description 2	RT 318
Description 3	RT 86
Land Code	

polygonLayer

Override 1

polygonLayer

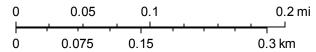
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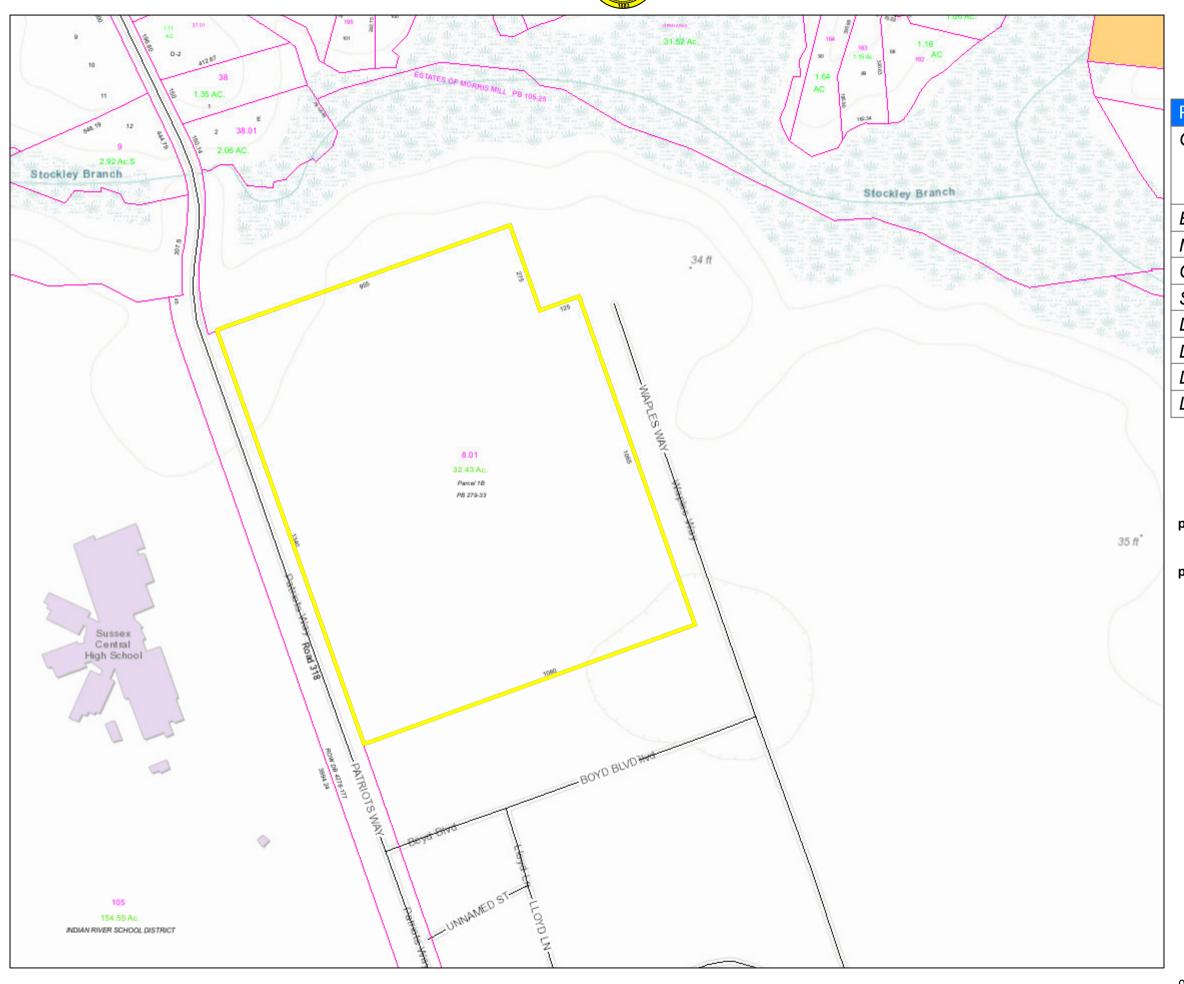
Tax Parcels

Streets

County Boundaries

1:4,514





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polygonLayer

Override 1

polygonLayer

Override 1

Tax Parcels

Streets

0

0.05 0.1 0.2 mi 0.075 0.15 0.3 km

1:4,514



JAMIE WHITEHOUSE, AICP
ACTING DIRECTOR OF PLANNING &
ZONING
(302) 855-7878 T
(302) 854-5079 F
jamie.whitehouse@sussexcountyde.gov



Sussex County

DELAWARE sussexcountyde.gov

Memorandum

To: Sussex County Planning Commission Members

From: Jennifer Norwood, Planner I

CC: Vince Robertson, Assistant County Attorney and applicant

Date: February 5, 2020

RE: Staff Analysis for CU 2211 Indian River School District

This memo is to provide background and analysis for the Planning Commission to consider as a part of application CU 2211 Indian River School District to be reviewed during the February 13, 2020 Planning Commission Meeting. This analysis should be included in the record of this application and is subject to comments and information that may be presented during the public hearing.

The request is for a Conditional Use for parcel 133-7.00-8.01 to allow for a special needs school. The size of the property is 32.43 ac. +/-. The property is zoned AR-1 (Agricultural Residential Zoning District) and located west of Patriot's Way approximately 0.73 mile south of Zoar Rd.

The 2018 Sussex County Comprehensive Plan Update (Comprehensive Plan) provides a framework for how land is to be developed. As part of the Comprehensive Plan a Future Land Use Map is included to help determine how land should be zoned to ensure responsible development. The Future Land Use map in the plan indicates that the property has the land use designation of Low-Density Area.

The surrounding parcels to the north, east, and south are all designated on the Future Land Use Map as "Low Density Area". The properties to the west have the land use designation of "Commercial Area", "Developing Area", and "Municipalities". The Low-Density Area land use designation recognizes are agricultural activities and homes. Business development should be largely confined to businesses addressing the needs of these two uses. Institutional and commercial uses may be appropriate depending on surrounding uses. The Developing Area land use designation recognizes a range of single-family homes, townhouses, and multi-family units. Careful mixtures of homes with light commercial and institutional uses can be appropriate to provide convenient services and to allow people to work close to home.

The property is zoned AR-1 (Agricultural Residential Zoning District). The adjoining and surrounding properties to the north, south, east and west are all zoned AR-1 (Agricultural Residential Zoning District).

There was a Conditional Use 1408 for a public school on a parcel zoned AR-1 (Agricultural Residential District), which was approved on July 24, 2001 to the west of the application site.

Based on the analysis of the land use, surrounding zoning and uses, the Conditional Use to allow for



a special needs school could be considered consistent with the land use, area zoning and surrounding uses.

Staff notes the conceptual site plan shows a proposed 94,500 square foot building, however, the Service Level Evaluation Response received is based on a 157,000 square foot special needs school. The increase in square footage is due to the potential future building additions that are shown on the site plan. For the purpose of the Traffic Operational Analysis, the maximum square footage possible is being used.

File #: (11821) 201913693

Planning & Zoning Commission Application Sussex County, Delaware

Sussex County Planning & Zoning Department

2 The Circle (P.O. Box 417) Georgetown, DE 19947

302-855-7878 ph. 302-854-5079 fax

Type of Application: (please che	ck applicable)					
Conditional Use <u>√</u>						
Zoning Map Amendment						
Site Address of Conditional Use/Zoning Map Amendment						
26351 Patriots Way Georgetown, Dela	ware 19947					
Type of Conditional Use Reques	ted:					
The permittance of a Public Special Ne		rict.				
Tax Map #: 133-7.00-8.01		Size of Parcel(s): 32.43± acres				
Current Zoning: AR-1 Pro	posed Zoning: AR-1	Size of Building: 149,661± SF				
Land Use Classification: Institution	al	- W				
Water Provider: On site well	Sew	er Provider: Town of Georgetown				
Applicant Information						
Applicant Name: Indian River School	ol District C/O Joseph Booth					
Applicant Address: 31 West Hosier						
City: Selbyville	State: <u>DE</u>	•				
Phone #: (302) 436-1000	E-mail: josep	h.booth@irsd.k12.de.us				
Owner Information						
Owner Name: Indian River School E	District C/O Joseph Booth					
Owner Address: 31 West Hosier Stre	et					
City: Selbyville	State: <u>DE</u>	Zip Code: <u>19975</u>				
Phone #: (302) 436-1000	E-mail: <u>"</u>					
Agent/Attorney/Engineer Inform	nation					
Agent/Attorney/Engineer Name:	CDA Engineering, Inc - C	Colmcille DeAscanis				
Agent/Attorney/Engineer Address		01				
City: Wilmington	State: <u>DE</u>	· · · · · · · · · · · · · · · · · · ·				
Phone #: (302) 998-9202	F_mail- cdeas	scanis@cdaengineering.com				





Check List for Sussex County Planning & Zoning Applications

The following shall be submitted with the application

✓ co	mpleted Application
<u>√</u> Pr	 o Survey shall show the location of existing or proposed building(s), building setbacks, parking area, proposed entrance location, etc. o Provide a PDF of Plans (may be e-mailed to a staff member) o Deed or Legal description
✓ Pro	ovide Fee \$500.00
ari	otional - Additional information for the Commission/Council to consider (ex. chitectural elevations, photos, exhibit books, etc.) If provided submit 8 copies and they all be submitted a minimum of ten (10) days prior to the Planning Commission meeting.
su	ease be aware that Public Notice will be sent to property owners within 200 feet of the bject site and County staff will come out to the subject site, take photos and place a sign the site stating the date and time of the Public Hearings for the application.
<u></u> ✓ De	IDOT Service Level Evaluation Request Response
PL	US Response Letter (if required)
	ed hereby certifies that the forms, exhibits, and statements contained in any papers or d as a part of this application are true and correct.
Zoning Commi and that I will a	nat I or an agent on by behalf shall attend all public hearing before the Planning and ssion and the Sussex County Council and any other hearing necessary for this application answer any questions to the best of my ability to respond to the present and future alth, safety, morals, convenience, order, prosperity, and general welfare of the inhabitants aty, Delaware.
Signature of	Applicant/Agent/Attorney
1	Mellacain Date: 11/22/19
Signature of	Owner Indian River Date: 11-25-19
For office use or Date Submitted Staff accepting a Location of prop	### Fee: \$500.00 Check #: 50 40 Application: Cel
	ing: Recommendation of PC Commission: ing: Decision of CC:



STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

February 7, 2020

Mr. Jamie Whitehouse, Acting Director Sussex County Planning & Zoning P.O. Box 417 Georgetown, DE 19947

Dear Mr. Whitehouse:

The Department has completed its review of a Service Level Evaluation Request for the **Indian River School District** conditional use application, which we received on February 6, 2020. This application is for an approximately 32.43-acre parcel (Tax Parcel: 133-7.00-8.01). The subject land is located on the east side of Patriots Way (Sussex Road 318), directly opposite the Sussex Central High School, south of Georgetown. The subject land is currently zoned AR-1 (Agricultural Residential), and the applicant is seeking a conditional use approval to develop a 157,000 square-foot special needs school.

Per the 2018 Delaware Vehicle Volume Summary, the annual average daily traffic volume along the segment of Patriots Way where the subject land is located is 2,913 vehicles per day.

Based on our review, we estimate that the proposed land use will generate more than 50 vehicle trips per a weekly peak hour or 500 vehicle trips per day and would be considered to have a Minor impact to the local area roadways. In this instance, the Department considers a Minor impact to be when a proposed land use would generate more than either 50 vehicle trips per a weekly peak hour and / or 500 vehicle trips per day but fewer than 200 vehicle trips per a weekly peak hour and 2,000 vehicle trips per day. Because of this impact, we recommend that the applicant be required to perform a Traffic Impact Study (TIS) for the subject application. However, our <u>Development Coordination Manual</u> provides that where a TIS is required only because the volume warrants are met, and the projected trip generation will be fewer than 200 vehicle trips per a weekly peak hour and fewer than 2,000 vehicle trips per day, DelDOT may permit the developer to pay an Area-Wide Study Fee of \$10 per daily trip in lieu of doing a TIS. For this application, if the County were agreeable, we would permit the developer to pay an Area-wide Study Fee.

Based on our review of site-specific data provided by the applicant, the proposed 157,000 square-foot special needs school would generate 420 vehicle trips per day, 199



Mr. Jamie Whitehouse Page 2 of 2 February 7, 2020

vehicle trips during the morning peak hour, and 158 vehicle trips during the evening peak hour. As stated above, because this development would generate fewer than 2,000 vehicle trips per day and fewer than 200 vehicle trips during a weekly peak hour, the applicant has the option to pay the Area-Wide Study Fee in lieu of doing a TIS. The Area-Wide Study Fee for the proposed development would be \$4,200.00. Payment of the Area-Wide Study Fee does not preclude a developer from having to make or participate in off-site improvements including a Traffic Operational Analysis (TOA) if one is found to be necessary.

On September 12, 2019, the applicant's consultant met with DelDOT to conduct a TOA scoping meeting for the subject development. In our TOA scope of work memorandum, issued October 14, 2019, DelDOT identified the facilities where analysis would be required. A copy of the scope of work memorandum is enclosed with this letter.

If the County approves this application, the applicant should be reminded that DelDOT requires compliance with State regulations regarding plan approvals and entrance permits, whether or not a TIS is required.

Please contact Mr. Claudy Joinville, at (302) 760-2124, if you have questions concerning this correspondence.

Sincerely,

T. William Brockenbrough, Jr.

County Coordinator

Development Coordination

TWB:cjm Enclosure

cc: Constance C. Holland, Coordinator, Cabinet Committee on State Planning Issues
 Indian River School District, Applicant
 J. Marc Coté, Assistant Director, Development Coordination
 Gemez Norwood, South District Public Works Manager, Maintenance and Operations
 Susanne Laws, Sussex County Subdivision Coordinator, Development Coordination
 Derek Sapp, Subdivision Manager, Development Coordination
 Kevin Hickman, Subdivision Manager, Development Coordination
 Brian Yates, Subdivision Manager, Development Coordination
 John Andrescavage, Subdivision Manager, Development Coordination
 James Argo, South District Project Reviewer, Maintenance & Operations
 Troy Brestel, Project Engineer, Development Coordination

Claudy Joinville, Project Engineer, Development Coordination



STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

MEMORANDUM

TO:

File

FROM:

Claudy Joinville, Project Engineer

C.J.

DATE:

October 14, 2019

SUBJECT:

Howard T. Ennis School

Traffic Operational Analysis (TOA) - Scoping Meeting (9/12/19)

Scope of Work

ATTENDANCE:

Janelle Cornwell, Sussex County Planning and Zoning (via skype)

Colmcille DeAscanis, CDA Engineering, Inc. (via skype)

Zak Kerstetter, CDA Engineering, Inc.

Eric Ostimchuk, Traffic Planning & Design (TPD)

Ken Fearn, Fearn Clendaniel Architects Peter Haag, DelDOT Traffic (via skype)

Marc Coté, DelDOT Planning Susanne Laws, DelDOT Planning John Andrescavage, DelDOT Planning

T. William Brockenbrough, Jr., DelDOT Planning

Claudy Joinville, DelDOT Planning

Background and Discussion

Indian River School District seeks to develop a 157,000 square-foot Special Needs school on an approximately 32.43-acre parcel (Tax Parcel: 133-7.00-8.01). The land is located on the east side of Patriots Way (Sussex Road 318), directly opposite the Sussex Central High School, south of Georgetown. The land is currently zoned as AR-1 (Agricultural Residential), and the developer does not plan to rezone the land.

One full access point is proposed along Patriots Way. Construction is anticipated to be complete in 2022.



Memorandum to File October 14, 2019 Page 2 of 5

Cases to be Evaluated

The study shall evaluate the weekday morning and weekday evening peak hours for the following situations:

- 1) Existing (2019);
- 2) 2022 without development; and
- 3) 2022 with development.

Facilities to be Evaluated

The TOA should evaluate conditions at the following intersections for capacity and level of service using the Highway Capacity Software (HCS). It should also evaluate the extent to which they meet the relevant DelDOT, AASHTO and MUTCD standards for geometry and traffic control devices.

- 1) Patriots Way (Sussex Road 318) / Site Entrance / Sussex Central High School Entrance (center)
- 2) Patriots Way / Sussex Central High School Entrance (north)
- 3) Patriots Way / Sussex Central High School Entrance (south)

Traffic Counts

The Consultant should conduct traffic counts for the intersections listed above from 7:00 a.m. to 9:00 a.m. and from 2:00 p.m. to 4:00 p.m., on a Tuesday, Wednesday or Thursday to determine when the peaks occur. The weekday traffic counts should be performed during a time when schools are open and operating at a normal capacity.

Additionally, an Automatic Traffic Recorder (ATR) should be used to collect traffic data on Patriots Way south of the south high school entrance. The ATR should be placed for a one-week time period that includes the date(s) of the manual traffic counts. The ATR data will be used to verify the manual counts and determine whether adjustments are required.

Section 2.2.8.5, item 19, under Existing Traffic and Transportation Conditions in the <u>Development Coordination Manual</u>, addresses how oversaturated intersections are to be counted.

The traffic counts should be submitted to DelDOT both electronically as Portable Document Format (PDF)/Excel files and as draft report figures showing peak hour volumes (<u>labeled with date and peak hour interval</u>) posted on diagrams of the road network.

The Consultant should include counts of pedestrians, a separate count of right-turn on red (in addition to right-turn movement counts), and a separate count of heavy vehicles.

Memorandum to File October 14, 2019 Page 3 of 5

The Consultant should be alert for events affecting the traffic counts, such as accidents or nearby construction and shall make note of any such events when submitting the counts. As necessary, DelDOT reserves the right to reject the counts or require adjustments to them.

Trip Generation

DelDOT is agreeable to the Consultant's use of historical and site specific data to calculate the trips for the proposed school.

Trip Distribution

A trip distribution to be used for the site were developed using DelDOT's Travel Demand Model and is attached. School bus and parent traffic should reflect the school's feeder patterns as the model distribution only applies to staff traffic.

Future Growth

The Consultant shall apply growth factors to the traffic counts. DelDOT will develop those factors after we receive the Consultant's traffic counts.

Pedestrian Traffic Analysis

The Consultant shall complete a pedestrian crossing analysis using NCHRP 562. The analysis should evaluate multiple crossing scenarios, such as, but not limited to, a single crossing and a two-stage crossing with a median refuge island. In addition, the analysis should assume the pedestrian volume has met the NCHRP threshold. For more guidance on this analysis, the Consultant shall contact Peter Haag, Traffic Studies Manager of DelDOT's Traffic Section. Mr. Haag may be reached at (302) 659-4084.

Highway Capacity Software & Synchro

The Consultant shall use the most recent version of the Highway Capacity Software (HCS) that implements the 6^{th} Edition of the <u>Highway Capacity Manual</u> (HCM). Presently, that is HCS7.

In addition, the Consultant shall use Synchro to conduct 15-minute analysis periods to accurately model the peak traffic volumes associated with the Sussex Central High School opposite the proposed school.

Memorandum to File October 14, 2019 Page 4 of 5

Seasonal Adjustment Factors for the roads in the study area are as follows:

Roads	September	October	November
Patriots Way (Sussex Road 318)	0.96	0.95	0.98
All Other Roads	1.00	1.00	1.00

DelDOT Projects

Currently, DelDOT has no active projects within the study area.

Transit, Bicycle, and Pedestrian Facilities

The study should describe the existing and proposed transit service in the project area and should also describe the existing and needed transit, bicycle, and pedestrian facilities on or near the project site. In determining these items, the Consultant shall contact Mr. David Dooley, a Service Development Planner at the Delaware Transit Corporation (DTC), and Mr. Anthony Aglio, of DelDOT's Local Systems Section. Mr. Dooley may be reached at (302) 576-6064. Mr. Aglio may be reached at (302) 760-2509.

General Notes

- 1) All submissions relating to this study should be made electronically via the Planning and Development Coordination Application (PDCA), preferably in Portable Document Format (PDF).
- 2) The Consultant should e-mail DelDOT's Transportation Management Center (TMC) at tmc1@delaware.gov to obtain advance approval for the use of any signal timings.
- Before deploying temporary unmanned devices, e.g. cameras or radar detectors, in the State-maintained right-of-way, the individual or company proposing to do so shall execute and file a Right-of-Way Use Agreement. Before each specific deployment of devices, the individual or company shall email a completed Temporary Data Collection Device Notification Form to TMC1@delaware.gov. Deployment of Automatic Traffic Recorders, a.k.a. tube counters, and devices on portable trailers does not require a Right-of-Way Use Agreement but does require submission of the Temporary Data Collection Device Notification Form. Copies of the standard agreement and the form are available from Ms. Lara Brown at (302) 659-4062 or Lara.Brown@delaware.gov.
- 4) The Consultant should refer to the attached memorandum from Scott Neidert of DelDOT's Traffic Section for guidance regarding requests for crash data within the study area. The Consultant shall report on this data and make recommendations for improvements if safety problems exist in the study area. Mr. Neidert may be reached at (302) 659-4075.

- 5) Both DelDOT and Sussex County reserve the right to change this scope of work if the study is not performed within a reasonable time.
- 6) The developer may choose to have DelDOT's Consultant perform the TOA rather than use their own Consultant. If this option is of interest, the developer should contact Mr. Troy Brestel at (302) 760-2167 to request a cost estimate.
- 7) By copy of this memorandum I ask those copied to contact me at (302) 760-2124 regarding any significant errors or omissions.

CJ:cim

Enclosure

cc: Drew Boyce, Director, Planning

Joseph Booth, Indian River School District

Michael Simmons, Assistant Director for Project Development South, DOTS

Alastair Probert, South District Engineer, DOTS

Gemez Norwood, South District Public Works Supervisor, DOTS

William Kirsch, South District Permit Supervisor, DOTS

Mark Whiteside, Project Manager, Project Development – South, DOTS

Scott Neidert, Design Resource Engineer, Traffic Section

Mark Buckalew, Traffic Safety Engineer, DelDOT Traffic

David Dooley, Service Development Planner, Delaware Transit Corporation

Anthony Aglio, Planning Supervisor, Local Systems

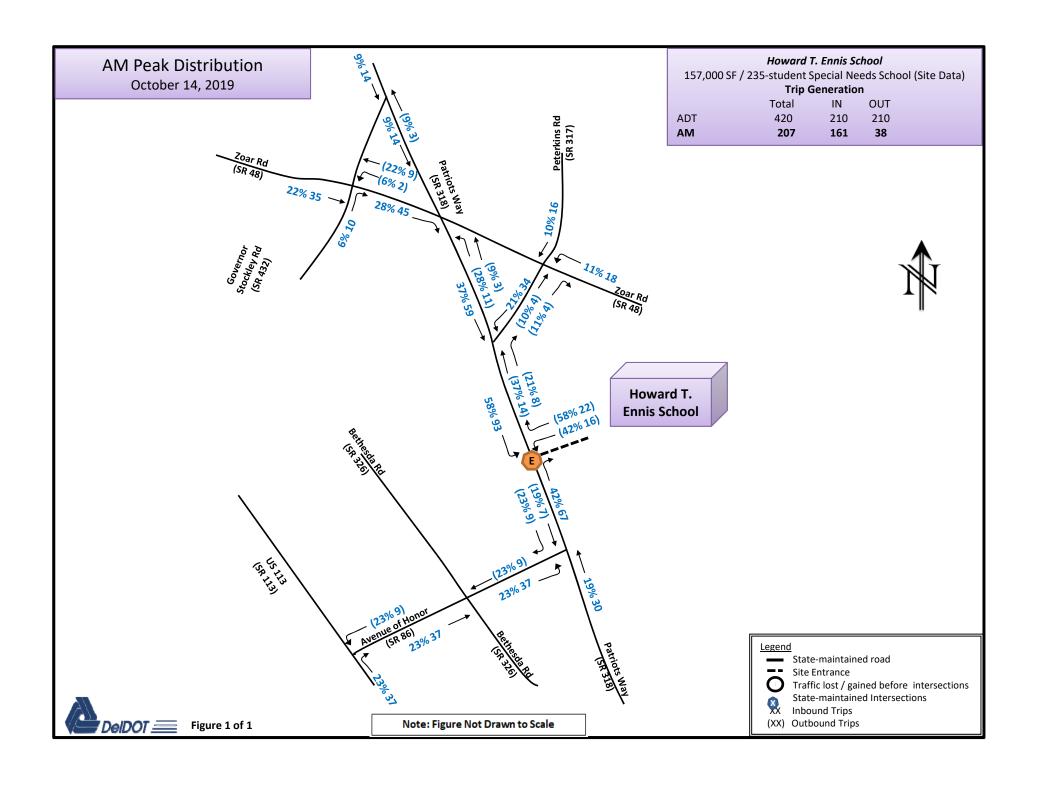
Lara Brown, Administrative Specialist, DelDOT Traffic DOTS

Kari Glanden, Statistical Information Supervisor, DelDOT Traffic, DOTS

Mark Galipo, Traffic Engineer, DelDOT Traffic, DOTS

Andrew Parker, McCormick & Taylor, Inc.

Mir Wahed, Johnson, Mirmiran, & Thompson, Inc.





STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

TO:

Requestors of Crash Data via DelDOT's Development Coordination Process

FROM:

Scott Neidert, Design Resource Engineer, Traffic Section

DATE:

September 5, 2019

SUBJECT: Revisions to Crash Data Requests and Releases

As of July 23, 2019, Governor Carney has signed SB 147 into law containing amendments to the release of crash data, namely permitting DelDOT to release certain de-identified data based on the nature of the requestor. Specifically, newly enacted 21 *Del. C.* §313(c)(1) provides that:

"The Department of Transportation may provide the information under this subsection if the person requesting the information provides <u>proof of identity and a sworn representation</u> that the data will be strictly used for any of the following purposes:

a. To comply with federal, State, or local law or regulations.

b. By a municipality or municipal planning organization in carrying out official functions."

To conform with the "proof of identity and... sworn representation..." clause, requestors will be required to complete an online crash data request as well as provide a <u>notarized</u> release form to be submitted with <u>each</u> crash request prior to being processed. A link to the release form is provided within the online crash request.

Effective immediately, all requests for crash data, when required, must be made at: https://tmc.deldot.gov/tmcx/app/crashdata/public/info.html

Requests for crash data will not be processed until all required fields are completed, and the release form has been completed and received.

SN

cc:

Nicole Majeski, Deputy Secretary

Shanté Hastings, Chief Engineer

Drew Boyce, Director, Planning

Annie Cordo, Deputy Attorney General

Mark Luszcz, Deputy Director, Division of Transportation Solutions

Kari Glanden Thompson, Statistical Information Supervisor, Traffic Section







Howard T. Ennis School

Traffic Operational Analysis

Georgetown, Sussex County, Delaware

For Submission To: DelDOT

HOWARD T. ENNIS SCHOOL DEVELOPMENT TRAFFIC OPERATIONAL ANALYSIS

Georgetown, Sussex County, Delaware

Prepared For:

CDA Engineering, Inc. 6 Larch Avenue, Suite 401

Wilmington, DE 19804

January 17, 2020

TPD # CDAE.00008



Prepared By:

Traffic Planning and Design, Inc.

1025 Andrew Drive, Suite 110 West Chester, PA 19380

Phone: (610) 326-3100 Fax: (610) 326-9410

E-mail: TPD@TrafficPD.com Website: www.trafficpd.com

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 Appendix B: Study Area Photographs
 Appendix C: Manual Traffic Count Printouts
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 Appendix E: Capacity Analyses
 Appendix F: Auxiliary Turn Lane Warrant Analyses
 Appendix G: Pedestrian Crossing Worksheets

EXECUTIVE SUMMARY

The purpose of this study is to examine the potential traffic impact associated with the proposed Howard T. Ennis School on the roadway network in Georgetown, Sussex County, Delaware. Based on this evaluation, the following conclusions were reached:

- 1. The project scope and the extent of the study area were based on the Scoping Memo issued by DelDOT on 10/14/2019. The study area intersections included in this TIS are as follows:
 - Patriots Way (Sussex Road 318) and Sussex Central High School Entrance (north)
 - » Patriots Way (Sussex Road 318) and Sussex Central High School Entrance (center)
 - » Patriots Way (Sussex Road 318) and Sussex Central High School Entrance (south)
- 2. The project site is located on the east side of Patriots way, north of Boyd Boulevard and opposite the existing Sussex Central High School. The proposed site will consist of a 157,000 square foot (s.f.) special needs school;
- 3. Access to the site is proposed via one full-access driveway to Patriots Way, opposite the existing Sussex Central School Driveway (center).
- 4. All proposed driveway location sight distances will exceed AASHTO's Safe Stopping Sight Distance (SSSD) criteria.
- 5. Upon full build-out, the proposed development is expected to generate 199 new vehicle-trips during the weekday A.M. peak hour, and 158 new vehicle-trips during the weekday P.M. peak hour.
- 6. Under the 2022 projected conditions, with implementation of the site-related recommendations, all approaches and turning movements at the site driveway intersection with the external roadway network will operate at LOS C or better during weekday A.M. and P.M. peak hours.
- 7. All overall intersection levels of service (ILOS) will operate at an ILOS A during the 2022 projected condition scenarios.
- 8. Traffic Planning and Design Inc. (TPD) recommends the following roadway improvements as outlined at the study area intersections:

Patriots Way & Proposed Site Driveway

- Provide a stop sign to control exiting traffic
- > Construct a 225-foot southbound left turn lane within the existing painted median
- » Consider installation of a painted crosswalk between the proposed Howard T. Ennis School driveway and the existing Sussex Central High School driveway (center)
- Levels of Service (LOS) for the study area intersections have been summarized in matrix form. Table I
 details the overall intersection LOS for each study area intersection.

TABLE I
OVERALL INTERSECTION LEVEL OF SERVICE SUMMARY

	Weekday A.M. Peak Hour			Weekday P.M. Peak Hour		
Intersection	2023 Coi		nditions	Entretion of	2023 Conditions	
	Existing	Base	Projected	Existing	Base	Projected
Patriots Way & Sussex Central High School Driveway (north)	A (7.4)	A (7.6)	A (7.4)	A (8.6)	A (8.7)	A (8.2)
Patriots Way & Sussex Central High School Driveway (center)	A (1.3)	A (1.2)	A (3.1)	A (0.4)	A (0.3)	A (5.2)
Patriots Way & Sussex Central High School Driveway (south)	A (5.5)	A (5.5)	A (4.8)	A (8.1)	A (8.1)	A (7.9)

Base = No-Build scenario Projected = Build scenario

Page ії — www.TrafficPD.com

INTRODUCTION

Traffic Planning and Design, Inc. (TPD) has completed a Transportation Impact Study (TIS) for the proposed Howard T. Ennis School in Georgetown, Sussex County, Delaware. As shown in **Figure 1**, the proposed development is located on the east side of Patriots way, north of Boyd Boulevard and opposite the existing Sussex Central High School. As shown in **Figure 2**, the proposed site will consist of a 157,000 square foot (s.f.) special needs school.

The project scope and the extent of the study area were confirmed with representatives of DelDOT through the scoping process. All relevant correspondence pertaining to this project has been included in **Appendix A**.

Site Access Locations

The proposed site will be served by one full-access driveway to Patriots Way, opposite the existing Sussex Central School Center Driveway

EXISTING ROADWAY NETWORK

A field review of the existing roadway system in the study area was conducted. The existing roadway characteristics within the study area are summarized in **Table 1**. Photographs of the study area intersections are included in **Appendix B**.

TABLE 1
ROADWAY CHARACTERISTICS WITHIN STUDY AREA

Roadway	Ownership	Functional Classification/ Roadway Type	Predominant Directional Orientation	Average Daily Traffic	Posted Speed Limit
Patriots Way	County (Road 318)	Local	North-South	2,913	25/35 mph ¹

¹School zone speed limit = 20 mph

Transit, Bicycle and Pedestrian Facilities

There is no mass transit provided in Georgetown, Sussex County, Delaware. Based on observations during field visits at the study area intersections, there are no pedestrian accommodations present in the vicinity of the proposed development. A bike lane is provided southbound on Patriots Way. A paved shoulder on the east side of Patriots Way accommodates pedestrian and bicycle activity traveling north.

TPD reached out to Mr. David Dooley and Mr. Anthony Aglio regarding existing, proposed and needed transit services, bicycle and pedestrian facilities in the area but has not received a response.

Crash Data Investigation

Crash data were obtained from DelDOT for the study area intersections for the three-year time period beginning 12/20/2016 and ending 12/20/2019. The number of crashes at the study area intersections is shown in **Table 2**.

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TABLE 2
DELDOT CRASH DATA

Church Auga Internaction	Number of Reportable Crashes			
Study Area Intersection	2017	2018	2019	
Patriots Way & Central High School Driveway (north)	1	2	1	
Patriots Way & Central High School Driveway (center)	1	0	0	
Patriots Way & Central High School Driveway (south)	0	0	0	

EXISTING TRAFFIC CONDITIONS

Manual Turning Movement Counts

Manual traffic counts were conducted on 15-minute intervals during the weekday morning (7:00 to 9:00 A.M.) and weekday afternoon (2:00 to 4:00 P.M.) peak periods. Data pertaining to heavy vehicles, and pedestrians were observed during the manual counts. Peak hours and count dates for the study area intersections are identified in **Table 3**.

TABLE 3
MANUAL TRAFFIC COUNT INFORMATION

Intersection ¹	Date of Traffic Counts	Time Period	Peak Hour ²
Patriots Way & Sussex Central High	Thursday November 7, 2019	Weekday A.M.	7:00 to 8:00 A.M.
School Driveway (north)	Thursday November 7, 2019	Weekday P.M.	2:30 to 3:30 P.M.
Patriots Way & Sussex Central High	Thursday November 7, 2019	Weekday A.M.	7:00 to 8:00 A.M.
School Driveway (center)	Thursday November 7, 2019	Weekday P.M.	2:30 to 3:30 P.M.
Patriots Way & Sussex Central High	Thursday Navambar 7, 2010	Weekday A.M.	7:00 to 8:00 A.M.
School Driveway (south)	Thursday November 7, 2019	Weekday P.M.	2:30 to 3:30 P.M.

¹Peak hours of the existing Sussex Central High School occurred from 7:00-8:00 AM and 2:15-3:15 PM

Existing condition traffic volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figures 4 and 5**, respectively. Manual traffic count data sheets are provided in **Appendix C**.

BASE (NO-BUILD) CONDITIONS

Annual Background Growth

Background growth factors for the roadways in the study area were provided by DelDOT. As such, the background growth factors of 0.5% for Patriots Way and 0% for all other roadways were applied annually to yield an overall growth percentage of 1.51% (0.5% per year compounded over 3 years) for Patriots Way and 0% for the Sussex Central High School Driveways.

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²Peak Hour consists of the four consecutive 15-minute intervals where the highest traffic volumes occur. Peak hours utilized are consistent among the Sussex Central High School driveways.

The additional traffic volumes due to background growth were added to the existing traffic data to produce 2022 base (no-build) condition traffic volumes. Base condition volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figures 5 and 6.**

PROPOSED SITE ACCESS

The proposed site will be served by one full-movement driveway to Patriots Way, opposite the existing Sussex Central High School Driveway (center)

Sight Distance Analysis

A sight distance analysis was prepared for the proposed site driveways. In general, recommended safe sight distances depend upon the posted speed limit and roadway grades. The measured sight distances at the proposed driveway were compared to AASHTO's safe stopping sight distance standard, which is calculated by the following equation:

$$SSSD = 1.47VT + V^2/[30(f\pm g)]$$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Vehicle Speed

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements

g = Percent of Roadway Grade Divided by 100

Tables 4 shows the measured and required sight distances at the site driveways for vehicles entering and exiting the site.

TABLE 4
SIGHT DISTANCE ANALYSIS
SITE DRIVEWAY TO PATRIOTS WAY

District.		6 11	6 13	Sight Distances (feet)	
	Direction	Speed ¹	Grade ²	SSSD	EXIST
Exiting	To the left	35 mph	1%	245	600+
Movements	To the right	35 mph	-1%	252	600+
Entering Left	Approaching same direction	35 mph	-1%	252	600+
Turns	Approaching opposite direction	35 mph	1%	245	600+

SSSD = AASHTO Safe Stopping Sight Distance EXIST = Existing (measured) Sight Distance 1 = Posted speed limit

2 = Roadway Grade Approaching Driveway

As shown in **Table 4** above, the measured sight distances at the site driveway exceeds AASHTO's safe stopping sight distance requirements.

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TRIP GENERATION

The trip generation for the proposed school was developed based on site specific data provided by Howard T. Ennis School staff as coordinated previously with DelDOT. **Table 5** summarizes the peak hour trip generation of the proposed school.

TABLE 5
TRIP GENERATION SUMMARY

Time Deviced	Howard T. Ennis School				
Time Period	Total	Enter	Exit		
Weekday A.M. Peak Hour	199	161	38		
Weekday P.M. Peak Hour	158	0	158		
Average Weekday	420	210	210		

Based on the trip generation summarized in **Table 6**, the proposed development will generate approximately 199 new trips during the weekday A.M. peak hour, and 158 new trips during the weekday P.M. peak hour. Details of the trip generation calculations are included in **Appendix D**.

TRIP DISTRIBUTION

The distribution of staff trips generated by the proposed development was based on information provided by DelDOT. The distribution of bus and parent pick-up/drop-off traffic is consistent with the school's feeder patterns based on the following information provided by the district.

- School buses serve the entirety of Sussex County
- Most buses serve Georgetown and Millsboro
- y 1 bus serving Lewis
- 3 buses serving western Sussex, including Delmar and Greenwood

The new trips for the proposed development were distributed to the local roadway network based on the percentages shown in **Table 6**.

TABLE 6
TRIP DISTRIBUTION PERCENTAGES

Direction - To/From	Assignment	Distribution Percenta	
	(To/From)	Staff	Bus/Parent
North	via Patriots Way	58%	60%
South	via Patriots Way	42%	40%

This distribution is also consistent with the distribution noted at the high school driveways. The assignment of site-generated trips for the proposed development during the weekday A.M. and P.M. peak hours are shown in **Figures 7 and 8**, respectively.

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PROJECTED (BUILD) CONDITION TRAFFIC VOLUMES

The site-generated trips for the proposed development were added to the 2022 base (no-build) condition traffic volumes to develop 2022 projected (build) condition traffic volumes.

In order to accurately model the peak traffic volumes associated with Sussex Central High School in conjunction with the Howard T. Ennis School peak hour traffic volumes, as requested in the DelDOT scoping memo, TPD developed future traffic volumes in 15-minute intervals over the duration of the A.M. and P.M. count periods. As such, the projected condition peak hour traffic volumes calculated based on the 15-minute interval volume development were utilized in the projected condition capacity analyses along with the corresponding calculated peak hour factor. Furthermore, since the P.M. peak hour of the proposed Howard T. Ennis School is slightly offset from the existing high school driveway P.M. peak hour, TPD shifted the proposed school P.M peak hour traffic to coincide with the existing high school driveway P.M. peak hour traffic, providing a conservative (i.e. highest volume) evaluation. The 15-minute volume development worksheets are provided in **Appendix D**.

Projected condition traffic volumes for the future year of 2022 for the weekday A.M. and P.M. peak hours are shown in **Figures 9 and 10**, respectively.

LEVELS OF SERVICE FOR AN INTERSECTION

For analysis of intersections, level of service is defined in terms of delay, which is a measure of driver discomfort and frustration, fuel consumption, and lost travel time. LOS criteria is stated in terms of control delay per vehicle for a one-hour analysis period. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The criteria are shown in **Table 7.** Delay, as it relates to level of service, is a complex measure and is dependent upon a number of variables. For signalized intersections, these variables include the quality of vehicle progression, the cycle length, the green time ratio, and the volume/capacity ratio for the lane group in question. For unsignalized intersections, delay is related to the availability of gaps in the flow of traffic on the major street and the driver's discretion in selecting an appropriate gap for a particular movement from the minor street (straight across, left or right turn).

TABLE 7
LEVEL OF SERVICE CRITERIA
UNSIGNALIZED AND SIGNALIZED INTERSECTIONS 1

Level of Service	Control Delay Per Vehicle (Seconds)			
	Signalized	Unsignalized		
А	< 10	< 10		
В	> 10 and < 20	> 10 and < 15		
С	> 20 and < 35	> 15 and < 25		
D	> 35 and < 55	> 25 and < 35		
E	> 55 and < 80	> 35 and < 50		
F	> 80 or v/c > 1.0	> 50 or v/c > 1.0		

¹Obtained from Exhibits 18-4 and 19-1 of the Transportation Research Board's Highway Capacity Manual 2010

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CAPACITY ANALYSIS METHODOLOGY

Capacity analyses were conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the *Highway Capacity Manual* (HCM) 6th Edition using *Synchro 10* software, a Trafficware product.

The following conditions were analyzed, as applicable:

- Existing conditions;
- » 2022 Base conditions (Build-out year without development);
- 2022 Projected conditions (Build-out year with development);

In addition, capacity analyses were conducted at the proposed site driveway intersection under the 2022 projected conditions. The capacity analysis worksheets are included in **Appendix E**.

LEVELS OF SERVICE IN THE STUDY AREA

Level of service (LOS) matrices for the study area intersections are shown in **Tables 8** for the weekday A.M., and weekday P.M. peak hours.

TABLE 8
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY

		Weekday A.M. Peak Hour			Weekday P.M. Peak Hour		
Intersection	Movement	Existing 2022 Conditions		Existing 2022 Conditions		nditions	
		Conditions	Base	Projected	Conditions	Base	Projected
Patriots Way & Sussex Central High School Driveway (north)	EB LR	E	E	E	С	С	С
	NB L	В	В	В	А	Α	Α
	ILOS	A (7.4)	A (7.6)	A (7.4)	A (8.6)	A (8.7)	A (8.2)
Patriots Way & Sussex Central High School Driveway (center)	WB LTR			С			С
	NB L	Α	А	Α	Α	А	Α
	SB L			Α			Α
	ILOS	A (1.3)	A (1.2)	A (3.1)	A (0.4)	A (0.3)	A (5.2)
Patriots Way & Sussex Central High School Driveway (south)	EB LR	С	С	С	С	С	С
	NB L	Α	А	Α	А	А	Α
	ILOS	A (5.5)	A (5.5)	A (4.8)	A (8.1)	A (8.1)	A (7.9)

Base = No-Build scenario; Projected = Build scenario; ILOS = intersection level of service

As shown in **Table 8** under 2022 projected conditions with the development of the proposed site, the study area intersections will operate at ILOS A during the weekday A.M. and P.M. peak hours.

All approaches and turning movements at the proposed site driveway intersection will operate at LOS C or better under 2022 Projected Conditions during the weekday A.M. and P.M. peak hours.

The capacity analysis worksheets are included in **Appendix E**.

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95TH PERCENTILE QUEUE ANALYSIS

Queue analyses were conducted at the study area intersections using *Synchro 10* software. For this analysis, the 95th percentile queue is defined as the queue length that is exceeded in 5% of the signal cycles. As an example, for a signal with a 90-second cycle, this means that the 95th percentile queue length will be exceeded during 2 of the 40 signal cycles that occur during the peak hour. The queue analysis results are summarized in **Table 9** for the analyzed peak hours.

TABLE 9
95TH PERCENTILE QUEUE ANALYSIS

Intersection		2022 Conditions				
	Movement	Available Storage	Weekday A.M. Peak Hour		Weekday P.M. Peak Hour	
			Base	Projected	Base	Projected
Patriots Way & Sussex Central High School Driveway (north)	EB LR	200+	120	123	98	108
	NB L	120	23	23	3	3
Patriots Way & Sussex Central High School Driveway (center)	WB LTR	100+		15		48
	NB L	235	8	8	3	3
	SB L	225		8		0
Patriots Way & Sussex Central High School Driveway (south)	EB LR	100+	70	63	90	98
	NB L	210	8	5	0	0

As shown in **Table 9**, adequate queue storage will be provided for the turn lanes in 2022 with construction and full build-out of the proposed development. Queue analysis worksheets are included with the capacity analysis worksheets provided in **Appendix E**.

TPD also completed a visual observation of the operations at the northern driveway of Sussex Central High School on Wednesday December 12, 2018 from 7:00 – 7:45 A.M. The existing northbound auxiliary left turn lane has approximately 100' of storage with a 75' taper length.

Based on this observation, the busiest time for northbound left turns entering the high school was between 7:15 and 7:35 A.M. Overall, the queue moved efficiently and was typically only one or two vehicles. The maximum queue of 6 vehicles, which occurred twice throughout the observed time period, did not extend beyond the provided left turn storage and the median area was therefore never occupied by a standing queue at any point.

AUXILIARY TURN LANE ANALYSIS

Methodology

Based on the ADT on Patriots Way, the daily trip generation for the site and the distribution of traffic, TPD evaluated the provision of a northbound right turn lane and southbound left turn lane for traffic entering the site.

Findings:

Table 10 summarizes the results of the auxiliary turn lane analysis at the proposed site driveway.

TABLE 10
AUXILIARY TURN LANE ANALYSIS SUMMARY

Intersection	Auxiliary Lane	Warrant Satisfied?	Required Lane Length ¹	Proposed Lane Length
Patriots Way & Proposed Site Access	NB Right-Turn Lane	No		
	SB Left-Turn Lane	Yes	220′	225′

¹Based on a 35 mph speed limit

The calculations for the auxiliary turn lane warrants are included in **Appendix F**.

PEDESTRIAN CROSSING ANALYSIS

Based on the *Guidelines for Pedestrian Crossing Treatments* contained in Appendix A of the NCHRP Report 562 "Improving Pedestrian Safety at Unsignalized Crossing" the following pedestrian crossing treatment could be considered at the proposed site.

» Painted crosswalk between the proposed Howard T. Ennis School Driveway and the existing Sussex Central High School center driveway

It should be noted, for purposes of this evaluation and at the request of DelDOT, TPD assumed the pedestrian peak hour volume threshold of 20 pedestrians per hour for both directions would be met under future conditions, despite the negligible existing pedestrian volumes and minimal pedestrian accommodations on the surrounding roadway network.

The completed NCHRP worksheets for the weekday A.M. and P.M. peak hours is provided in **Appendix G**.

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RECOMMENDATIONS

TPD has made the following recommendations in relation to the proposed development in Sussex County Delaware:

Patriots Way & Proposed Site Driveway

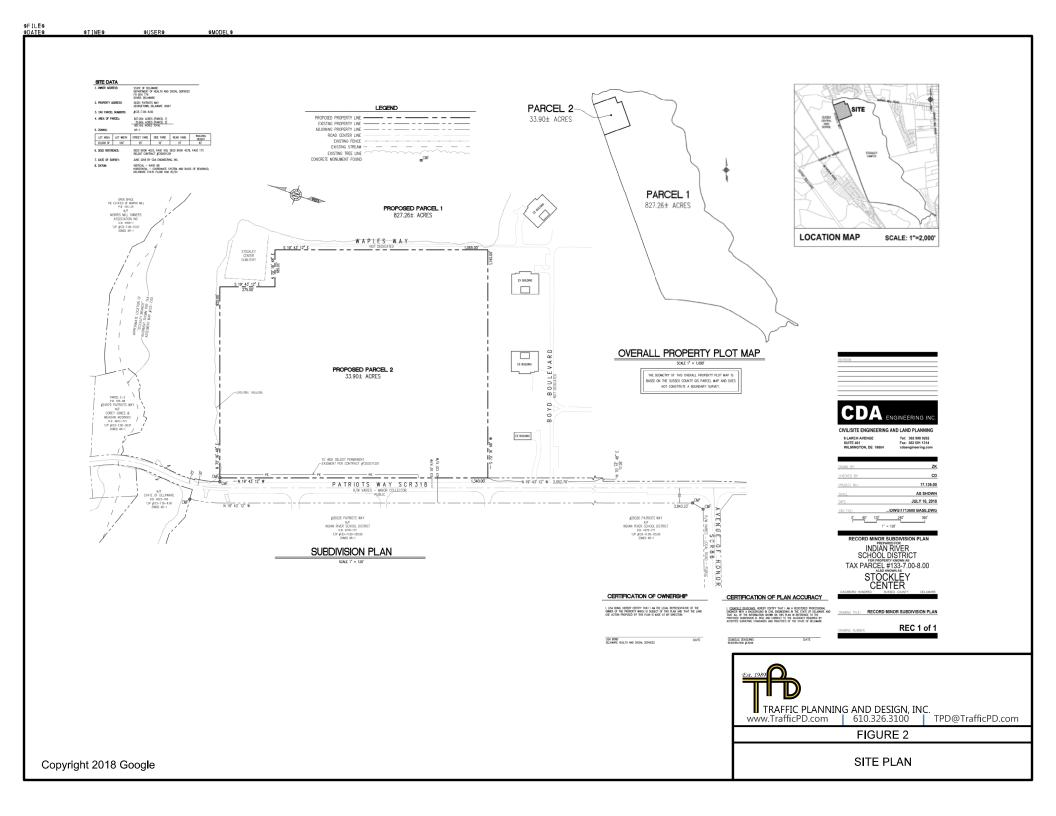
- Provide a stop sign to control exiting traffic
- » Construct a 225-foot southbound left turn lane within the existing painted median
- Consider installation of a painted crosswalk between the proposed Howard T. Ennis School driveway and the existing Sussex Central High School Driveway (center)

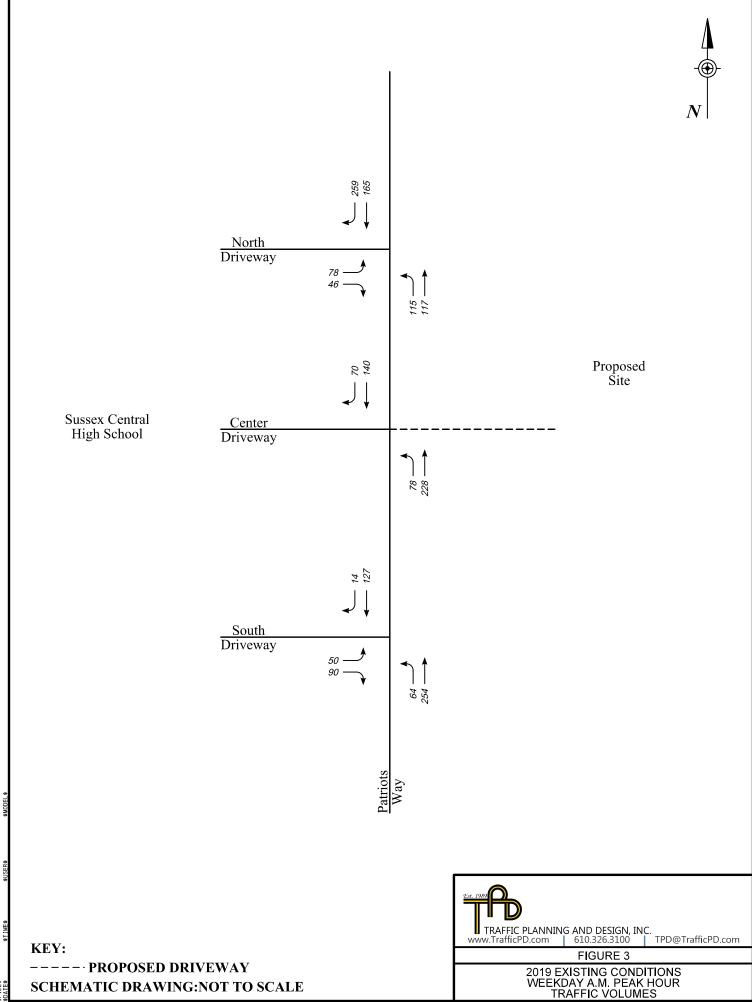
CONCLUSIONS

Based on the results of the transportation impact study, TPD offers the following conclusions:

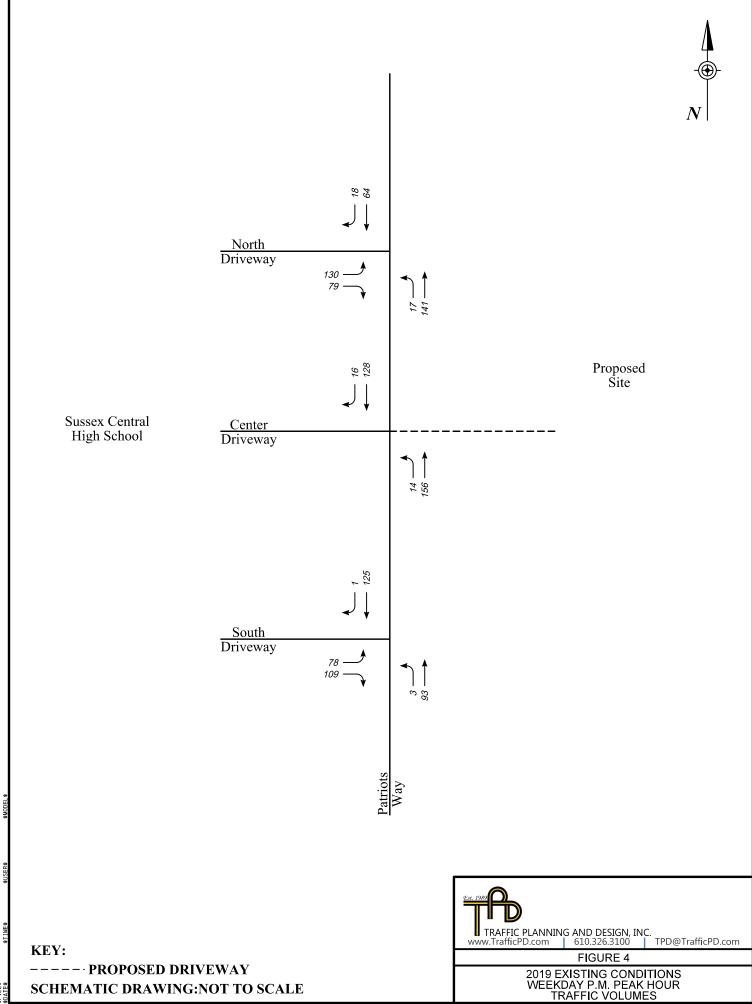
- The project site is located on the east side of Patriots way, north of Boyd Boulevard and opposite the existing Sussex Central High School. The proposed site will consist of a 157,000 s.f. special needs school
- » Access to the site is proposed via one full-access driveway to Patriots Way, opposite the existing Sussex Central School Driveway (center)
- » All proposed driveway location sight distances will exceed AASHTO's Safe Stopping Sight Distance (SSSD) criteria.
- Upon full build-out, the proposed development is expected to generate 199 new vehicle-trips during the weekday A.M. peak hour, and 158 new vehicle-trips during the weekday P.M. peak hour.
- Under the 2022 projected conditions, with implementation of the site-related recommendations, all approaches and turning movements at the site driveway intersection with the external roadway network will operate at <u>LOS C or better</u> during weekday A.M. and P.M. peak hours.
- All overall intersection levels of service (ILOS) will operate at an ILOS A during the 2022 projected condition scenarios.

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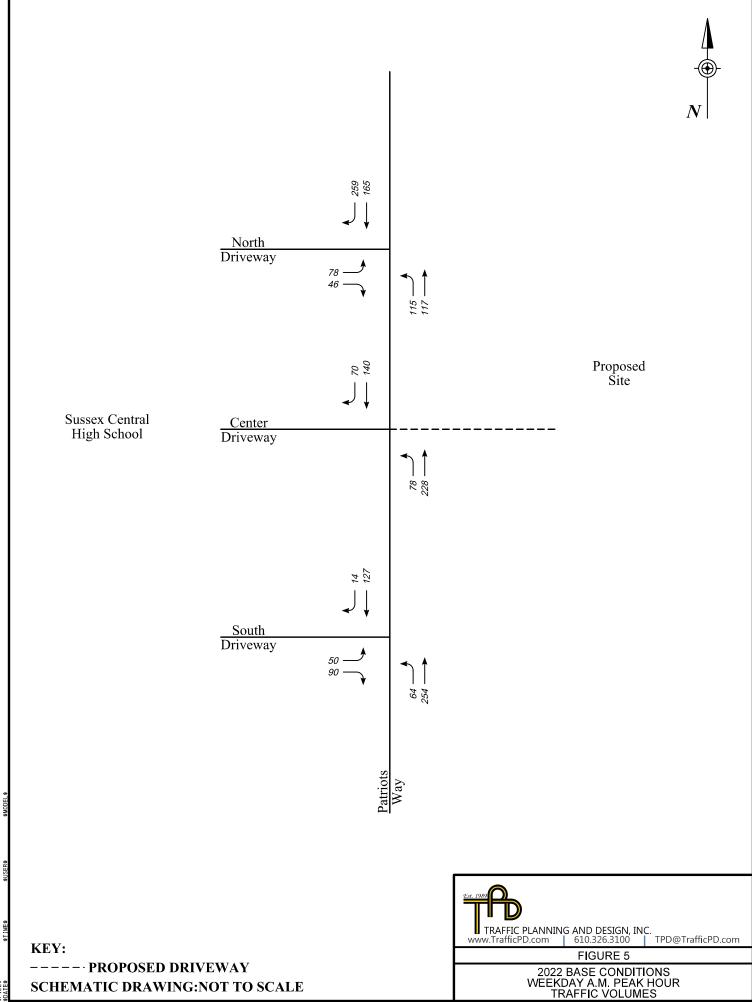




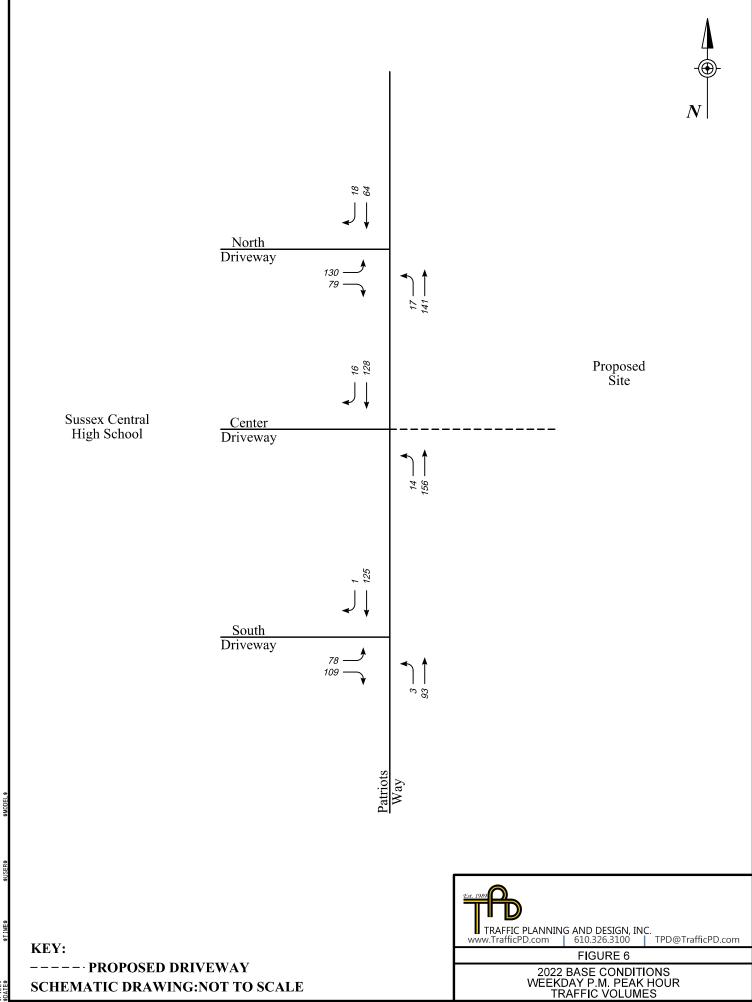
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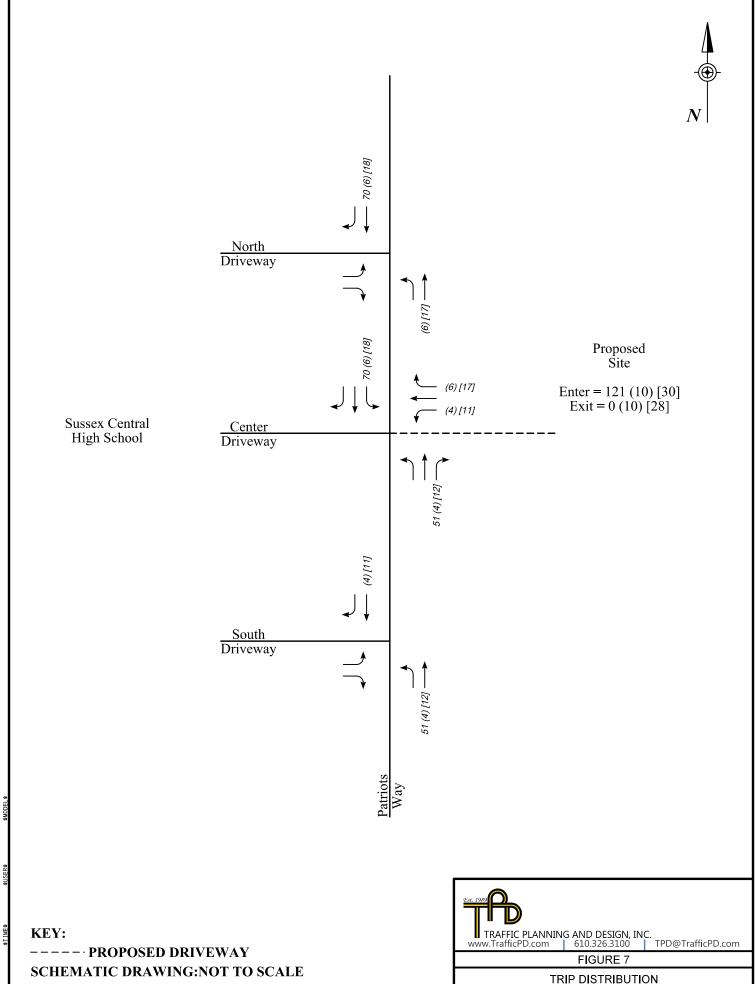
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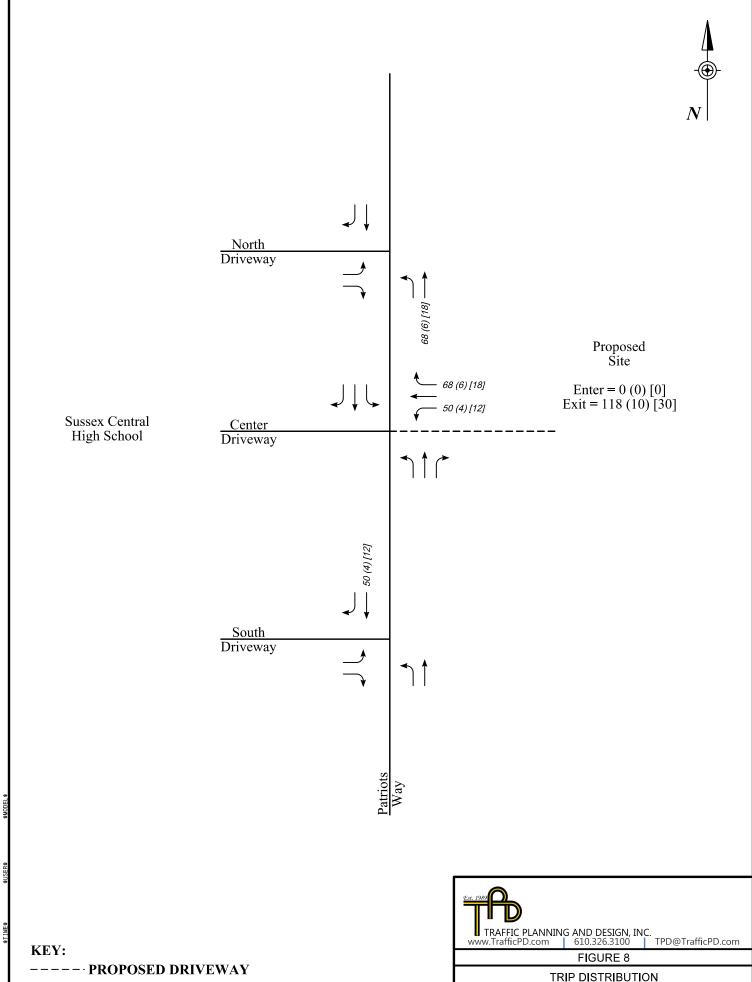
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WEEKDAY A.M. PEAK HOUR

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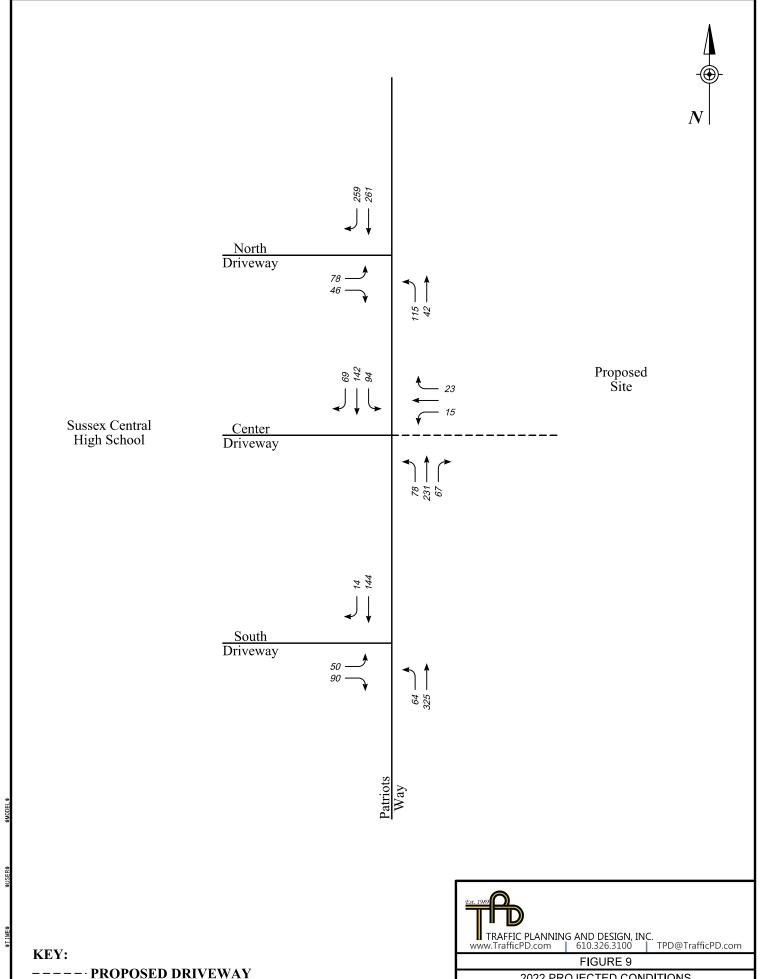
STAFF (PARENT) [BUS] TRIPS



WEEKDAY P M PEAK HOUR

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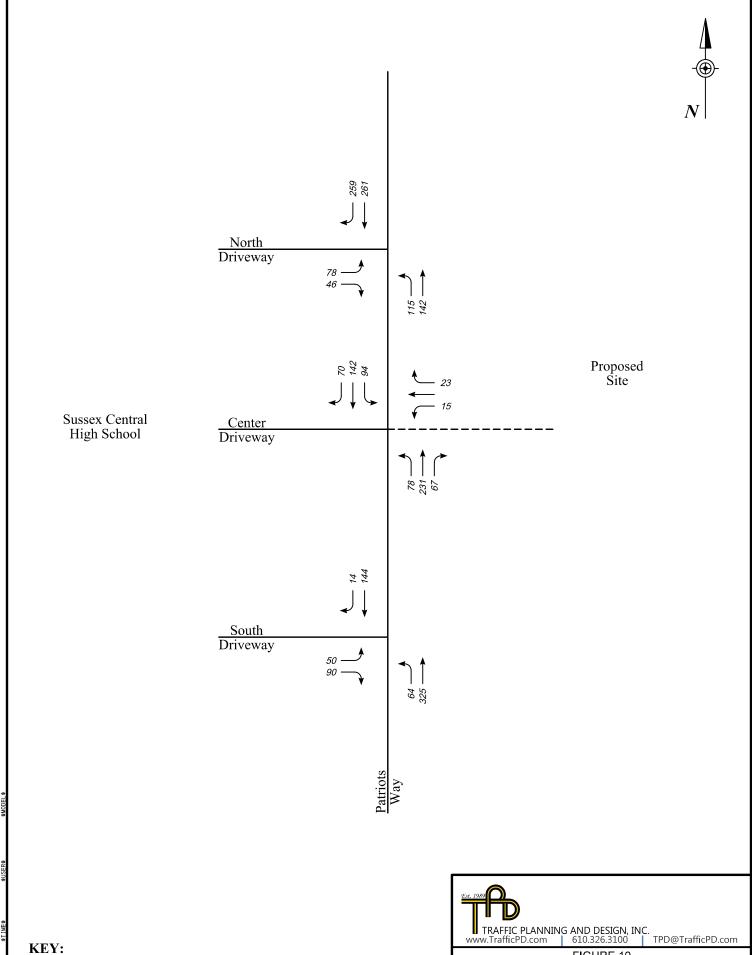
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2022 PROJECTED CONDITIONS WEEKDAY A.M. PEAK HOUR TRAFFIC VOLUMES

FF ILES

SCHEMATIC DRAWING:NOT TO SCALE



---- PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE FIGURE 10

2022 PROJECTED CONDITIONS WEEKDAY P.M. PEAK HOUR TRAFFIC VOLUMES

Appendix A PROJECT CORRESPONDENCE



STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

December 20, 2019

Mr. Eric Ostimchuk Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, PA 19464

Dear Mr. Ostimchuk:

We have reviewed the traffic counts that we received on December 6, 2019 for the **Howard T. Ennis School** (Protocol Tax Parcel 133-7.00-8.00) traffic impact study (TIS). Upon our review, we find that the traffic counts are acceptable as submitted.

Considering background growth factors, please apply the following growth factors to the seasonally adjusted traffic volumes in developing future traffic:

Road	Growth Factor	Total Growth from 2019 to 2022
Patriots Way (Sussex Road 318)	1.005	1.015
All Other Roads	1.000	1.000

You may contact Mr. Troy Brestel at (302) 760-2167 if you have any questions concerning this correspondence.

Sincerely,

T. William Brockenbrough, Jr.

Trey But I for

County Coordinator

TWB:tbf

cc: Shelby Lynch, Traffic Planning and Design, Inc.

Janelle Cornwell, Sussex County Planning and Zoning

J. Marc Coté, Assistant Director, Development Coordination

Troy Brestel, Project Engineer, Development Coordination

Claudy Joinville, Project Engineer, Development Coordination





STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

MEMORANDUM

TO: File

FROM: Claudy Joinville, Project Engineer

DATE: October 14, 2019

SUBJECT: Howard T. Ennis School

Traffic Operational Analysis (TOA) - Scoping Meeting (9/12/19)

Scope of Work

ATTENDANCE: Janelle Cornwell, Sussex County Planning and Zoning (via skype)

Colmcille DeAscanis, CDA Engineering, Inc. (via skype)

Zak Kerstetter, CDA Engineering, Inc.

Eric Ostimchuk, Traffic Planning & Design (TPD)

Ken Fearn, Fearn Clendaniel Architects Peter Haag, DelDOT Traffic (via skype)

Marc Coté, DelDOT Planning Susanne Laws, DelDOT Planning John Andrescavage, DelDOT Planning

T. William Brockenbrough, Jr., DelDOT Planning

Claudy Joinville, DelDOT Planning

Background and Discussion

Indian River School District seeks to develop a 157,000 square-foot Special Needs school on an approximately 32.43-acre parcel (Tax Parcel: 133-7.00-8.01). The land is located on the east side of Patriots Way (Sussex Road 318), directly opposite the Sussex Central High School, south of Georgetown. The land is currently zoned as AR-1 (Agricultural Residential), and the developer does not plan to rezone the land.

One full access point is proposed along Patriots Way. Construction is anticipated to be complete in 2022.



Memorandum to File October 14, 2019 Page 2 of 5

Cases to be Evaluated

The study shall evaluate the weekday morning and weekday evening peak hours for the following situations:

- 1) Existing (2019);
- 2) 2022 without development; and
- 3) 2022 with development.

Facilities to be Evaluated

The TOA should evaluate conditions at the following intersections for capacity and level of service using the Highway Capacity Software (HCS). It should also evaluate the extent to which they meet the relevant DelDOT, AASHTO and MUTCD standards for geometry and traffic control devices.

- 1) Patriots Way (Sussex Road 318) / Site Entrance / Sussex Central High School Entrance (center)
- 2) Patriots Way / Sussex Central High School Entrance (north)
- 3) Patriots Way / Sussex Central High School Entrance (south)

Traffic Counts

The Consultant should conduct traffic counts for the intersections listed above from 7:00 a.m. to 9:00 a.m. and from 2:00 p.m. to 4:00 p.m., on a Tuesday, Wednesday or Thursday to determine when the peaks occur. The weekday traffic counts should be performed during a time when schools are open and operating at a normal capacity.

Additionally, an Automatic Traffic Recorder (ATR) should be used to collect traffic data on Patriots Way south of the south high school entrance. The ATR should be placed for a one-week time period that includes the date(s) of the manual traffic counts. The ATR data will be used to verify the manual counts and determine whether adjustments are required.

Section 2.2.8.5, item 19, under Existing Traffic and Transportation Conditions in the <u>Development Coordination Manual</u>, addresses how oversaturated intersections are to be counted.

The traffic counts should be submitted to DelDOT both electronically as Portable Document Format (PDF)/Excel files and as draft report figures showing peak hour volumes (<u>labeled with date and peak hour interval</u>) posted on diagrams of the road network.

The Consultant should include counts of pedestrians, a separate count of right-turn on red (in addition to right-turn movement counts), and a separate count of heavy vehicles.

Memorandum to File October 14, 2019 Page 3 of 5

The Consultant should be alert for events affecting the traffic counts, such as accidents or nearby construction and shall make note of any such events when submitting the counts. As necessary, DelDOT reserves the right to reject the counts or require adjustments to them.

Trip Generation

DelDOT is agreeable to the Consultant's use of historical and site specific data to calculate the trips for the proposed school.

Trip Distribution

A trip distribution to be used for the site were developed using DelDOT's Travel Demand Model and is attached. School bus and parent traffic should reflect the school's feeder patterns as the model distribution only applies to staff traffic.

Future Growth

The Consultant shall apply growth factors to the traffic counts. DelDOT will develop those factors after we receive the Consultant's traffic counts.

Pedestrian Traffic Analysis

The Consultant shall complete a pedestrian crossing analysis using NCHRP 562. The analysis should evaluate multiple crossing scenarios, such as, but not limited to, a single crossing and a two-stage crossing with a median refuge island. In addition, the analysis should assume the pedestrian volume has met the NCHRP threshold. For more guidance on this analysis, the Consultant shall contact Peter Haag, Traffic Studies Manager of DelDOT's Traffic Section. Mr. Haag may be reached at (302) 659-4084.

Highway Capacity Software & Synchro

The Consultant shall use the most recent version of the Highway Capacity Software (HCS) that implements the 6th Edition of the <u>Highway Capacity Manual</u> (HCM). Presently, that is HCS7.

In addition, the Consultant shall use Synchro to conduct 15-minute analysis periods to accurately model the peak traffic volumes associated with the Sussex Central High School opposite the proposed school.

Memorandum to File October 14, 2019 Page 4 of 5

Seasonal Adjustment Factors for the roads in the study area are as follows:

Roads	September	October	November
Patriots Way (Sussex Road 318)	0.96	0.95	0.98
All Other Roads	1.00	1.00	1.00

DelDOT Projects

Currently, DelDOT has no active projects within the study area.

Transit, Bicycle, and Pedestrian Facilities

The study should describe the existing and proposed transit service in the project area and should also describe the existing and needed transit, bicycle, and pedestrian facilities on or near the project site. In determining these items, the Consultant shall contact Mr. David Dooley, a Service Development Planner at the Delaware Transit Corporation (DTC), and Mr. Anthony Aglio, of DelDOT's Local Systems Section. Mr. Dooley may be reached at (302) 576-6064. Mr. Aglio may be reached at (302) 760-2509.

General Notes

- 1) All submissions relating to this study should be made electronically via the Planning and Development Coordination Application (PDCA), preferably in Portable Document Format (PDF).
- 2) The Consultant should e-mail DelDOT's Transportation Management Center (TMC) at tmc1@delaware.gov to obtain advance approval for the use of any signal timings.
- Before deploying temporary unmanned devices, e.g. cameras or radar detectors, in the State-maintained right-of-way, the individual or company proposing to do so shall execute and file a Right-of-Way Use Agreement. Before each specific deployment of devices, the individual or company shall email a completed Temporary Data Collection Device Notification Form to TMC1@delaware.gov. Deployment of Automatic Traffic Recorders, a.k.a. tube counters, and devices on portable trailers does not require a Right-of-Way Use Agreement but does require submission of the Temporary Data Collection Device Notification Form. Copies of the standard agreement and the form are available from Ms. Lara Brown at (302) 659-4062 or Lara.Brown@delaware.gov.
- 4) The Consultant should refer to the attached memorandum from Scott Neidert of DelDOT's Traffic Section for guidance regarding requests for crash data within the study area. The Consultant shall report on this data and make recommendations for improvements if safety problems exist in the study area. Mr. Neidert may be reached at (302) 659-4075.

- 5) Both DelDOT and Sussex County reserve the right to change this scope of work if the study is not performed within a reasonable time.
- 6) The developer may choose to have DelDOT's Consultant perform the TOA rather than use their own Consultant. If this option is of interest, the developer should contact Mr. Troy Brestel at (302) 760-2167 to request a cost estimate.
- 7) By copy of this memorandum I ask those copied to contact me at (302) 760-2124 regarding any significant errors or omissions.

CJ:cim

Enclosure

cc: Drew Boyce, Director, Planning

Joseph Booth, Indian River School District

Michael Simmons, Assistant Director for Project Development South, DOTS

Alastair Probert, South District Engineer, DOTS

Gemez Norwood, South District Public Works Supervisor, DOTS

William Kirsch, South District Permit Supervisor, DOTS

Mark Whiteside, Project Manager, Project Development – South, DOTS

Scott Neidert, Design Resource Engineer, Traffic Section

Mark Buckalew, Traffic Safety Engineer, DelDOT Traffic

David Dooley, Service Development Planner, Delaware Transit Corporation

Anthony Aglio, Planning Supervisor, Local Systems

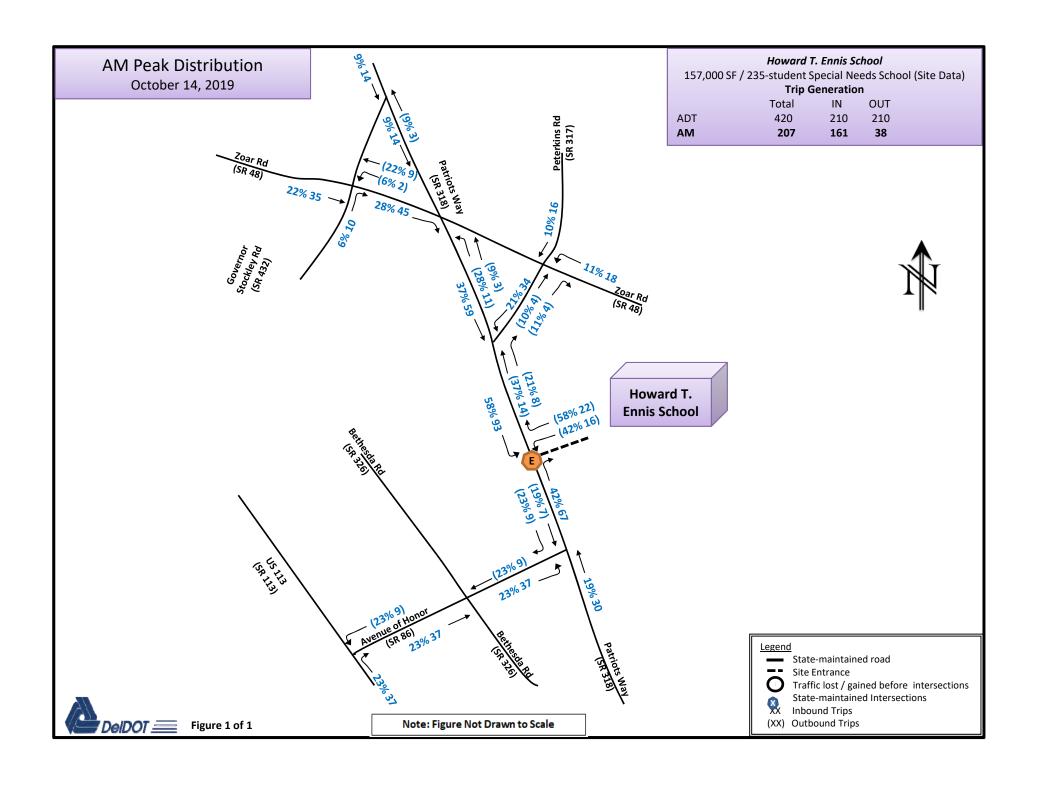
Lara Brown, Administrative Specialist, DelDOT Traffic DOTS

Kari Glanden, Statistical Information Supervisor, DelDOT Traffic, DOTS

Mark Galipo, Traffic Engineer, DelDOT Traffic, DOTS

Andrew Parker, McCormick & Taylor, Inc.

Mir Wahed, Johnson, Mirmiran, & Thompson, Inc.





STATE OF DELAWARE

DEPARTMENT OF TRANSPORTATION

800 BAY ROAD
P.O. BOX 778
DOVER, DELAWARE 19903

JENNIFER COHAN SECRETARY

TO: Requestors of Crash Data via DelDOT's Development Coordination Process

FROM: Scott Neidert, Design Resource Engineer, Traffic Section

DATE: September 5, 2019

SUBJECT: Revisions to Crash Data Requests and Releases

As of July 23, 2019, Governor Carney has signed SB 147 into law containing amendments to the release of crash data, namely permitting DelDOT to release certain de-identified data based on the nature of the requestor. Specifically, newly enacted 21 *Del. C.* §313(c)(1) provides that:

"The Department of Transportation may provide the information under this subsection if the person requesting the information provides <u>proof of identity and a sworn representation</u> that the data will be strictly used for any of the following purposes:

- a. To comply with federal, State, or local law or regulations.
- b. By a municipality or municipal planning organization in carrying out official functions."

To conform with the "proof of identity and... sworn representation..." clause, requestors will be required to complete an online crash data request as well as provide a <u>notarized</u> release form to be submitted with <u>each</u> crash request prior to being processed. A link to the release form is provided within the online crash request.

Effective immediately, all requests for crash data, when required, must be made at: https://tmc.deldot.gov/tmcx/app/crashdata/public/info.html

Requests for crash data will not be processed until all required fields are completed, and the release form has been completed and received.

SN

cc: Nicole Majeski, Deputy Secretary

Shanté Hastings, Chief Engineer

Drew Boyce, Director, Planning

Annie Cordo, Deputy Attorney General

Mark Luszcz, Deputy Director, Division of Transportation Solutions

Kari Glanden Thompson, Statistical Information Supervisor, Traffic Section



Appendix B STUDY AREA PHOTOGRAPHS



Direction / Road: Approach / Departure: Approach

> Distance: 50 feet



Approach / Departure: Distance:

Approach

200 feet



NB Patriots Way Direction / Road: Approach / Departure: Departure Distance: 50 feet



Direction / Road: Approach / Departure:

NB Patriots Way Departure



SB Patriots Way Direction / Road: Approach / Departure: Approach 50 feet Distance:



SB Patriots Way **Direction / Road:** Approach / Departure: Approach 200 feet Distance:



SB Patriots Way Direction / Road: Approach / Departure: Departure

Distance: 50 feet



Direction / Road: Approach / Departure:

SB Patriots Way

Departure



EB Sussex Central High School DW (North) Direction / Road:

Approach / Departure: Approach

Distance: 50 feet



EB Sussex Central High School DW (North) Direction / Road:

Approach / Departure: Approach Distance:

200 feet



EB Sussex Central High School DW (North) Direction / Road:

Approach / Departure:

Departure

Distance: 50 Feet



Approach / Departure:

EB Sussex Central High School DW (North)

Departure



NB Patriots Way Direction / Road: Approach / Departure: Approach

50 feet

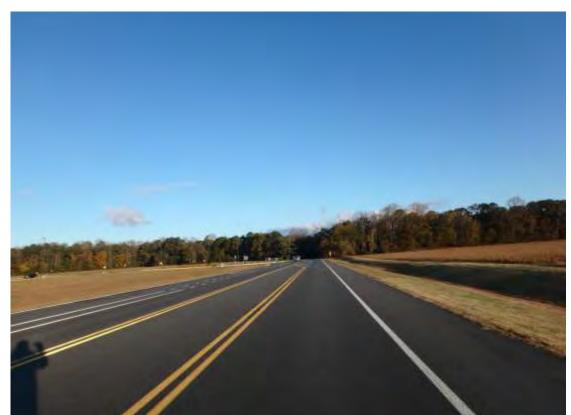
Distance:



Approach / Departure:

NB Patriots Way

Approach



NB Patriots Way Direction / Road: Approach / Departure: Departure Distance: 50 feet

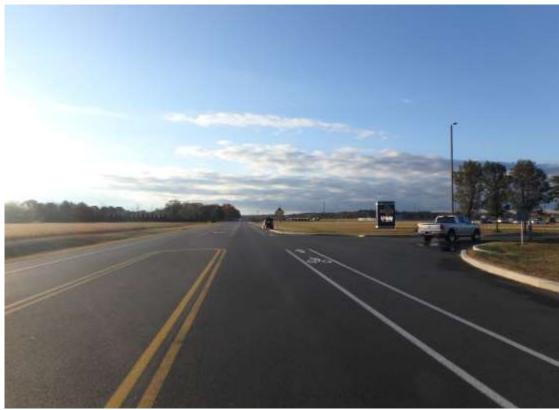


Direction / Road: Approach / Departure:

NB Patriots Way Departure

Distance:

200 feet



SB Patriots Way **Direction / Road:** Approach / Departure: Approach

> Distance: 50 feet



SB Patriots Way Direction / Road:

Approach / Departure: Approach



Direction / Road: SB Patriots Way Approach / Departure: Departure

Distance:

50 feet



Approach / Departure:

SB Patriots Way

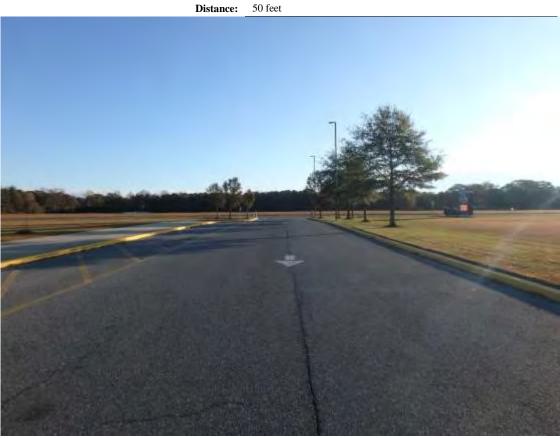
Departure



EB Sussex Central High School DW (Center) Direction / Road:

Approach / Departure:

Approach



Direction / Road: Approach / Departure:

EB Sussex Central High School DW (Center)

Approach



EB Sussex Central High School DW (Center) Direction / Road:

Approach / Departure:

Departure

Distance: 50 Feet



Direction / Road:

EB Sussex Central High School DW (Center)

Approach / Departure:

Departure

Distance:

200 feet



Direction / Road: Approach / Departure: Approach

Distance: 50 feet



Direction / Road: Approach / Departure: Distance: NB Patriots Way Approach

200 feet



Direction / Road: Approach / Departure:

NB Patriots Way Departure

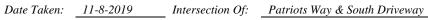
Distance: 50 feet





Direction / Road: Approach / Departure: NB Patriots Way

Departure





Direction / Road: Approach / Departure: Approach Distance: 50 feet



SB Patriots Way Direction / Road: Approach / Departure: Approach 200 feet Distance:



Direction / Road: Approach / Departure: Departure Distance: 50 feet



Direction / Road: Approach / Departure:

SB Patriots Way Departure

Distance:

200 feet





Direction / Road: EB Sussex Central High School DW (South)

Approach / Departure: Approach



EB Sussex Central High School DW (South) Direction / Road: Approach / Departure: Approach



EB Sussex Central High School DW (South) **Direction / Road:** Approach / Departure: Departure

Distance: 50 Feet



Approach / Departure: Departure

Appendix C *MANUAL TRAFFIC COUNT PRINTOUTS*



Counted By: Mio: Set Up By: JH: Weather:Clear:

Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 1

Turning Movement Data

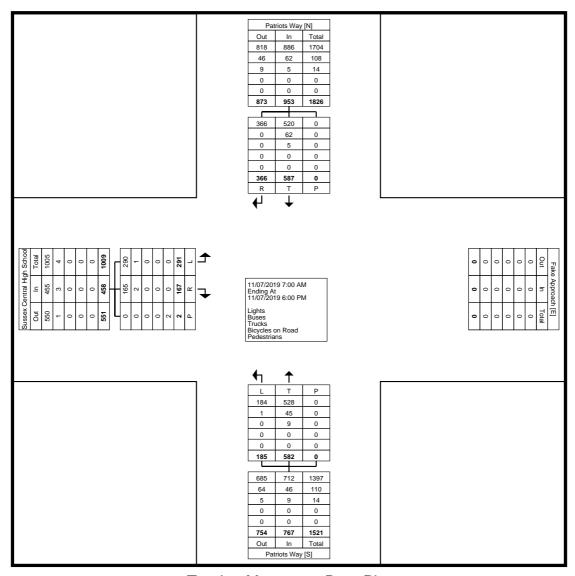
				ΙŲ	irning I	Movem	nent Da	ata					
	Susse	x Central High	School DW	/ (North)	Patriots Way								
O:		Easth	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	14	13	0	27	44	28	0	72	51	75	0	126	225
7:15 AM	30	22	0	52	62	32	0	94	56	146	0	202	348
7:30 AM	29	9	0	38	10	38	0	48	32	37	0	69	155
7:45 AM	5	2	0	7	1	21	0	22	29	6	0	35	64
Hourly Total	78	46	0	124	117	119	0	236	168	264	0	432	792
8:00 AM	3	0	0	3	6	13	0	19	21	5	0	26	48
8:15 AM	0	0	0	0	5	19	. 0	24	19	9	0	28	52
8:30 AM	3	. 1	0	4	1	17	0	18	19	4	0	23	45
8:45 AM	4	2	0	6	1	16	0	17	17	6	0	23	46
Hourly Total	10	3	0	13	13	65	0	78	76	24	0	100	191
*** BREAK ***	-		-	-	-	-	-		-	-	-	-	-
2:00 PM	4	4	0	8	5	23	0	28	22	6	0	28	64
2:15 PM	3	1	0	4	14	19	. 0	33	20	25	0	45	82
2:30 PM	64	46	0	110	11	70	. 0	81	13	5	0	18	209
2:45 PM	17	12	0	29	0	19	0	19	14	5	0	19	67
Hourly Total	88	63	0	151	30	131	0	161	69	41	0	110	422
3:00 PM	24	. 8	1	32	2	31	. 0	33	15	7	0	22	87
3:15 PM	25	13	1	38	4	24	0	28	23	1	0	24	90
3:30 PM	12	5	0	17	5	22	0	27	20	2	0	22	66
3:45 PM	12	9	0	21	3	25	. 0	28	25	5	0	30	79
Hourly Total	73	35	2	108	14	102	0	116	83	15	0	98	322
4:00 PM	4	5	0	9	0	31	0	31	25	6	0	31	71
4:15 PM	12	4	0	16	2	24	0	26	23	10	0	33	75
4:30 PM	7	10	0	17	4	26	0	30	29	2	0	31	78
4:45 PM	13	0	0	13	1	13	0	14	19	2	0	21	48
Hourly Total	36	19	0	55	7	94	. 0	101	96	20	0	116	272
5:00 PM	3	1	0	4	4	24	0	28	19	0	0	19	51
5:15 PM	0	0	0	0	0	15	0	15	35	2	0	37	52
5:30 PM	1	. 0	0	. 1	0	19	. 0	19	24	0	0	24	44
5:45 PM	2	0	0	2	0	13	0	13	17	0	0	17	32
Hourly Total	6	1	0	7	4	71	0	75	95	2	0	97	179
Grand Total	291	167	2	458	185	582	. 0	767	587	366	0	953	2178
Approach %	63.5	36.5	-	-	24.1	75.9		-	61.6	38.4	-	-	-
Total %	13.4	7.7	-	21.0	8.5	26.7	-	35.2	27.0	16.8	-	43.8	
Lights	290	165	-	455	184	528	-	712	520	366	-	886	2053
% Lights	99.7	98.8	-	99.3	99.5	90.7	-	92.8	88.6	100.0	-	93.0	94.3
Buses	1	2	-	3	1	45	-	46	62	0	-	62	111
% Buses	0.3	1.2	-	0.7	0.5	7.7	-	6.0	10.6	0.0	-	6.5	5.1
Trucks	0	0	-	0	0	9	-	9	5	0	-	5	14
% Trucks	0.0	0.0	-	0.0	0.0	1.5	-	1.2	0.9	0.0	-	0.5	0.6
Bicycles on Road	0	. 0	-	. 0	0	0		0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-		2	-	-		0	-			0	-	
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-



Counted By: Mio: Set Up By: JH: Weather:Clear:

Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 2



Turning Movement Data Plot



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North)
Site Code: Start Date: 11/07/2019
Page No: 3

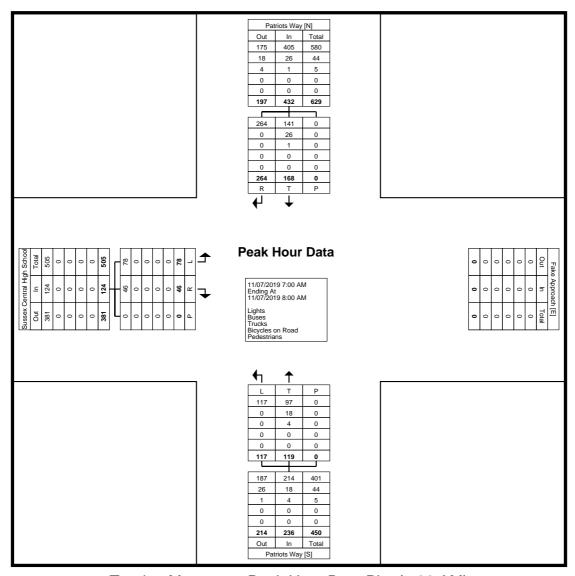
Turning Movement Peak Hour Data (7:00 AM)

				.9				~ (, , , , , , ,				
	Susse	x Central High	School DV	/ (North)		Patriot	ts Way			Patriot	s Way		
Start Time		Easth	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	14	13	0	27	44	28	0	72	51	75	0	126	225
7:15 AM	30	22	0	52	62	32	0	94	56	146	0	202	348
7:30 AM	29	9	0	38	10	38	0	48	32	37	0	69	155
7:45 AM	5	2	0	7	1	21	0	22	29	6	0	35	64
Total	78	46	0	124	117	119	0	236	168	264	0	432	792
Approach %	62.9	37.1	-	-	49.6	50.4	-	-	38.9	61.1	-	-	-
Total %	9.8	5.8	-	15.7	14.8	15.0	-	29.8	21.2	33.3	-	54.5	-
PHF	0.650	0.523	-	0.596	0.472	0.783	-	0.628	0.750	0.452	-	0.535	0.569
Lights	78	46	-	124	117	97	-	214	141	264	-	405	743
% Lights	100.0	100.0	-	100.0	100.0	81.5	-	90.7	83.9	100.0	-	93.8	93.8
Buses	0	0	-	0	0	18	-	18	26	0	-	26	44
% Buses	0.0	0.0	-	0.0	0.0	15.1	-	7.6	15.5	0.0	-	6.0	5.6
Trucks	0	0	-	0	0	4	-	4	1	0	-	1	5
% Trucks	0.0	0.0	-	0.0	0.0	3.4	-	1.7	0.6	0.0	-	0.2	0.6
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	-		-	-	-	-	-	-	-	-	-



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 4



Turning Movement Peak Hour Data Plot (7:00 AM)



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 5

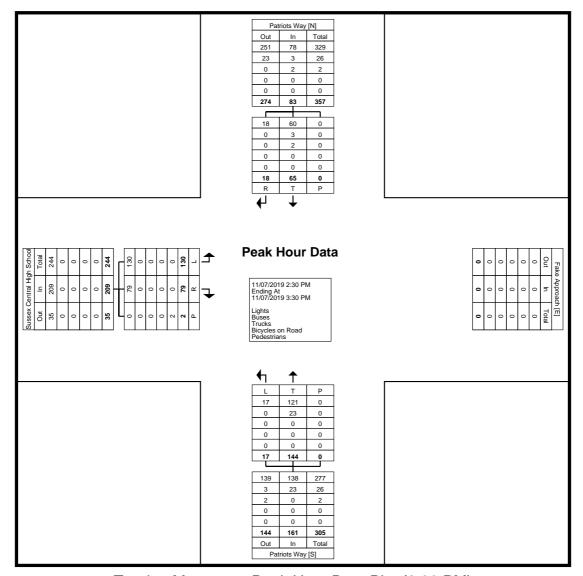
Turning Movement Peak Hour Data (2:30 PM)

								` `	,				
	Susse	x Central High	n School DW	/ (North)		Patriot	ts Way			Patriot	s Way		
Ctart Time		Easth	oound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	64	46	0	110	11	70	0	81	13	5	0	18	209
2:45 PM	17	12	0	29	0	19	0	19	14	5	0	19	67
3:00 PM	24	8	1	32	2	31	0	33	15	7	0	22	87
3:15 PM	25	13	1	38	4	24	0	28	23	1	0	24	90
Total	130	79	2	209	17	144	0	161	65	18	0	83	453
Approach %	62.2	37.8	-	-	10.6	89.4	-	-	78.3	21.7	-	-	-
Total %	28.7	17.4	-	46.1	3.8	31.8	-	35.5	14.3	4.0	-	18.3	-
PHF	0.508	0.429	-	0.475	0.386	0.514	-	0.497	0.707	0.643	-	0.865	0.542
Lights	130	79	-	209	17	121	-	138	60	18	-	78	425
% Lights	100.0	100.0	-	100.0	100.0	84.0	-	85.7	92.3	100.0	-	94.0	93.8
Buses	0	0	-	0	0	23	-	23	3	0	-	3	26
% Buses	0.0	0.0	-	0.0	0.0	16.0	-	14.3	4.6	0.0	-	3.6	5.7
Trucks	0	0	-	0	0	0	-	0	2	0	-	2	2
% Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	3.1	0.0	-	2.4	0.4
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	2	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	-		-	-	-	-	-



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 6



Turning Movement Peak Hour Data Plot (2:30 PM)



Traffic Planning and Design, Inc
2500 East High Street
Suite 650
Pottstown, Pennsylvania, United States 19464
610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (North) Site Code: Start Date: 11/07/2019 Page No: 7



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 1

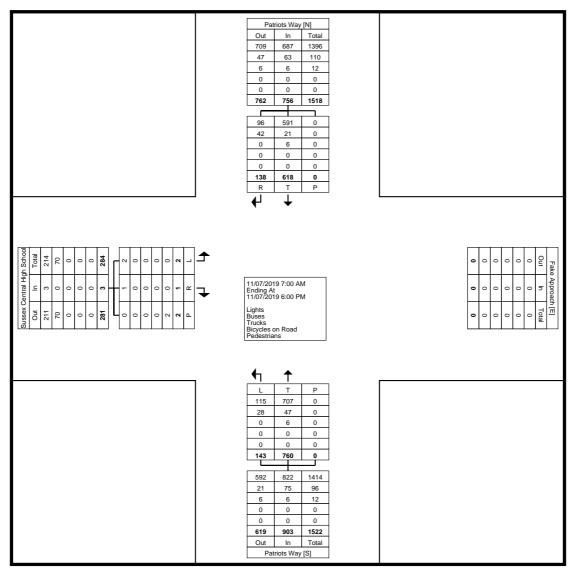
Turning Movement Data

	_			ΙŲ	irning i	Movem	ient D	ata					
	Sussex	Central High	School DW	(Center	•	Patriot	ts Way			Patriot	s Way		
		Eastb	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	0	0	0	0	28	78	0	106	34	34	0	68	174
7:15 AM	0	0	0	0	32	90	0	122	50	27	0	77	199
7:30 AM	0	0	0	0	17	44	0	61	32	10	0	42	103
7:45 AM	0	0	0	0	3	21	0	24	27	0	0	27	51
Hourly Total	0	0	0	0	80	233	0	313	143	71	0	214	527
8:00 AM	0	0	0	0	5	21	0	26	18	5	0	23	49
8:15 AM	0	0	0	0	2	23	0	25	21	0	0	21	46
8:30 AM	1	0	0	1	5	17	0	22	17	3	0	20	43
8:45 AM	0	0	0	0	2	18	0	20	16	1	0	17	37
Hourly Total	1	0	0	. 1	14	79	0	93	72	9	0	81	175
*** BREAK ***	-		-	-	-	_	-	-	-	_	-	-	-
2:00 PM	0	0	0	0	9	32	0	41	11	11	0	22	63
2:15 PM	0	0	0	0	9	30	0	39	14	9	0	23	62
2:30 PM	0	0	0	0	5	79	0	84	52	6	0	58	142
2:45 PM	1	0	0	1	5	19	0	24	25	4	0	29	54
Hourly Total	1	0	0	1	28	160	0	188	102	30	0	132	321
3:00 PM	0	0	1	0	3	36	0	39	25	4	0	29	68
3:15 PM	0	0	1	0	1	25	0	26	29	2	0	31	57
3:30 PM	0	0	0	0	4	29	0	33	22	2	0	24	57
3:45 PM	0	0	0	0	4	27	0	31	28	4	0	32	63
Hourly Total	0	0	2	0	12	117	0	129	104	12	0	116	245
4:00 PM	0	0	0	0	3	33	0	36	28	5	0	33	69
4:15 PM	0	0	0	. 0	3	23	0	26	24	1	0	25	51
4:30 PM	0	0	0	0	0	27	0	27	33	5	0	38	65
4:45 PM	0	0	0	0	2	20	0	22	20	0	0	20	42
Hourly Total	0	0	0	0	8	103	0	111	105	11	0	116	227
5:00 PM	0	0	0	0	0	22	0	22	19	2	0	21	43
5:15 PM	0	0	0	0	1	14	0	15	34	2	0	36	51
5:30 PM	0	1	0	1	0	19	0	19	20	1	0	21	41
5:45 PM	0	0	0	0	0	13	0	13	19	0	0	19	32
Hourly Total	0	1	0	1	1	68	0	69	92	5	0	97	167
Grand Total	2	1	2	3	143	760	0	903	618	138	0	756	1662
Approach %	66.7	33.3	-	-	15.8	84.2	-	-	81.7	18.3	-	-	-
Total %	0.1	0.1	-	0.2	8.6	45.7	-	54.3	37.2	8.3	-	45.5	-
Lights	2	1	-	3	115	707	-	822	591	96	-	687	1512
% Lights	100.0	100.0	-	100.0	80.4	93.0	-	91.0	95.6	69.6	-	90.9	91.0
Buses	0	0	-	0	28	47	-	75	21	42	-	63	138
% Buses	0.0	0.0	-	0.0	19.6	6.2	-	8.3	3.4	30.4	-	8.3	8.3
Trucks	0	0	-	0	0	6	-	6	6	0	-	6	12
% Trucks	0.0	0.0	-	0.0	0.0	0.8	-	0.7	1.0	0.0	-	0.8	0.7
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	2	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 2



Turning Movement Data Plot



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 jhudak@trafficpd.com

Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 3

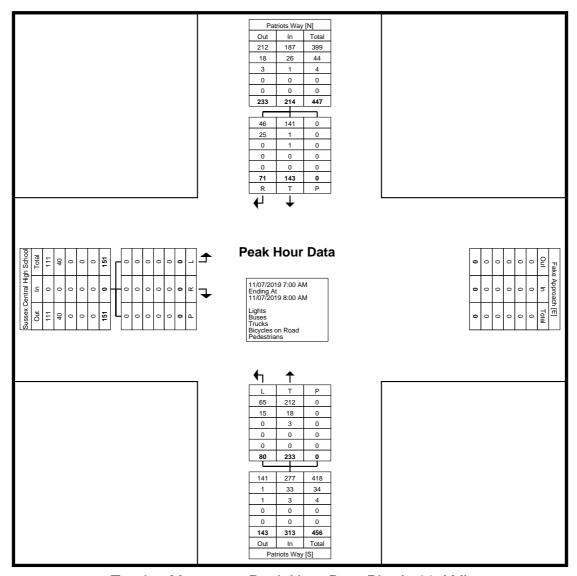
Turning Movement Peak Hour Data (7:00 AM)

	Sussex	Central High	School DW	_		Patrio	ts Way		'		s Way		
Start Time		Eastb	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	0	0	0	0	28	78	0	106	34	34	0	68	174
7:15 AM	0	0	0	0	32	90	0	122	50	27	0	77	199
7:30 AM	0	0	0	0	17	44	0	61	32	10	0	42	103
7:45 AM	0	0	0	0	3	21	0	24	27	0	0	27	51
Total	0	0	0	0	80	233	0	313	143	71	0	214	527
Approach %	0.0	0.0	-	-	25.6	74.4	-	-	66.8	33.2	-	-	-
Total %	0.0	0.0	-	0.0	15.2	44.2	-	59.4	27.1	13.5	-	40.6	-
PHF	0.000	0.000	-	0.000	0.625	0.647	-	0.641	0.715	0.522	-	0.695	0.662
Lights	0	0	-	0	65	212	-	277	141	46	-	187	464
% Lights	-	-	-	-	81.3	91.0	-	88.5	98.6	64.8	-	87.4	88.0
Buses	0	0	-	0	15	18	-	33	1	25	-	26	59
% Buses	-	-	-	-	18.8	7.7	-	10.5	0.7	35.2	-	12.1	11.2
Trucks	0	0	-	0	0	3	-	3	1	0	-	1	4
% Trucks	-	-	-	-	0.0	1.3	-	1.0	0.7	0.0	-	0.5	0.8
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	-	_	-	-	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	_	0	-	-	_	0	-	1	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 4



Turning Movement Peak Hour Data Plot (7:00 AM)



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Count Name: Patriots Way & Sussex Central High School DW (Center)
Site Code:
Start Date: 11/07/2019
Page No: 3

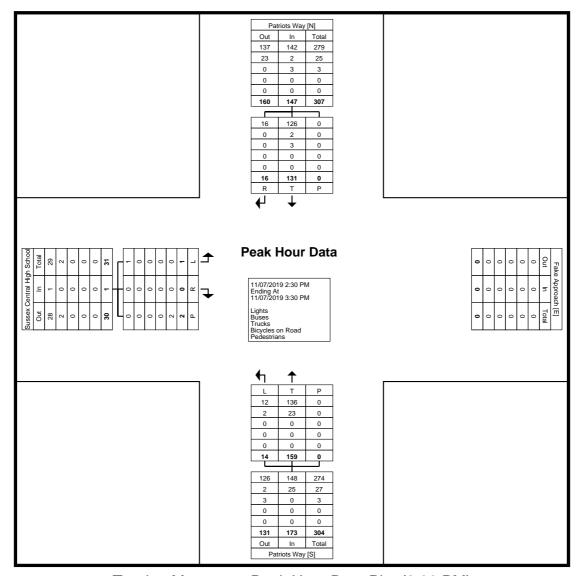
Turning Movement Peak Hour Data (2:30 PM)

				.9				~.~ (, o ,				ı
	Sussex	c Central High	School DW	(Center		Patriot	s Way			Patriot	s Way		
Start Time		Eastb	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	0	0	0	5	79	0	84	52	6	0	58	142
2:45 PM	1	0	0	1	5	19	0	24	25	4	0	29	54
3:00 PM	0	0	1	0	3	36	0	39	25	4	0	29	68
3:15 PM	0	0	1	0	1	25	0	26	29	2	0	31	57
Total	1	0	2	1	14	159	0	173	131	16	0	147	321
Approach %	100.0	0.0	-	-	8.1	91.9	-	-	89.1	10.9	-	-	-
Total %	0.3	0.0	-	0.3	4.4	49.5	-	53.9	40.8	5.0	-	45.8	-
PHF	0.250	0.000	-	0.250	0.700	0.503	-	0.515	0.630	0.667	-	0.634	0.565
Lights	1	0	-	1	12	136	-	148	126	16	-	142	291
% Lights	100.0	-	-	100.0	85.7	85.5	-	85.5	96.2	100.0	-	96.6	90.7
Buses	0	0	-	0	2	23	-	25	2	0	-	2	27
% Buses	0.0	-	-	0.0	14.3	14.5	-	14.5	1.5	0.0	-	1.4	8.4
Trucks	0	0	-	0	0	0	-	0	3	0	-	3	3
% Trucks	0.0	-	-	0.0	0.0	0.0	-	0.0	2.3	0.0	-	2.0	0.9
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	-	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	_	2	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-



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Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 4



Turning Movement Peak Hour Data Plot (2:30 PM)



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Count Name: Patriots Way & Sussex Central High School DW (Center) Site Code: Start Date: 11/07/2019 Page No: 5



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 1

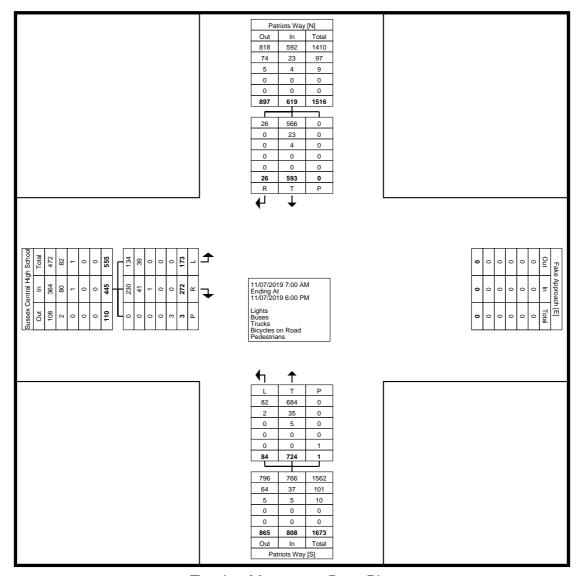
Turning Movement Data

	_			ΙŲ	irning l	Movem	nent Da	ata					
	Susse	x Central High	School DW	(South)		Patrio	ts Way			Patriot	s Way		
		Eastb	ound			North	bound			South	oound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	15	16	0	31	21	88	0	109	29	5	0	34	174
7:15 AM	18	39	0	57	40	105	1	145	41	9	0	50	252
7:30 AM	15	31	0	46	2	47	0	49	32	0	0	32	127
7:45 AM	2	4	0	6	2	19	0	21	28	0	0	28	55
Hourly Total	50	90	0	140	65	259	1	324	130	14	0	144	608
8:00 AM	3	4	0	7	1	23	0	24	16	2	0	18	49
8:15 AM	2	. 5	0	. 7	2	23	0	25	21	0	0	21	53
8:30 AM	3	3	0	6	2	18	0	20	16	1	0	17	43
8:45 AM	3	4	0	7	1	18	0	19	15	1	0	16	42
Hourly Total	11	16	0	27	6	82	0	88	68	4	0	72	187
*** BREAK ***	-	-	-	-	-	-	-	_	-	-	-	-	-
2:00 PM	4	6	0	10	4	35	0	39	12	1	0	13	62
2:15 PM	2	1	1	3	2	37	0	39	9	4	0	13	55
2:30 PM	57	66	0	123	0	27	0	27	52	0	0	52	202
2:45 PM	6	19	0	25	0	19	0	19	24	0	0	24	68
Hourly Total	69	92	1	161	6	118	0	124	97	5	0	102	387
3:00 PM	11	14	1	25	2	27	0	29	22	1	0	23	77
3:15 PM	4	10	1	14	1	22	0	23	30	0	0	30	67
3:30 PM	3	11	0	14	2	28	0	30	21	0	0	21	65
3:45 PM	6	9	0	15	0	25	0	25	29	0	0	29	69
Hourly Total	24	44	2	68	5	102	0	107	102	1	0	103	278
4:00 PM	5	9	0	14	1	32	0	33	28	0	0	28	75
4:15 PM	4	2	0	. 6	0	22	0	22	24	0	0	24	52
4:30 PM	1	2	0	3	1	26	0	27	32	0	0	32	62
4:45 PM	4	6	0	10	0	18	0	18	20	0	0	20	48
Hourly Total	14	19	0	33	2	98	0	100	104	0	0	104	237
5:00 PM	2	3	0	5	0	21	0	21	19	0	0	19	45
5:15 PM	1	3	0	4	0	15	0	15	31	2	0	33	52
5:30 PM	2	5	0	. 7	0	16	. 0	16	21	0	0	21	44
5:45 PM	0	0	0	0	0	13	0	13	21	0	0	21	34
Hourly Total	5	11	0	16	0	65	0	65	92	2	0	94	175
Grand Total	173	272	3	445	84	724	. 1	808	593	26	0	619	1872
Approach %	38.9	61.1	-	-	10.4	89.6	-	-	95.8	4.2	-	-	-
Total %	9.2	14.5	-	23.8	4.5	38.7	-	43.2	31.7	1.4	-	33.1	-
Lights	134	230	-	364	82	684	-	766	566	26	-	592	1722
% Lights	77.5	84.6	-	81.8	97.6	94.5	-	94.8	95.4	100.0	-	95.6	92.0
Buses	39	41	-	80	2	35	-	37	23	0	-	23	140
% Buses	22.5	15.1	-	18.0	2.4	4.8	-	4.6	3.9	0.0	-	3.7	7.5
Trucks	0	1	-	1	0	5	-	5	4	0	-	4	10
% Trucks	0.0	0.4	-	0.2	0.0	0.7	-	0.6	0.7	0.0	-	0.6	0.5
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	3	-	-	-	1	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	100.0	-	-	-	-	-	-



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 2



Turning Movement Data Plot



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 3

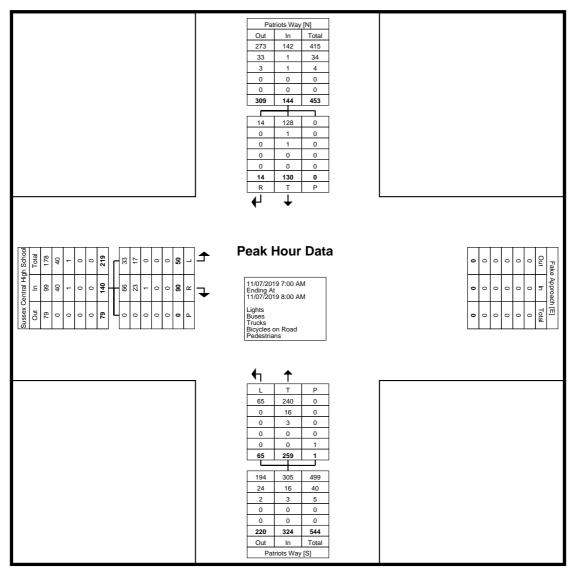
Turning Movement Peak Hour Data (7:00 AM)

			I GIIIII	ig iviove	,,,,,	Carri	ioui D	ala (1.c	, , , , , ,				
	Sussex	Central High	School DV	/ (South)		Patrio	ts Way			Patriot	s Way		
Start Time		Eastb	ound			North	bound			South	oound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	15	16	0	31	21	88	0	109	29	5	0	34	174
7:15 AM	18	39	0	57	40	105	1	145	41	9	0	50	252
7:30 AM	15	31	0	46	2	47	0	49	32	0	0	32	127
7:45 AM	2	4	0	6	2	19	0	21	28	0	0	28	55
Total	50	90	0	140	65	259	1	324	130	14	0	144	608
Approach %	35.7	64.3	-	-	20.1	79.9	-	-	90.3	9.7	-	-	-
Total %	8.2	14.8	-	23.0	10.7	42.6	-	53.3	21.4	2.3	-	23.7	-
PHF	0.694	0.577	-	0.614	0.406	0.617	-	0.559	0.793	0.389	-	0.720	0.603
Lights	33	66	-	99	65	240	-	305	128	14	-	142	546
% Lights	66.0	73.3	-	70.7	100.0	92.7	-	94.1	98.5	100.0	-	98.6	89.8
Buses	17	23	-	40	0	16	-	16	1	0	-	1	57
% Buses	34.0	25.6	-	28.6	0.0	6.2	-	4.9	0.8	0.0	-	0.7	9.4
Trucks	0	1	-	1	0	3	-	3	1	0	-	1	5
% Trucks	0.0	1.1	-	0.7	0.0	1.2	-	0.9	0.8	0.0	-	0.7	0.8
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	0	-	-	_	1	-	1	_	0	-	-
% Pedestrians	-	_	-	-	-	-	100.0	-	_	_	-	-	-



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 4



Turning Movement Peak Hour Data Plot (7:00 AM)



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 5

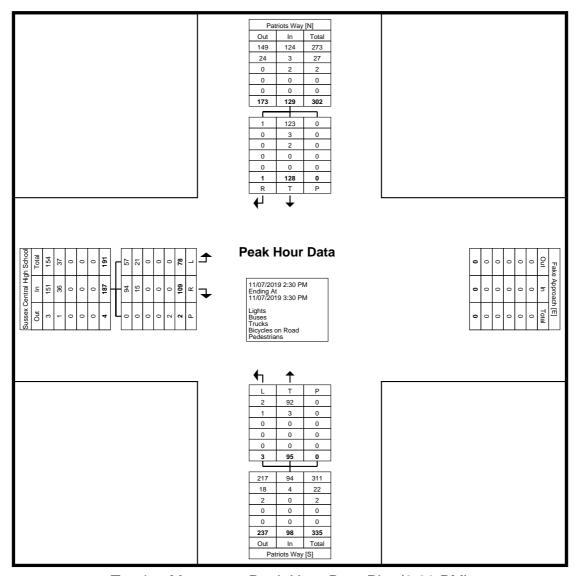
Turning Movement Peak Hour Data (2:30 PM)

				.9				~.~ (, o ,				
	Sussex	x Central High	School DW	(South)		Patriot	ts Way			Patriot	s Way		
Start Time		Eastb	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	57	66	0	123	0	27	0	27	52	0	0	52	202
2:45 PM	6	19	0	25	0	19	0	19	24	0	0	24	68
3:00 PM	11	14	1	25	2	27	0	29	22	1	0	23	77
3:15 PM	4	10	1	14	1	22	0	23	30	0	0	30	67
Total	78	109	2	187	3	95	0	98	128	1	0	129	414
Approach %	41.7	58.3	-	-	3.1	96.9	-	-	99.2	0.8	-	-	-
Total %	18.8	26.3	-	45.2	0.7	22.9	-	23.7	30.9	0.2	-	31.2	-
PHF	0.342	0.413	-	0.380	0.375	0.880	-	0.845	0.615	0.250	-	0.620	0.512
Lights	57	94	-	151	2	92	-	94	123	1	-	124	369
% Lights	73.1	86.2	-	80.7	66.7	96.8	-	95.9	96.1	100.0	-	96.1	89.1
Buses	21	15	-	36	1	3	-	4	3	0	-	3	43
% Buses	26.9	13.8	-	19.3	33.3	3.2	-	4.1	2.3	0.0	-	2.3	10.4
Trucks	0	0	-	0	0	0	-	0	2	0	-	2	2
% Trucks	0.0	0.0	-	0.0	0.0	0.0	-	0.0	1.6	0.0	-	1.6	0.5
Bicycles on Road	0	0	-	0	0	0	-	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	2	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-



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Count Name: Patriots Way & Sussex Central High School DW (South) Site Code: Start Date: 11/07/2019 Page No: 6



Turning Movement Peak Hour Data Plot (2:30 PM)

Appendix D VOLUME DEVELOPMENT INFORMATION

TPD# CDAE.00008 1/16/2020 Traffic Volumes Worksheet Intersection: Synchro Node:

Patriots Way & Existing High School Entrance (north)

1 Adjacent intersections: West 0 East 0 North 0 South 2

Time Period: Weekday A.M. Peak Hour

	E	Eastbour	nd	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	78		46				117	119			168	264	792
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	78	0	46	0	0	0	115	117	0	0	165	259	779
Base growth	0	0	0	0	0	0	0	2	0	0	2	0	4
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	
2022 Base Volumes	78	0	46	0	0	0	115	119	0	0	167	259	783
Staff Trips											70		70
Parent Trips								6			6		
Bus Trips								(17)			(18)		35
Total Trip Distribution	0	0	0	0	0	0	0	23	0	0	94	0	117
2022 Projected Volumes	78	0	46	0	0	0	115	142	0	0	261	259	900

Time Period: Weekday P.M. Peak Hour

	Е	astboun	ıd	V	Vestbou	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	130		79				17	144			65	18	453
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	130	0	79	0	0	0	17	141	0	0	64	18	448
Base growth	0	0	0	0	0	0	0	2	0	0	1	0	3
Dace growth	1 -	Ŭ	Ŭ	Ŭ	Ŭ	Ŭ	Ŭ						0
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	0
2022 Base Volumes	130	0	79	0	0	0	17	143	0	0	65	18	451
Staff Trips								68					68
Parent Trips								6					
Bus Trips								(18)					18
Total Trip Distribution	0	0	0	0	0	0	0	92	0	0	0	0	92
2022 Projected Volumes	130	0	79	0	0	0	17	235	0	0	65	18	543

TPD# CDAE.00008 1/16/2020 Traffic Volumes Worksheet Intersection: Synchro Node:

Patriots Way & Existing High School Entrance (Center)/Proposed Site Entrance

3 Adjacent intersections: West 0 East 0 North 0 South 2

Time Period: Weekday A.M. Peak Hour

	Е	astboun	d	V	/estboui	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	0		0				80	233			143	71	527
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	0	0	0	0	0	0	78	228	0	0	140	70	516
Base growth	0	0	0	0	0	0	0	3	0	0	2	0	5
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	
2022 Base Volumes	0	0	0	0	0	0	78	231	0	0	142	70	521
Staff Trips									51	70			121
Parent Trips				4		6			4	6			
Bus Trips				(11)		(17)			(12)	(18)			58
Total Trip Distribution	0	0	0	15	0	23	0	0	67	94	0	0	199
2022 Projected Volumes	0	0	0	15	0	23	78	231	67	94	142	70	720

Time Period: Weekday P.M. Peak Hour

	Е	astboun	ıd	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	Volume									
Existing Counts			0				14	159			131	16	320
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	0	0	0	0	0	0	14	156	0	0	128	16	314
Dana mandh		_		_		_	0	2	0		2		
Base growth)	U	U	U	U	0	U		U	0		U	0
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	0
2022 Base Volumes	0	0	0	0	0	0	14	158	0	0	130	16	318
Staff Trips				50		68							118
Parent Trips				4		6							
Bus Trips				(12)		(18)							30
Total Trip Distribution	0	0	0	66	0	92	0	0	0	0	0	0	158
2022 Projected Volumes	0	0	0	66	0	92	14	158	0	0	130	16	476

TPD# CDAE.00008 1/16/2020 Traffic Volumes Worksheet Intersection: Synchro Node:

Patriots Way & Existing High School Entrance (south)

3 Adjacent intersections: West 0 East 0 North 0 South 2

Time Period: Weekday A.M. Peak Hour

	Е	astboun	d	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	Volume									
Existing Counts	50		90				65	259			130	14	608
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	50	0	90	0	0	0	64	254	0	0	127	14	599
Base growth	0	0	0	0	0	0	0	4	0	0	2	0	6
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	
2022 Base Volumes	50	0	90	0	0	0	64	258	0	0	129	14	605
Staff Trips								51					51
Parent Trips								4			4		
Bus Trips								(12)			(11)		23
Total Trip Distribution	0	0	0	0	0	0	0	67	0	0	15	0	82
2022 Projected Volumes	50	0	90	0	0	0	64	325	0	0	144	14	687

Time Period: Weekday P.M. Peak Hour

	Е	astboun	ıd	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	Volume									
Existing Counts	78		109				3	95			128	1	414
Seasonal Adjustment Factor	1.00		1.00				0.98	0.98			0.98	0.98	6
Existing Volumes (Seasonally Adjusted)	78	0	109	0	0	0	3	93	0	0	125	1	409
Base growth	0	0	0	0	0	0	0	1	0	0	2	0	3
0)												0
Base growth (1% compounded for 2 years)	0	0	0	0	0	0	0	0	0	0	0	0	0
2022 Base Volumes	78	0	109	0	0	0	3	94	0	0	127	1	412
Staff Trips											50		50
Parent Trips											4		
Bus Trips											(12)		12
Total Trip Distribution	0	0	0	0	0	0	0	0	0	0	66	0	66
2022 Projected Volumes	78	0	109	0	0	0	3	94	0	0	193	1	478

15-MIN VOLUME DEVELOPMENT WORKSHEETS

																		Patriots	Way & Sus	sex Central	HS Driveway	(north)																
		E	xisting Cour	its					S	easonall	ly Adjuste	ed			Base				Base							New Trips						Proje	ected Volur	nes				
Time	East	bound	North	bound	South	bound	Time		Eastbound		Northbo	ound	South	bound	Growth	Time	Eastb	ound	North	bound	South	oound	Time	Easth	oound	Northbound	Southboun	j ,	me _	Eastbo	ound	Northb	ound	Southb	oound	To	tal	PHF
Tille	Left	Right	Left	Thru	Thru	Right		Le	eft Right	Le	eft	Thru	Thru	Right	1.0151		Left	Right	Left	Thru	Thru	Right		Left	Right	Left Thru	Thru Ri	ght		Left	Right	Left	Thru	Thru	Right	15 min.	Hour	
7:00	14	13	44	28	51	75	7:00	1	14 13	4	43	27	50	74		7:00	14	13	43	28	51	74	7:00			17	2	7	:00	14	13	43	45	53	74	242	901	0.633
7:15	30	22	62	32	56	146	7:15	3	30 22	6	51	31	55	143		7:15	30	22	61	32	56	143	7:15			0	12	7	:15	30	22	61	32	68	143	356	707	
7:30	29	9	10	38	32	37	7:30	2	29 9	1	10	37	31	36		7:30	29	9	10	38	31	36	7:30			0	0		:30	29	9	10	38	31	36	153	403	
7:45	5	2	1	21	29	6	7:45		5 2		1	21	28	6		7:45	5	2	1	21	28	6	7:45			6	81	7	:45	5	2	1	27	109	6	150	295	
8:00	3	0	6	13	21	5	8:00		3 0		6	13	21	5		8:00	3	0	6	13	21	5	8:00			0	0	8	:00	3	0	6	13	21	5	48	191	
8:15	0	0	5	19	19	9	8:15	(0 0		5	19	19	9		8:15	0	0	5	19	19	9	8:15			0	0	8	:15	0	0	5	19	19	9	52		
8:30	3	1	1	17	19	4	8:30	- 1	3 1		1	17	19	4		8:30	3	1	1	17	19	4	8:30			0	0	8	:30	3	1	1	17	19	4	45		
8:45	4	2	1	16	17	6	8:45	4	4 2		1	16	17	6		8:45	4	2	1	16	17	6	8:45			0	0	8	:45	4	2	1	16	17	6	46		
2:00	4	4	5	23	22	6	2:00	4	4 4		5	23	22	6		2:00	4	4	5	23	22	6	2:00			1	0		:00	4	4	5	24	22	6	65	454	
2:15	3	1	14	19	20	25	2:15		3 1		14	19	20	25		2:15	3	1	14	19	20	25	2:15			0	6		:15	3	1	14	19	26	25	88	474	
2:30	64	46	11	70	13	5	2:30	6		1	11	68	13	5		2:30	64	46	11	70	14	5	2:30			24	0		:30	64	46	11	94	14	5	234	544	0.581
2:45	17	12	0	19	14	5	2:45	1	.,		0	19	14	5		2:45	17	12	0	19	14	5	2:45			0	0		:45	17	12	0	19	14	5	67	376	
3:00	24	8	2	31	15	7	3:00	2		:	2	30	14	7		3:00	24	8	2	30	14	7	3:00			0	0		:00	24	8	2	30	14	7	85	406	
3:15	25	13	4	24	23	1	3:15		25 13		4	24	23	1		3:15	25	13	4	24	23	1	3:15			68	0		:15	25	13	4	92	23	1	158	391	
3:30	12	5	5	22	20	2	3:30	1			5	22	20	2		3:30	12	5	5	22	20	2	3:30			0	0		:30	12	5	5	22	20	2	66	308	
3:45	12	9	3	25	25	5	3:45	1	12 9		3	25	25	5		3:45	12	9	3	25	25	5	3:45			0	18		:45	12	9	3	25	43	5	97	318	
4:00	4	5	0	31	25	6	4:00	4	4 5		0	30	25	6		4:00	4	5	0	30	25	6	4:00			0	0		:00	4	5	0	30	25	6	70	269	
4:15	12	4	2	24	23	10	4:15	1	12 4	:	2	24	23	10		4:15	12	4	2	24	23	10	4:15			0	0		:15	12	4	2	24	23	10	75	250	
4:30	7	10	4	26	29	2	4:30	;	7 10		4	25	28	2	1	4:30	7	10	4	25	28	2	4:30			0	0		:30	7	10	4	25	28	2	76	227	└
4:45	13	0	1	13	19	2	4:45	1	13 0	:	1	13	19	2	1	4:45	13	0	1	13	19	2	4:45			0	0		:45	13	0	1	13	19	2	48	195	
5:00	3	1	4	24	19	0	5:00	1 1	3 1	- 4	4	24	19	0	1	5:00	3	1	4	24	19	0	5:00			0	0		:00	3	1	4	24	19	0	51	179	
5:15	0	0	0	15	35	2	5:15		0 0		0	15	34	2	1	5:15	0	0	0	15	35	2	5:15			0	0		:15	0	0	0	15	35	2	52		
5:30	1	0	0	19	24	0	5:30		1 0		0	19	24	0		5:30	1	0	0	19	24	0	5:30			0	0		:30	1	0	0	19	24	0	44		
5:45	2	0	0	13	17	0	5:45	1 2	2 0	- 1 (0	13	17	0	1	5:45	2	0	0	13	17	0	5:45			0	0	5	:45	2	0	0	13	17	0	32		1

AM Peak	78	46	115	142	261	259	901
PHF	0.650	0.523	0.471	0.789	0.599	0.453	0.633
PM Peak	130	79	17	235	65	18	544
PHF	0.508	0.429	0.386	0.625	0.707	0.643	0.581

																	Patriot	s Way & Sussex Cent	al HS Drive	vay (center)																
	E	kisting Cour	ıts			Sea	sonally Adj	usted		Base			Base						N	ew Trips									Projecte	d Volumes						j
Time	North	bound	South	bound	Time	North	nbound	So	outhbound	Growth	Time	Nort	nbound	Sout	nbound	Time		Westbound		Northbou	ınd		Southbound	Tim		Westbou	nd		Northboun	d		Southbound	1	To	tal	PHF
Tille	Left	Thru	Thru	Right	Tille	Left	Thru	Thru	ı Right	1.0151		Left	Thru	Thru	Right	IIIIIe	Left	Thru Righ	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	15 min.	Hour	1
7:00	28	78	34	34	7:00	27	76	33	34		7:00	27	77	34	34	7:00	11	17			2	1		7:00	11	0	17	27	77	2	1	34	34	203	720	0.833
7:15	32	90	50	27	7:15	31	88	49	26		7:15	31	89	50	26	7:15	0	0			8	12		7:15	0	0	0	31	89	8	12	50	26	216	566	
7:30	17	44	32	10	7:30	17	43	31	10		7:30	17	44	32	10	7:30	0	0			0	0		7:30	0	0	0	17	44	0	0	32	10	103	396	
7:45	3	21	27	0	7:45	3	21	26	0		7:45	3	21	26	0	7:45	4	6			57	81		7:45	4	0	6	3	21	57	81	26	0	198	335	1
8:00	5	21	18	5	8:00	5	21	18	5		8:00	5	21	18	5	8:00	0	0			0	0		8:00	0	0	0	5	21	0	0	18	5	49	174	
8:15	2	23	21	0	8:15	2	23	21	0		8:15	2	23	21	0	8:15	0	0			0	0		8:15	0	0	0	2	23	0	0	21	0	46		
8:30	5	17	17	3	8:30	5	17	17	3		8:30	5	17	17	3	8:30	0	0			0	0		8:30	0	0	0	5	17	0	0	17	3	42		
8:45	2	18	16	1	8:45	2	18	16	1		8:45	2	18	16	1	8:45	0	0			0	0		8:45	0	0	0	2	18	0	0	16	1	37		
2:00	9	32	11	11	2:00	9	31	11	11		2:00	9	31	11	11	2:00	0	1			0	0		2:00	0	0	1	9	31	0	0	11	11	63	368	1
2:15	9	30	14	9	2:15	9	29	14	9		2:15	9	29	14	9	2:15	0	0			4	6		2:15	0	0	0	9	29	4	6	14	9	71	373	
2:30	5	79	52	6	2:30	5	77	50	6		2:30	5	78	51	6	2:30	16	24			0	0		2:30	16	0	24	5	78	0	0	51	6	180	476	0.661
2:45	5	19	25	4	2:45	5	19	25	4		2:45	5	19	26	4	2:45	0	0			0	0		2:45	0	0	0	5	19	0	0	26	4	54	352	
3:00	3	36	25	4	3:00	3	35	25	4		3:00	3	36	25	4	3:00	0	0			0	0		3:00	0	0	0	3	36	0	0	25	4	68	389	
3:15	1	25	29	2	3:15	1	25	28	2		3:15	1	25	28	2	3:15	50	68			0	0		3:15	50	0	68	1	25	0	0	28	2	174	388	1
3:30	4	29	22	2	3:30	4	28	22	2		3:30	4	28	22	2	3:30	0	0			0	0		3:30		0	0	4	28	0	0	22	2	56	265	1
3:45	4	27	28	4	3:45	4	26	27	4		3:45	4	26	27	4	3:45	0	0			12	18		3:45	0	0	0	4	26	12	18	27	4	91	272	1
4:00	3	33	28	5	4:00	3	32	27	5		4:00	3	32	27	5	4:00	0	0			0	0		4:00	0	0	0	3	32	0	0	27	5	67	223	1
4:15	3	23	24	1	4:15	3	23	24	1		4:15	3	23	24	1	4:15	0	0			0	0		4:15	0	0	0	3	23	0	0	24	1	51	199	
4:30	0	27	33	5	4:30	0	26	32	5		4:30	0	26	32	5	4:30	0	0			0	0		4:30	0	0	0	0	26	0	0	32	5	63	198	1
4:45	2	20	20	0	4:45	2	20	20	0		4:45	2	20	20	0	4:45	0	0			0	0		4:45		0	0	2	20	0	0	20	0	42	175	1
5:00	0	22	19	2	5:00	0	22	19	2		5:00	0	22	19	2	5:00	0	0			0	0		5:00		0	0	0	22	0	0	19	2	43	165	1
5:15	1	14	34	2	5:15	1	14	33	2		5:15	1	14	33	2	5:15	0	0			0	0		5:15		0	0	1	14	0	0	33	2	50		
5:30	0	19	20	1	5:30	0	19	20	1		5:30	0	19	20	1	5:30	0	0			0	0		5:30		0	0	0	19	0	0	20	1	40		-
5:45	0	13	19	0	5:45	0	13	19	0		5:45	0	13	19	0	5:45	0	0	- 1	1	0	0	1	5:45	0	0	0	0	13	0	0	19	0	32		1

AM Peak	15	0	23	78	231	67	94	142	70	720
PHF	0.341		0.338	0.629	0.649	0.294	0.290	0.710	0.515	0.833
PM Peak	66	0	92	14	158	0	0	130	16	476
PHF	0.330		0.338	0.700	0.506			0.637	0.667	0.661

																	Patriots	Way & Suss	ex Central	HS Driveway	(south)															
		E	xisting Cour	nts					Sei	asonally A	djusted			Base				Base							New Trips					Proje	ected Volur	mes				
Time	East	bound	North	bound	South	hbound	Time		Eastbound	No	thbound	South	bound	Growth	Time	Eastb	ound	North	bound	South	oound	Time	Eastb	oound	Northbound	Southbound	Time	Eas	tbound	Northb	ound	Southb	ound	Tot	tal	PHF
Time	Left	Right	Left	Thru	Thru	Right	Time	Lef	ft Right	Left	Thru	Thru	Right	1.0151	Time	Left	Right	Left	Thru	Thru	Right	Time	Left	Right	Left Thru	Thru Rigi	it	Left	Right	Left	Thru	Thru	Right	15 min.	Hour	1
7:00	15	16	21	88	29	5	7:00	15	16	21	86	28	5		7:00	15	16	21	87	29	5	7:00			2	11	7:00	15	16	21	89	40	5	186	687	0.663
7:15	18	39	40	105	41	9	7:15	18	39	39	103	40	9		7:15	18	39	39	105	41	9	7:15			8	0	7:15	18	39	39	113	41	9	259	550	
7:30	15	31	2	47	32	0	7:30	15	31	2	46	31	0		7:30	15	31	2	47	32	0	7:30			0	0	7:30	15	31	2	47	32	0	127	344	
7:45	2	4	2	19	28	0	7:45	2	4	2	19	27	0		7:45	2	4	2	19	27	0	7:45			57	4	7:45	2	4	2	76	31	0	115	260	
8:00	3	4	1	23	16	2	8:00	3	4	1	23	16	2		8:00	3	4	1	23	16	2	8:00			0	0	8:00	3	4	1	23	16	2	49	187	
8:15	2	5	2	23	21	0	8:15	2	5	2	23	21	0		8:15	2	5	2	23	21	0	8:15			0	0	8:15	2	5	2	23	21	0	53		
8:30	3	3	2	18	16	1	8:30	3	3	2	18	16	1		8:30	3	3	2	18	16	1	8:30			0	0	8:30	3	3	2	18	16	1	43		
8:45	3	4	1	18	15	1	8:45	3	4	1	18	15	1		8:45	3	4	1	18	15	1	8:45			0	0	8:45	3	4	1	18	15	1	42		
2:00	4	6	4	35	12	1	2:00	4	6	4	34	12	1		2:00	4	6	4	35	12	1	2:00			0	0	2:00	4	6	4	35	12	1	62	407	
2:15	2	1	2	37	9	4	2:15	2	1	2	36	9	4		2:15	2	1	2	37	9	4	2:15			4	0	2:15	2	1	2	41	9	4	59	421	
2:30	57	66	0	27	52	0	2:30	57	7 66	0	26	51	0		2:30	57	66	0	27	52	0	2:30			0	16	2:30	57	66	0	27	68	0	218	478	0.548
2:45	6	19	0	19	24	0	2:45	6	19	0	19	24	0		2:45	6	19	0	19	24	0	2:45			0	0	2:45	6	19	0	19	24	0	68	324	1
3:00	11	14	2	27	22	1	3:00	11	14	2	26	22	1		3:00	11	14	2	26	22	1	3:00			0	0	3:00	11	14	2	26	22	1	76	336	
3:15	4	10	1	22	30	0	3:15	4	10	1	22	29	0		3:15	4	10	1	22	29	0	3:15			0	50	3:15	4	10	1	22	79	0	116	333	
3:30	3	11	2	28	21	0	3:30	3	11	2	27	21	0		3:30	3	11	2	27	21	0	3:30			0	0	3:30	3	11	2	27	21	0	64	269	
3:45	6	9	0	25	29	0	3:45	6	9	0	25	28	0		3:45	6	9	0	25	28	0	3:45			12	0	3:45	6	9	0	37	28	0	80	265	
4:00	5	9	1	32	28	0	4:00	5	9	1	31	27	0		4:00	5	9	1	31	27	0	4:00			0	0	4:00	5	9	1	31	27	0	73	233	
4:15	4	2	0	22	24	0	4:15	4	2	0	22	24	0		4:15	4	2	0	22	24	0	4:15			0	0	4:15	4	2	0	22	24	0	52	205	1
4:30	1	2	1	26	32	0	4:30	1	2	1	25	31	0	1	4:30	1	2	1	25	31	0	4:30			0	0	4:30	1	2	1	25	31	0	60	204	
4:45	4	6	0	18	20	0	4:45	4	6	0	18	20	0	1	4:45	4	6	0	18	20	0	4:45			0	0	4:45	4	6	0	18	20	0	48	188	
5:00	2	3	0	21	19	0	5:00	2	3	0	21	19	0	1	5:00	2	3	0	21	19	0	5:00			0	0	5:00	2	3	0	21	19	0	45	174	
5:15	1	3	0	15	31	2	5:15	1	3	0	15	30	2		5:15	1	3	0	15	30	2	5:15			0	0	5:15	1	3	0	15	30	2	51		
5:30	2	5	0	16	21	0	5:30	2	5	0	16	21	0		5:30	2	5	0	16	21	0	5:30			0	0	5:30	2	5	0	16	21	0	44		
5:45	0	0	0	13	21	0	5:45	0	0	0	13	21	0	1	5:45	0	0	0	13	21	0	5:45		1	0	0	5:45	0	0	0	13	21	0	34		1

AM Peak	50	90	64	325	144	14	687
PHF	0.694	0.577	0.410	0.719	0.878	0.389	0.663
PM Peak	78	109	3	94	193	1	478
PHF	0.342	0.413	0.375	0.870	0.611	0.250	0.548

Appendix E CAPACITY ANALYSES

EXISTING CONDITIONS

	٦	•	4	†	+	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		Ť		†	7
Traffic Volume (vph)	78	46	115	117	165	259
Future Volume (vph)	78	46	115	117	165	259
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	9	9	9	9
Grade (%)	0%			1%	0%	
Storage Length (ft)	0	0	120			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	725			447	479	
Travel Time (s)	19.8			12.2	13.1	
Peak Hour Factor	0.57	0.57	0.57	0.57	0.57	0.57
Heavy Vehicles (%)	0%	0%	0%	19%	16%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	7.4					
Movement	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			↑	↑	7
Traffic Vol, veh/h	78	46	115	117	165	259
Future Vol, veh/h	78	46	115	117	165	259
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	57	57	57	57	57	57
Heavy Vehicles, %	0	0	0	19	16	0
Mvmt Flow	137	81	202	205	289	454
IVIVIIIL I IUW	137	01	202	200	203	404
Major/Minor N	Minor2	N	//ajor1	N	Major2	
Conflicting Flow All	898	289	743	0	-	0
Stage 1	289	209	743	-	_	
•						-
Stage 2	609	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	312	755	873	-	-	-
Stage 1	765	-	-	-	-	-
Stage 2	547	-	-	-	-	-
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	240	755	873	_	_	_
Mov Cap-1 Maneuver	240	- 100	- 075	_	_	_
Stage 1	588	-	-	-	-	-
Stage 2	547	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	36.9		5.1		0	
HCM LOS	Е					
Minor Lane/Major Mvm	t	NBL	NRT	EBLn1	SBT	SBR
	•		-		- 301	אופט
Capacity (veh/h)		873				-
HCM Control Dalay (a)		0.231	-	0.678	-	-
HCM Control Delay (s)		10.4	-	36.9	-	-
HCM Lane LOS		В	-	Е	-	-
HCM 95th %tile Q(veh)		0.9	-	4.6	-	-

	•		_	•	ı	ر
		*	7	ı	*	₹
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		J.	†	†	7
Traffic Volume (vph)	0	0	78	228	140	70
Future Volume (vph)	0	0	78	228	140	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			1%	-1%	
Storage Length (ft)	0	0	235			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	427			549	447	
Travel Time (s)	11.6			15.0	12.2	
Confl. Peds. (#/hr)		1	1			
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles (%)	0%	0%	19%	9%	1%	36%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
	Other					
Area Type:						
Control Type: Unsignalize	u					

Intersection						
Int Delay, s/veh	1.3					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	•	7	↑	↑	7
Traffic Vol, veh/h	0	0	78	228	140	70
Future Vol, veh/h	0	0	78	228	140	70
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	235	-	-	210
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-1	-	-	1	-1	-
Peak Hour Factor	66	66	66	66	66	66
Heavy Vehicles, %	0	0	19	9	1	36
Mymt Flow	0	0	118	345	212	106
	J		. 10	010	_ L L	100
Major/Minor N	linor2	ı	Major1	N	/lajor2	
Conflicting Flow All	794	214	319	0	-	0
Stage 1	213	-	-	-	-	-
Stage 2	581	-	_	-	_	-
Critical Hdwy	6.2	6.1	4.29	_	_	-
Critical Hdwy Stg 1	5.2	-	0	_	_	_
Critical Hdwy Stg 2	5.2	_	_	_	_	_
Follow-up Hdwy	3.5		2.371			_
Pot Cap-1 Maneuver	376	836	1151	_	-	<u>-</u>
	837		1101	_		
Stage 1		-	-			-
Stage 2	582	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	337	835	1150	-	-	-
Mov Cap-2 Maneuver	337	-	-	-	-	-
Stage 1	750	-	-	-	-	-
Stage 2	581	-	-	-	-	-
J						
Approach	EB		NB		SB	
HCM Control Delay, s	0		2.2		0	
HCM LOS	Α					
Minor Long/Major Muset		NIDI	NDT	EBLn1	CDT	CDD
Minor Lane/Major Mvmt		NBL			SBT	SBR
Capacity (veh/h)		1150	-	-	-	-
HCM Lane V/C Ratio		0.103	-	-	-	-
HCM Control Delay (s)		8.5	-	0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0.3	-	-	-	-

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		Ť		f)	
Traffic Volume (vph)	50	90	64	254	127	14
Future Volume (vph)	50	90	64	254	127	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Peak Hour Factor	0.60	0.60	0.60	0.60	0.60	0.60
Heavy Vehicles (%)	34%	27%	0%	7%	2%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	5.5					
Movement	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		<u> </u>	↑	(
Traffic Vol, veh/h	50	90	64	254	127	14
Future Vol, veh/h	50	90	64	254	127	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	210	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	-1	-
Peak Hour Factor	60	60	60	60	60	60
Heavy Vehicles, %	34	27	0	7	2	0
Mymt Flow	83	150	107	423	212	23
	- 00	100	101	120	_ L L	20
Major/Minor	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	861	224	235	0	-	0
Stage 1	224	-	-	-	_	-
Stage 2	637	_	_	_	_	_
Critical Hdwy	6.54	6.37	4.1	_	_	_
Critical Hdwy Stg 1	5.54	0.57	4.1	_	_	_
	5.54					
Critical Hdwy Stg 2		2 5 4 2	-	-	-	-
Follow-up Hdwy		3.543	2.2	-	-	-
Pot Cap-1 Maneuver	302	762	1344	-	-	-
Stage 1	753	-	-	-	-	-
Stage 2	488	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	278	762	1344	-	-	-
Mov Cap-2 Maneuver	278	-	-	-	-	-
Stage 1	693	-	-	-	-	-
Stage 2	488	_	-	_	_	_
	,00					
Approach	EB		NB		SB	
HCM Control Delay, s	20		1.6		0	
HCM LOS	С					
A 41 1 10 4 1 5 1		NE	Not	-DL 4	057	000
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1344	-		-	-
HCM Lane V/C Ratio		0.079	-	0.496	-	-
HCM Control Delay (s)		7.9	-	20	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)	0.3	-	2.7	-	-
	,					

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		Ť			7
Traffic Volume (vph)	130	79	17	141	64	18
Future Volume (vph)	130	79	17	141	64	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	9	9	9	9
Grade (%)	0%			1%	0%	
Storage Length (ft)	0	0	120			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	725			447	479	
Travel Time (s)	19.8			12.2	13.1	
Confl. Peds. (#/hr)			2			2
Peak Hour Factor	0.54	0.54	0.54	0.54	0.54	0.54
Heavy Vehicles (%)	0%	0%	0%	16%	8%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	t					

Intersection						
Int Delay, s/veh	8.6					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	100	70	<u> </u>			7
Traffic Vol, veh/h	130	79	17	141	64	18
Future Vol, veh/h	130	79	17	141	64	18
Conflicting Peds, #/hr	0	0	_ 2	0	_ 0	_ 2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	54	54	54	54	54	54
Heavy Vehicles, %	0	0	0	16	8	0
Mvmt Flow	241	146	31	261	119	33
WWW.CT IOW		110	01	201	110	00
Major/Minor M	linor2	N	//ajor1	N	//ajor2	
Conflicting Flow All	444	121	154	0	-	0
Stage 1	121	-	-	-	-	-
Stage 2	323	_	_	_	_	_
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-		_	_	_
Critical Hdwy Stg 2	5.4		_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	-	_	
						-
Pot Cap-1 Maneuver	575	936	1439	-	-	-
Stage 1	909	-	-	-	-	-
Stage 2	738	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	560	934	1436	-	-	-
Mov Cap-2 Maneuver	560	-	-	-	-	-
Stage 1	887	-	-	-	-	-
Stage 2	737	-	-	-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	17.9		0.8		0	
HCM LOS	С					
NAC - 1		NDI	NET	EDL 4	OPT	000
Minor Lane/Major Mvmt		NBL	NRI	EBLn1	SBT	SBR
Capacity (veh/h)		1436	-		-	-
HCM Lane V/C Ratio		0.022		0.586		-
HCM Control Delay (s)		7.6	-	17.9	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)		0.1	-	3.8	-	-
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Existing Conditions
Timing Plan: PM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	LDIX	*	A	<u> </u>	ØDI€
Traffic Volume (vph)	0	0	14	156	128	16
Future Volume (vph)	0	0	14	156	128	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1300	12	9	9	9	9
Grade (%)	-1%	12	<u> </u>	1%	-1%	<u> </u>
Storage Length (ft)	0	0	235	1 /0	1 /0	210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			•
Link Speed (mph)	25			25	25	
Link Distance (ft)	427			549	447	
Travel Time (s)	11.6			15.0	12.2	
Confl. Peds. (#/hr)			1			1
Peak Hour Factor	0.57	0.57	0.57	0.57	0.57	0.57
Heavy Vehicles (%)	0%	0%	14%	15%	2%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary	•					
	Other					
Area Type:	Other					
Control Type: Unsignalize	a					

Existing Conditions
Timing Plan: PM Peak Hour

Intersection						
Int Delay, s/veh	0.4					
Movement	EDI	EDD	NDI	NBT	SBT	SBR
	EBL	EBR	NBL			
Lane Configurations	Y	•	<u>ነ</u>	450	100	7
Traffic Vol, veh/h	0	0	14	156	128	16
Future Vol, veh/h	0	0	14	156	128	16
Conflicting Peds, #/hr	0	0	_ 1	_ 0	_ 0	_ 1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	235	-	-	210
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-1	-	-	1	-1	-
Peak Hour Factor	57	57	57	57	57	57
Heavy Vehicles, %	0	0	14	15	2	0
Mymt Flow	0	0	25	274	225	28
			20		LLU	20
	linor2	1	Major1	N	/lajor2	
Conflicting Flow All	550	226	254	0	-	0
Stage 1	226	-	-	-	-	-
Stage 2	324	_	_	_	_	_
Critical Hdwy	6.2	6.1	4.24	_	_	_
Critical Hdwy Stg 1	5.2	-	1. L T	<u>-</u>	<u>-</u>	<u>-</u>
Critical Hdwy Stg 2	5.2	_		_	_	
Follow-up Hdwy	3.5		2.326	_	_	-
	515	824	1244	-		
Pot Cap-1 Maneuver		024	1244			
Stage 1	827	-	-	-	-	-
Stage 2	751	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	504	823	1243	-	-	-
Mov Cap-2 Maneuver	504	-	-	-	-	-
Stage 1	810	-	-	-	-	-
Stage 2	750	-	-	-	-	-
U						
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.7		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBL	NDT	EBLn1	SBT	SBR
				LOLIII	SDI	אמט
Capacity (veh/h)		1243	-	-	-	-
HCM Lane V/C Ratio		0.02	-	-	-	-
HCM Control Delay (s)		8	-	0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0.1	-	-	-	-

Existing Conditions
Timing Plan: PM Peak Hour

	•	•	•	†	 	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		7	*	f)	
Traffic Volume (vph)	78	109	3	93	125	1
Future Volume (vph)	78	109	3	93	125	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Confl. Peds. (#/hr)			2			2
Peak Hour Factor	0.51	0.51	0.51	0.51	0.51	0.51
Heavy Vehicles (%)	27%	14%	33%	3%	4%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	l					

Intersection						
Int Delay, s/veh	8.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	EBL	LDK				אמט
Lane Configurations	7 7	109	ኝ 3	93	1 →	1
Traffic Vol, veh/h						
Future Vol, veh/h	78 0	109	3	93	125 0	1 2
Conflicting Peds, #/hr		O Ctop			Free	
Sign Control RT Channelized	Stop -	Stop None	Free	Free	Free -	Free
	0	None -	210	None	-	None
Storage Length				-	_	
Veh in Median Storage	e, # 0 -1	-	-	-1	0	-
Grade, %		-	-		-1	-
Peak Hour Factor	51	51	51	51	51	51
Heavy Vehicles, %	27	14	33	3	4	0
Mvmt Flow	153	214	6	182	245	2
Major/Minor	Minor2		Major1	N	//ajor2	
Conflicting Flow All	442	248	249	0	-	0
Stage 1	248	240	243	-	_	-
Stage 2	194		_	_	_	
Critical Hdwy	6.47	6.24	4.43	<u>-</u>	<u>-</u>	
Critical Hdwy Stg 1	5.47	0.24	4.43	-	-	-
	5.47	-	-	-	-	-
Critical Hdwy Stg 2	3.743	3.426	2.497	-	-	-
Follow-up Hdwy		767		-	-	-
Pot Cap-1 Maneuver	542	101	1156	-	-	-
Stage 1	749	-	-	-	-	-
Stage 2	791	-	-	-	-	-
Platoon blocked, %	F0-	700	4454	-	-	-
Mov Cap-1 Maneuver	537	766	1154	-	-	-
Mov Cap-2 Maneuver	537	-	-	-	-	-
Stage 1	744	-	-	-	-	-
Stage 2	789	-	-	-	-	-
Approach	EB		NB		SB	
			0.3			
HCM Control Delay, s	17.5		0.3		0	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1154	-		-	-
HCM Lane V/C Ratio		0.005		0.564	_	-
HCM Control Delay (s)		8.1	-		-	-
HCM Lane LOS		A	_	С	_	_
HCM 95th %tile Q(veh)	0	_	3.5	_	_
TIOW JOHN JUNIO Q(VEI)	1	U		0.0		

2022 BASE CONDITIONS

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		7	†	†	7
Traffic Volume (vph)	78	46	115	119	167	259
Future Volume (vph)	78	46	115	119	167	259
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	9	9	9	9
Grade (%)	0%			1%	0%	
Storage Length (ft)	0	0	120			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	725			447	479	
Travel Time (s)	19.8			12.2	13.1	
Peak Hour Factor	0.57	0.57	0.57	0.57	0.57	0.57
Heavy Vehicles (%)	2%	2%	0%	19%	16%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	7.6					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	40	ች	↑	↑	7
Traffic Vol, veh/h	78	46	115	119	167	259
Future Vol, veh/h	78	46	115	119	167	259
Conflicting Peds, #/hr	0	0	_ 0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	57	57	57	57	57	57
Heavy Vehicles, %	2	2	0	19	16	0
Mvmt Flow	137	81	202	209	293	454
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	906	293	747	0	-	0
Stage 1	293	-	-	-	-	-
Stage 2	613	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.1	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	_	-
Critical Hdwy Stg 2	5.42	-	_	-	_	-
Follow-up Hdwy		3.318	2.2	_	_	_
Pot Cap-1 Maneuver	307	746	870	_	_	_
Stage 1	757	-	-	_	_	_
Stage 2	541	_	_	_	_	_
Platoon blocked, %	341	-	-	-		
	236	746	870	-	-	-
Mov Cap-1 Maneuver			0/0	-	-	-
Mov Cap-2 Maneuver	236	-	-	-	-	-
Stage 1	581	-	-	-	-	-
Stage 2	541	-	-	-	_	-
Approach	EB		NB		SB	
			5.1		0	
HCM Control Delay, s	38.2		J. I		U	
HCM LOS	E					
Minor Lane/Major Mvn	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		870	-			-
HCM Lane V/C Ratio		0.232		0.688		_
HCM Control Delay (s)	\	10.4	_	38.2		_
HCM Lane LOS		В	_	30.2 E	_	-
HCM 95th %tile Q(veh		0.9	_	4.8		
HOW SOUL WILLE MICHAIL)	0.9	-	4.0	-	-

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		ሻ	^	†	7
Traffic Volume (vph)	0	0	78	231	142	70
Future Volume (vph)	0	0	78	231	142	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			1%	-1%	
Storage Length (ft)	0	0	235			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	427			549	447	
Travel Time (s)	11.6			15.0	12.2	
Confl. Peds. (#/hr)		1	1			
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles (%)	0%	0%	19%	9%	1%	36%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					

Lanes, Volumes, Timings c:\pwworking\projectwise\slynch\d0651718\BAM.syn

Intersection						
Int Delay, s/veh	1.2					
	ED!	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	•	<u> </u>	↑	↑	7
Traffic Vol, veh/h	0	0	78	231	142	70
Future Vol, veh/h	0	0	78	231	142	70
Conflicting Peds, #/hr	0	1	1	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	235	-	-	210
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-1	-	-	1	-1	-
Peak Hour Factor	66	66	66	66	66	66
Heavy Vehicles, %	0	0	19	9	1	36
Mvmt Flow	0	0	118	350	215	106
			. 10	000	210	100
Major/Minor M	inor2	ı	Major1	N	//ajor2	
Conflicting Flow All	802	217	322	0	-	0
Stage 1	216	-	-	-	-	-
Stage 2	586	-	_	-	_	-
Critical Hdwy	6.2	6.1	4.29	_	_	-
Critical Hdwy Stg 1	5.2	-	0	_	_	_
Critical Hdwy Stg 2	5.2	_	_	_	_	_
Follow-up Hdwy	3.5		2.371		_	_
Pot Cap-1 Maneuver	372	833	1148	_		<u>-</u>
	835		1140			
Stage 1		-	-	-	-	-
Stage 2	579	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	333	832	1147	-	-	-
Mov Cap-2 Maneuver	333	-	-	-	-	-
Stage 1	748	-	-	-	-	-
Stage 2	578	-	-	-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		2.1		0	
HCM LOS	Α					
NA' I /NA . ' NA . '		ND	Not	EDL 4	OPT	000
Minor Lane/Major Mvmt		NBL	NBL	EBLn1	SBT	SBR
Capacity (veh/h)		1147	-	-	-	-
HCM Lane V/C Ratio		0.103	-	-	-	-
HCM Control Delay (s)		8.5	-	0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0.3	-	_	_	_
		3.5				

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		7	†	f)	
Traffic Volume (vph)	50	90	64	258	129	14
Future Volume (vph)	50	90	64	258	129	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Peak Hour Factor	0.60	0.60	0.60	0.60	0.60	0.60
Heavy Vehicles (%)	34%	27%	0%	7%	2%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	5.5					
Movement	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	^^	`	↑	♣	
Traffic Vol, veh/h	50	90	64	258	129	14
Future Vol, veh/h	50	90	64	258	129	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	210	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	-1	-
Peak Hour Factor	60	60	60	60	60	60
Heavy Vehicles, %	34	27	0	7	2	0
Mvmt Flow	83	150	107	430	215	23
	- 00		. • ,	.00		
	Minor2	N	Major1	١	/lajor2	
Conflicting Flow All	871	227	238	0	-	0
Stage 1	227	-	-	-	-	-
Stage 2	644	-	-	_	_	_
Critical Hdwy	6.54	6.37	4.1	_	_	-
Critical Hdwy Stg 1	5.54	-	-	_	_	_
Critical Hdwy Stg 2	5.54	_		_	_	
Follow-up Hdwy		3.543	2.2	_	_	_
	297	759	1341	-	-	
Pot Cap-1 Maneuver		139	1341			-
Stage 1	750	-	-	-	-	-
Stage 2	484	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	273	759	1341	-	-	-
Mov Cap-2 Maneuver	273	-	-	-	-	-
Stage 1	690	-	-	-	-	-
Stage 2	484	-	-	-	-	-
J						
					0.5	
Approach	EB		NB		SB	
HCM Control Delay, s	20.3		1.6		0	
HCM LOS	С					
Minor Lane/Major Mvn	nt	NBL	MRT	EBLn1	SBT	SBR
	IL					אמט
Capacity (veh/h)		1341	-		-	-
HCM Lane V/C Ratio		0.08	-	0.503	-	-
HCM Control Delay (s)		7.9	-	20.3	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh		0.3	-	2.8	-	-

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	***		7	†	†	7	
Traffic Volume (vph)	130	79	17	143	65	18	
Future Volume (vph)	130	79	17	143	65	18	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	9	9	9	9	
Grade (%)	0%			1%	0%		
Storage Length (ft)	0	0	120			210	
Storage Lanes	1	0	1			1	
Taper Length (ft)	25		25				
Link Speed (mph)	25			25	25		
Link Distance (ft)	725			447	479		
Travel Time (s)	19.8			12.2	13.1		
Confl. Peds. (#/hr)			2			2	
Peak Hour Factor	0.54	0.54	0.54	0.54	0.54	0.54	
Heavy Vehicles (%)	0%	0%	0%	16%	8%	0%	
Shared Lane Traffic (%)							
Sign Control	Stop			Free	Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	d						

Intersection						
Int Delay, s/veh	8.7					
•		EDD	ND	NET	OPT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**			↑		7
Traffic Vol, veh/h	130	79	17	143	65	18
Future Vol, veh/h	130	79	17	143	65	18
Conflicting Peds, #/hr	0	0	2	0	0	2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	54	54	54	54	54	54
Heavy Vehicles, %	0	0	0	16	8	0
Mvmt Flow	241	146	31	265	120	33
NA - : /NA:	Alia e C		1-:- 4		4-i- C	
	/linor2		//ajor1		/lajor2	
Conflicting Flow All	449	122	155	0	-	0
Stage 1	122	-	-	-	-	-
Stage 2	327	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	571	935	1438	-	-	-
Stage 1	908	-	-	-	-	-
Stage 2	735	_	-	-	-	-
Platoon blocked, %				-	_	-
Mov Cap-1 Maneuver	556	933	1435	-	-	-
Mov Cap-2 Maneuver	556	-	00	_	_	_
Stage 1	886	_	_	_	_	_
Stage 2	734	_	_	_	_	_
Glaye Z	1 34	_	_	_	_	_
Approach	EB		NB		SB	
HCM Control Delay, s	18.1		0.8		0	
HCM LOS	С					
3 200						
Minor Lane/Major Mvm		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1435	-	656	-	-
HCM Lane V/C Ratio		0.022	-	0.59	-	-
HCM Control Delay (s)		7.6	-	18.1	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)		0.1	-	3.9	-	-
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		*	7	ı	*	~
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		J.	†	†	7
Traffic Volume (vph)	0	0	14	158	130	16
Future Volume (vph)	0	0	14	158	130	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			1%	-1%	
Storage Length (ft)	0	0	235			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	427			549	447	
Travel Time (s)	11.6			15.0	12.2	
Confl. Peds. (#/hr)			1			1
Peak Hour Factor	0.57	0.57	0.57	0.57	0.57	0.57
Heavy Vehicles (%)	0%	0%	14%	15%	2%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
•	Othor					
Area Type:	Other					
Control Type: Unsignalize	u					

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	LDIX	ሻ	<u>↑</u>	<u> </u>	T T
Traffic Vol, veh/h	0	0	14	158	130	16
			14			16
Future Vol, veh/h	0	0		158	130	
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	235	-	-	210
Veh in Median Storage,		-	-	0	0	-
Grade, %	-1	-	-	1	-1	-
Peak Hour Factor	57	57	57	57	57	57
Heavy Vehicles, %	0	0	14	15	2	0
Mymt Flow	0	0	25	277	228	28
WWITHER TOW	U	U	20	211	220	20
Major/Minor Mi	nor2		Major1	N	/lajor2	
Conflicting Flow All	556	229	257	0	-	0
Stage 1	229	-		-	-	-
Stage 2	327	_	_	_	_	_
Critical Hdwy	6.2	6.1	4.24	_	_	_
Critical Hdwy Stg 1	5.2	-	7.27	_	<u>-</u>	_
			_			
Critical Hdwy Stg 2	5.2	-	-	-	-	-
Follow-up Hdwy	3.5		2.326	-	-	-
Pot Cap-1 Maneuver	511	820	1241	-	-	-
Stage 1	824	-	-	-	-	-
Stage 2	749	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	500	819	1240	-	-	-
Mov Cap-2 Maneuver	500	-	_	-	_	-
Stage 1	807	_	_	_	_	_
Stage 2	748	_	_	<u>-</u>	<u>-</u>	_
Glaye Z	7-10	-	_	_	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.6		0	
HCM LOS	A					
	, ,					
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1240	-	-	-	-
HCM Lane V/C Ratio		0.02	-	-	_	-
HCM Control Delay (s)		8	_	0	-	-
HCM Lane LOS		A	-	A	_	_
HCM 95th %tile Q(veh)		0.1	_	-		_
HOW JOHN JOHN (VEII)		0.1	_		_	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		*	A	1	
Traffic Volume (vph)	78	109	3	94	127	1
Future Volume (vph)	78	109	3	94	127	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Confl. Peds. (#/hr)			2			2
Peak Hour Factor	0.51	0.51	0.51	0.51	0.51	0.51
Heavy Vehicles (%)	27%	14%	33%	3%	4%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection						
Int Delay, s/veh	8.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	₩.	LDK				אמט
Lane Configurations		100	\frac{1}{2}	↑	127	1
Traffic Vol, veh/h	78	109	3	94	127	1
Future Vol, veh/h	78	109	3	94	127	1
Conflicting Peds, #/hr	0	0	2	0	0	2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	210	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	-1	-1	-
Peak Hour Factor	51	51	51	51	51	51
Heavy Vehicles, %	27	14	33	3	4	0
Mvmt Flow	153	214	6	184	249	2
Major/Minor	Minor2		Major1		/lajor2	
						^
Conflicting Flow All	448	252	253	0	-	0
Stage 1	252	-	-	-	-	-
Stage 2	196	-	-	-	-	-
Critical Hdwy	6.47	6.24	4.43	-	-	-
Critical Hdwy Stg 1	5.47	-	-	-	-	-
Critical Hdwy Stg 2	5.47	-	-	-	-	-
Follow-up Hdwy	3.743	3.426		-	-	-
Pot Cap-1 Maneuver	538	764	1151	-	-	-
Stage 1	745	-	-	-	-	-
Stage 2	789	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	533	763	1149	-	_	_
Mov Cap-2 Maneuver		-	-	-	_	_
Stage 1	740	_	-	_	_	_
Stage 2	787	_	_	_	_	_
Glaye Z	707	_	_	_	-	_
Approach	EB		NB		SB	
HCM Control Delay, s	17.6		0.3		0	
HCM LOS	С					
, = 0 0						
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1149	-	647	-	-
HCM Lane V/C Ratio		0.005	-	0.567	-	-
HCM Control Delay (s)	8.1	-	17.6	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh	1)	0	-	3.6	-	-
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2022 PROJECTED CONDITIONS

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M		7	†	†	7
Traffic Volume (vph)	78	46	115	142	261	259
Future Volume (vph)	78	46	115	142	261	259
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	9	9	9	9
Grade (%)	0%			1%	0%	
Storage Length (ft)	0	0	120			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	725			447	479	
Travel Time (s)	19.8			12.2	13.1	
Peak Hour Factor	0.63	0.63	0.63	0.63	0.63	0.63
Heavy Vehicles (%)	0%	0%	0%	19%	16%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	7.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ሻ	†	†	7
Traffic Vol, veh/h	78	46	115	142	261	259
Future Vol, veh/h	78	46	115	142	261	259
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	63	63	63	63	63	63
Heavy Vehicles, %	0	0	0	19	16	0
Mvmt Flow	124	73	183	225	414	411
Major/Minor	Minor2	N	Major1	N	Major2	
Conflicting Flow All	1005	414	825	0	-	0
Stage 1	414	-	-	_	_	-
Stage 2	591	_	_	_	_	_
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-		_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	-
Pot Cap-1 Maneuver	270	643	814	_	-	_
Stage 1	671	-	-	-	_	-
Stage 2	557	-	-	-	-	-
Platoon blocked, %				-	_	-
Mov Cap-1 Maneuver	209	643	814	-	-	-
Mov Cap-2 Maneuver	209	-	-	-	-	-
Stage 1	520	-	-	-	-	-
Stage 2	557	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	43.8		4.8		0	
HCM LOS	+5.0 E		7.0		U	
I IOIVI LOO						

Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT	SBR	
Capacity (veh/h)	814	- 279	-	-	
HCM Lane V/C Ratio	0.224	- 0.705	-	-	
HCM Control Delay (s)	10.7	- 43.8	-	-	
HCM Lane LOS	В	- E	-	-	
HCM 95th %tile Q(veh)	0.9	- 4.9	-	-	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		†			4		Ţ	f)		Ť		7
Traffic Volume (vph)	0	0	0	15	0	23	78	231	67	94	142	70
Future Volume (vph)	0	0	0	15	0	23	78	231	67	94	142	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	9	9	12	12	9	9
Grade (%)		-1%			0%			1%			-1%	
Storage Length (ft)	0		0	0		0	235		0	225		210
Storage Lanes	0		0	0		0	1		0	1		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		427			556			549			447	
Travel Time (s)		11.6			15.2			15.0			12.2	
Confl. Peds. (#/hr)			1				1					
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	0%	2%	0%	73%	2%	74%	19%	9%	18%	19%	1%	36%
Shared Lane Traffic (%)												
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Other

Area Type:
Control Type: Unsignalized

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		†			4			ĵ.		*	†	7
Traffic Vol, veh/h	0	0	0	15	0	23	78	231	67	94	142	70
Future Vol, veh/h	0	0	0	15	0	23	78	231	67	94	142	70
Conflicting Peds, #/hr	0	0	1	0	0	0	1	0	0	0	0	0
	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	235	_	-	225	_	210
Veh in Median Storage, #	# -	0	_	_	0	_		0	_		0	
Grade, %	_	-1	_	_	0	_	_	1	_	_	-1	_
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	0	2	0	73	2	74	19	9	18	19	1	36
Mvmt Flow	0	0	0	18	0	28	94	278	81	113	171	84
	•			.,								
Major/Minor Mi	nor2		N	Minor1			Major1			Major2		
		945			989			0			0	^
Conflicting Flow All	-		-	946		319	256	0	0	359	0	0
Stage 1	-	398	-	507	507	-	-	-	-	-	-	-
Stage 2	-	547	-	439	482	6.04	4.00	-	-	4.00	-	-
Critical Hdwy	-	6.32	-	7.83	6.52	6.94	4.29	-	-	4.29	-	-
Critical Hdwy Stg 1	-	5.32	-	6.83	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	5.32	-	6.83	5.52	2.000	0.074	-	-	- 0.274	-	-
Follow-up Hdwy		1.0.0		4.157	4.018	3.966	2.371	-	-	2.371	-	-
Pot Cap-1 Maneuver	0	276	0	182	247	582	1216	-	-	1111	-	-
Stage 1	0	616	0	437	539	-	-	-	-	-	-	-
Stage 2	0	533	0	480	553	-	-	-	-	-	-	-
Platoon blocked, %		000		450	005	F00	1015	-	-	1111	-	-
Mov Cap-1 Maneuver	-	229	-	158	205	582	1215	-	-	1111	-	-
Mov Cap-2 Maneuver	-	229	-	158	205	-	-	-	-	-	-	-
Stage 1	-	553	-	403	497	-	-	-	-	-	-	-
Stage 2	-	492	-	431	496	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			20.2			1.7			2.6		
HCM LOS	Α			С								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1215	_	-	_	283	1111	-	-			
HCM Lane V/C Ratio		0.077	_	_	_	0.162		_	-			
HCM Control Delay (s)		8.2	-	-	0	20.2	8.6	-	-			
HCM Lane LOS		A	-	-	A	С	A	-	-			
HCM 95th %tile Q(veh)		0.3	_	-	-	0.6	0.3	_	_			
2000 2(1311)												

	٦	•	4	†	+	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	***			+	- ↑	
Traffic Volume (vph)	50	90	64	325	144	14
Future Volume (vph)	50	90	64	325	144	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles (%)	34%	27%	0%	7%	2%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other	_			_	_

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	4.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ኘ	†	ĵ.	02.1
Traffic Vol, veh/h	50	90	64	325	144	14
Future Vol, veh/h	50	90	64	325	144	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None		None
Storage Length	0	-	210	-	_	-
Veh in Median Storage		_	-	0	0	_
Grade, %	,, # 0 -1	<u>-</u>	_	-1	-1	_
Peak Hour Factor	66	66	66	66	66	66
	34	27			2	0
Heavy Vehicles, %	76	136	0	7	218	21
Mvmt Flow	76	130	97	492	718	21
Major/Minor	Minor2	N	Major1	N	Major2	
Conflicting Flow All	915	229	239	0	-	0
Stage 1	229	-	-	-	-	-
Stage 2	686	_	_	-	_	-
Critical Hdwy	6.54	6.37	4.1	-	_	-
Critical Hdwy Stg 1	5.54	-	-	-	-	-
Critical Hdwy Stg 2	5.54	_	_	-	_	_
Follow-up Hdwy	3.806	3.543	2.2	_	_	_
Pot Cap-1 Maneuver	280	757	1340	_	_	_
Stage 1	749	101	-1040	_	_	_
Stage 2	463		_		-	_
Platoon blocked, %	403	-	-	-	<u>-</u>	-
Mov Cap-1 Maneuver	260	757	1340	-		
		131	1340	-	-	-
Mov Cap-2 Maneuver	260	-	-	-	-	-
Stage 1	695	-	-	-	-	-
Stage 2	463	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	19.9		1.3		0	
HCM LOS	C		1.0			
1 TOWN LOO	J					
Minor Lane/Major Mvm	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1340	-	450	-	-
HCM Lane V/C Ratio		0.072	-	0.471	-	-
HCM Control Delay (s)		7.9	-	19.9	-	-
				0		
HCM Lane LOS		Α	-	С	-	-

	۶	•	•	<u>†</u>	 	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		7			7
Traffic Volume (vph)	130	79	17	235	65	18
Future Volume (vph)	130	79	17	235	65	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	9	9	9	9
Grade (%)	0%			1%	0%	
Storage Length (ft)	0	0	120			210
Storage Lanes	1	0	1			1
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	725			447	479	
Travel Time (s)	19.8			12.2	13.1	
Confl. Peds. (#/hr)			2			2
Peak Hour Factor	0.58	0.58	0.58	0.58	0.58	0.58
Heavy Vehicles (%)	0%	0%	0%	16%	8%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					

Intersection						
Int Delay, s/veh	8.2					
Movement		EDD	NDL	NBT	CDT	CDD
	EBL	EBR	NBL		SBT	SBR
Lane Configurations	420	70	<u>ነ</u>	^	↑	7
Traffic Vol, veh/h	130	79	17	235	65	18
Future Vol, veh/h	130	79	17	235	65	18
Conflicting Peds, #/hr	0	0	_ 2	0	_ 0	_ 2
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	120	-	-	210
Veh in Median Storage,	,# 0	-	-	0	0	-
Grade, %	0	-	-	1	0	-
Peak Hour Factor	58	58	58	58	58	58
Heavy Vehicles, %	0	0	0	16	8	0
Mvmt Flow	224	136	29	405	112	31
		.00		.00	. 12	01
	/linor2		/lajor1		/lajor2	
Conflicting Flow All	577	114	145	0	-	0
Stage 1	114	-	-	-	-	-
Stage 2	463	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	_	_	-
Critical Hdwy Stg 1	5.4	-	_	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	482	944	1450	_		_
		344	1400			
Stage 1	916	-	-	-	-	-
Stage 2	638	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	470	942	1447	-	-	-
Mov Cap-2 Maneuver	470			-	-	-
Stage 1	896	-	-	-	-	-
Stage 2	637	-	-	-	-	-
g -	- • .					
Approach	EB		NB		SB	
HCM Control Delay, s	20.8		0.5		0	
HCM LOS	С					
Minor Long/Maior M		NDI	NDT	TDI1	CDT	CDD
Minor Lane/Major Mvm		NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1447	-	580	-	-
HCM Lane V/C Ratio		0.02	-	0.621	-	-
HCM Control Delay (s)		7.5	-	20.8	-	-
HCM Lane LOS		Α	-	С	-	-
HCM 95th %tile Q(veh)		0.1	-	4.3	-	-

	•	→	•	•	+	•	•	†	/	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		†			4		*	ĵ.		*	†	7
Traffic Volume (vph)	0	0	0	66	0	92	14	158	0	0	130	16
Future Volume (vph)	0	0	0	66	0	92	14	158	0	0	130	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	9	9	12	12	9	9
Grade (%)		-1%			0%			1%			-1%	
Storage Length (ft)	0		0	0		0	235		0	225		210
Storage Lanes	0		0	0		0	1		0	1		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		427			545			549			447	
Travel Time (s)		11.6			14.9			15.0			12.2	
Confl. Peds. (#/hr)							1					1
Peak Hour Factor	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66	0.66
Heavy Vehicles (%)	0%	2%	0%	18%	2%	20%	14%	15%	2%	2%	2%	0%
Shared Lane Traffic (%)												
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Other

Area Type:
Control Type: Unsignalized

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑			4		*	î,		*	†	7
Traffic Vol, veh/h	0	0	0	66	0	92	14	158	0	0	130	16
Future Vol, veh/h	0	0	0	66	0	92	14	158	0	0	130	16
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	0	0	0	1
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	_	None	_	-	None
Storage Length	_	-	_	-	_	-	235	-	_	225	-	210
Veh in Median Storage,	# -	0	_	-	0	_	_	0	_	_	0	-
Grade, %	_	-1	_	-	0	-	-	1	_	_	-1	_
Peak Hour Factor	66	66	66	66	66	66	66	66	66	66	66	66
Heavy Vehicles, %	0	2	0	18	2	20	14	15	2	2	2	0
Mymt Flow	0	0	0	100	0	139	21	239	0	0	197	24
Major/Minor	1inor2			Minor1			Major1			Major		
		470		Minor1	F00		Major1	^		Major2	^	^
Conflicting Flow All	-	479	-	490	503	239	222	0	0	239	0	0
Stage 1	-	198	-	281	281	-	-	-	-	-	-	-
Stage 2	-	281	-	209	222	- 0.4	4.04	-	-	4.40	-	-
Critical Hdwy	-	6.32	-	7.28	6.52	6.4	4.24	-	-	4.12	-	-
Critical Hdwy Stg 1	-	5.32	-	6.28	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	5.32	-	6.28	5.52	- 10	-	-	-	-	-	-
Follow-up Hdwy	-	4.018	-	3.662	4.018	3.48	2.326	-		2.218	-	-
Pot Cap-1 Maneuver	0	499	0	463	471	758	1279	-	-	1328	-	-
Stage 1	0	745	0	692	678	-	-	-	-	-	-	-
Stage 2	0	689	0	758	720	-	-	-	-	-	-	-
Platoon blocked, %		404		157	400	750	1070	-	-	1200	-	-
Mov Cap-1 Maneuver	-	491	-	457	463	758	1278	-	-	1328	-	-
Mov Cap-2 Maneuver	-	491	-	457	463	-	-	-	-	-	-	-
Stage 1	-	744	-	681	667	-	-	-	-	-	-	-
Stage 2	-	678	-	758	719	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			15.1			0.6			0		
HCM LOS	Α			С								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1278	-			594	1328					
HCM Lane V/C Ratio		0.017	_	_	_	0.403	-	_	_			
HCM Control Delay (s)		7.9	_	_	0	15.1	0	_	_			
HCM Lane LOS		Α	_	_	A	C	A	_	_			
HCM 95th %tile Q(veh)		0.1	_	_	-	1.9	0	_	_			
		J. 1				1.0						

	•	•	•	†	. ↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
		EDR	INDL	INDI		SDR
Lane Configurations	Y		<u>ግ</u>	<u> 1</u>	(Î	
Traffic Volume (vph)	78	109	3	94	193	1
Future Volume (vph)	78	109	3	94	193	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	9	9	9	9
Grade (%)	-1%			-1%	-1%	
Storage Length (ft)	0	0	210			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	578			700	549	
Travel Time (s)	15.8			19.1	15.0	
Confl. Peds. (#/hr)			2			2
Peak Hour Factor	0.55	0.55	0.55	0.55	0.55	0.55
Heavy Vehicles (%)	27%	14%	33%	3%	4%	0%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
	'					
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\slynch\d0651718\PPM_15 min.syn

7.9					
EBI	EBR	NBI	NBT	SBT	SBR
	LDIX				ODIT
	100				1
					1
					2
					Free
					-
					-
					-
					55
					0
142	198	5	171	351	2
Minor2		Major1	N	Major2	
					0
					-
					_
			-		_
		7.40			_
	-	-	-		
	2 426				-
			-		-
	0/0	1051	-		-
	-	-	-		-
801	-	-	-	-	-
		40	-	-	-
	669	1049	-	-	-
	-	-	-	-	-
	-	-	-	-	-
799	-	-	-	-	-
FB		NB		SB	
		0.0		U	
nt	NBL	NBT	EBLn1	SBT	SBR
	1049	-	572	-	-
	0.005	-	0.594	-	-
	8.4	-	20.1	_	-
	Α	-	С	-	-
)	0	-	3.9	-	-
	EBL 78 78 78 0 Stop - 0 - 1 55 27 142 Minor2 535 354 181 6.47 5.47 5.47 3.743 479 671 801 475 666 799 EB 20.1 C	TREAT BEAT TO THE TOTAL TOTAL TOTAL TO THE TOTAL	EBL EBR NBL 78 109 3 78 109 3 0 0 2 Stop Stop Free None - 210 2,# 0 - - 55 55 55 27 14 33 142 198 5 Minor2 Major1 535 354 355 354 - - 181 - - 6.47 6.24 4.43 5.47 - - 3.743 3.426 2.497 479 670 1051 671 - - 801 - - 475 669 1049 475 - - 666 - - 799 - - et NBL NBT MIND NBT <td>EBL EBR NBL NBT 78 109 3 94 78 109 3 94 0 0 2 0 Stop Stop Free Free - None - None - None 0 - 210 - -1 1 1 55 55 55 55 27 14 33 3 142 198 5 171 Minor2 Major1 M 535 354 355 0 354 - - 181 - - 6.47 6.24 4.43 - 5.47 - - - 3.743 3.426 2.497 - 475 669 1049 - 475 - - - 666 - - - 799 <</td> <td>EBL EBR NBL NBT SBT 78 109 3 94 193 78 109 3 94 193 0 0 2 0 0 Stop Free Free Free Free None - None - 0 - 210 - - 4,# 0 - - 0 0 -1 - - - 1 - 55 55 55 55 55 55 27 14 33 3 4 142 198 5 171 351 Minor2 Major1 Major2 Major2 Major2 Major2 Major2 Major2 - <td< td=""></td<></td>	EBL EBR NBL NBT 78 109 3 94 78 109 3 94 0 0 2 0 Stop Stop Free Free - None - None - None 0 - 210 - -1 1 1 55 55 55 55 27 14 33 3 142 198 5 171 Minor2 Major1 M 535 354 355 0 354 - - 181 - - 6.47 6.24 4.43 - 5.47 - - - 3.743 3.426 2.497 - 475 669 1049 - 475 - - - 666 - - - 799 <	EBL EBR NBL NBT SBT 78 109 3 94 193 78 109 3 94 193 0 0 2 0 0 Stop Free Free Free Free None - None - 0 - 210 - - 4,# 0 - - 0 0 -1 - - - 1 - 55 55 55 55 55 55 27 14 33 3 4 142 198 5 171 351 Minor2 Major1 Major2 Major2 Major2 Major2 Major2 Major2 - <td< td=""></td<>

Appendix F AUXILIARY TURN LANE WARRANT ANALYSES



DelDOT Auxiliary Lane Worksheet

Manually Update Cell	XX
Auto-Calculated Cells	XX

Roadway Information and Entrance

DOIDO								•												
Name of Pro	oject			Howard 1	. Ennis	Date of Sub	mittal			1/16/	/2020				Pro				Pro	
Maintenand	ce Road N	lo. (i.e. K234A)				Road Name				Patriot	ts Way				Proposed				Proposed	
Signalized /	' Unsignal	lized		Unsigna	ilized	Posted Spec	ed Limit			3!	5				ed Er	,	y		ed Er	
Roadway Al (From DelDe		: Manual)		291	3	Traffic Patto	ern Grou	р		7	7	Ľ,			Entrance		<u>+</u>		Entrance	
Left Approach Site ADT		Committed Development ADT	0	Total Left Approach ADT	248	Right Approach Site ADT	172	Committed Development ADT	0	Total Right Approach ADT	172	$ \rightarrow$	ti 1		1	×	/ <u>;</u>	<u> </u>	1	
Total Numb (Does Not Ir		_		2 lan	es	Number of	intersect	ion legs		4	1		100'			×		*\		
Roadway Fu	unctional	Classification		Loca	al	Calculation	for (spec	cify leg)		Proposed E	Entrance 1	220								
		ed 10 yr Roadwa I Development <i>I</i>		362	7			cted 10 yr Roadw d Development <i>l</i>		35	51		60	 		25				Ŧ
K Factor				14.1	.5	D Factor				62.	.08	`	↓			× '	 		\uparrow	V
Left Tu	rn Inf	ormation				Right T	urn lı	nformatio	on							<u></u>	y	-		Howard
Left Turn Vi	РН			94		Right Turn /	ADT			51 -	100		*				•	†	RTL	T.
Left Turn Aբ	pproach (Grade		-1.0	%	Right Turn /	Approach	n Grade		1.0)%				RTL No				Not Red X	Ennis
Heavy Vehic	cle %			20		Effective Ra	dius of E	intrance		R≤S	50'				t Rec			:: ×	quire	
10 Yr Oppos	sing Vol. ((Manual Input	- Veh/hr)	0											luired					
10 Yr Oppos	sing Volu	me (Calculated	1)	297 Ve	h/hr	Right Turn I	ength.			RTL Not F	Required				d taper		1 11	/	50	
10 Yr Oppos	sing Volu	me (Calculated	l Vol.)	297 Ve	h/hr									*					50'	
Left Turn Le	ength			220	ft	<u> </u>												å		
						<u> </u>														
						1							The Eng spreads							
													spreads	neet I	or trie (これらいけ	y en	uance		

NOTE:

This worksheet is for Right Turn Auxiliary Lanes, and Unsignalized Left Turn Auxiliary Lanes. If a signal analysis is required, please refer to signalized intersection analysis spreadsheet (Tab 6). *L1 - See Typical Entrance Diagram located at: http://www.deldot.gov/information/business/subdivisions/Typical_Entrance_Diagrams.pdf

Appendix G PEDESTRIAN CROSSING WORKSHEETS

WORKSHEET 1: PEAK-	OUR, 35 MPH (55 KM/H) OR	LES	S					
Analyst and Site Information									
Analyst: Shelby Lynch Analysis Date: 1/16/2020 Data Collection Date: 11/7/19 Major Street: Patriots Way Minor Street or Location: Proposed Site Ac Peak Hour: AM Sussex Central H									
Step 1: Select worksheet (speed reflects poster a) Worksheet 1 – 35 mph (55 km/h) or less b) Worksheet 2 – exceeds 35 mph (55 km/h)			-	·					
Step 2: Does the crossing meet minimum pede	strian volumes to be conside	red for a TCD type of trea	atment?						
Peak-hour pedestrian volume (ped/h), V _p	2a	20							
If $2a \ge 20$ ped/h, then go to Step 3. (assur									
If 2a < 20 ped/h, then consider median refug	e islands, curb extensions, tra	affic calming, etc. as feas	ible.						
Step 3: Does the crossing meet the pedestrian	volume warrant for a traffic s	ignal?							
Major road volume, total of both approaches	during peak hour (veh/h), V _m	aj-s	3a	682					
Minimum signal warrant volume for peak hou $SC = (0.00021 \text{ V}_{\text{maj-s}}^2 - 0.74072 \text{ V}_{\text{maj-s}} + 0.74072 \text{ OR}$ $OR [(0.00021 3a^2 - 0.74072 3a^2 + 0.74072 3a$	3b	436							
If $3b < 133$, then enter 133. If $3b \ge 133$, then	3c	436							
If 15 th percentile crossing speed of pedestria up to 50 percent; otherwise enter 3c.	3d	436							
If 2a ≥ 3d, then the warrant has been met an another traffic signal. Otherwise, the warra			ft (91 n arrant N						
Step 4: Estimate pedestrian delay.									
Pedestrian crossing distance, curb to curb (f	4a	55							
Pedestrian walking speed (ft/s), Sp	4b	3.5							
Pedestrian start-up time and end clearance	me (s), t _s		4c	3					
Critical gap required for crossing pedestrian	(s), $t_c = (L/S_p) + t_s$ OR [(4a/4	4b) + 4c	4d	18.7					
Major road volume, total both approaches or island is present during peak hour (veh/h		edian refuge	4e	682					
Major road flow rate (veh/s), v = V _{maj-d} /3600	4f	0.189							
Average pedestrian delay (s/person), $d_p = (\epsilon$	$v^{tc} - v t_c - 1) / v \text{ OR } [(e^{4f \times 4d})]$	- 4f x 4d - 1) / 4f]	4g	157.3					
Total pedestrian delay (h), $D_p = (d_p \times V_p)/3,6$ (this is estimated delay for all pedestrians of treatment – assumes 0% compliance). This total pedestrian delay measured at the site	ossing the major roadway wi		4h	0.874					
Step 5: Select treatment based upon total ped	strian delay and expected me	otorist compliance.							
Expected motorist compliance at pedestrian	crossings in region, Comp = h	nigh or low	<i>5</i> a	high					
Total Pedestrian Delay, D _p (from 4h) and Motorist Compliance, Comp (from 5a)	Treatment Category (see Descriptions of Samp	le Treatments for examp	les)						
$D_p \ge 21.3 \text{ h (Comp = high or low)}$ OR		RED							
$5.3 \text{ h} \leq D_p < 21.3 \text{ h} \text{ and Comp} = \text{low}$									
$ \begin{array}{c c} 1.3 \ h \leq D_p < 5.3 \ h \ (Comp = high \ or \ low) & ACTIVE \\ OR & OR & OR \end{array} $									
$5.3 \text{ h} \le D_p < 21.3 \text{ h} \text{ and Comp} = \text{high}$ ENHANCED									
$D_p < 1.3 h$ (Comp = high or low)		CROSSWALK							

Figure A-2. Worksheet 1.

WORKSHEET 1: PEAK-H	OUR, 35 MPH (55 KM/H) OR LES	S					
Analyst and Site Information								
Analyst: Shelby Lynch Analysis Date: 1/16/2020 Analysis Date: 1/16/2020 Data Collection Date: 11/7/19 Major Street: Patriots Way Minor Street or Location: Proposed Site Acceeding the Hourist Patriots Way Minor Street: Patriots Way Minor								
Step 1: Select worksheet (speed reflects poste a) Worksheet 1 – 35 mph (55 km/h) or less b) Worksheet 2 – exceeds 35 mph (55 km/h),								
Step 2: Does the crossing meet minimum pede	strian volumes to be considered for a TCD t	ype of treatment?						
Peak-hour pedestrian volume (ped/h), Vp	2a	20						
If $2a \ge 20$ ped/h, then go to Step 3. (assi								
If 2a < 20 ped/h, then consider median refug	e islands, curb extensions, traffic calming, et	c. as feasible.						
Step 3: Does the crossing meet the pedestrian	volume warrant for a traffic signal?							
Major road volume, total of both approaches	during peak hour (veh/h), V _{maj-s}	3a	324					
Minimum signal warrant volume for peak hou $SC = (0.00021 \text{ V}_{\text{maj-s}}^2 - 0.74072 \text{ V}_{\text{maj-s}} + 0.74072 \text{ OR}$ $OR [(0.00021 3a^2 - 0.74072 \text{ OR})]$	3b	688						
If $3b < 133$, then enter 133. If $3b \ge 133$, then	3c	688						
If 15 th percentile crossing speed of pedestria up to 50 percent; otherwise enter 3c.	<i>3c</i> by <i>3d</i>	688						
If 2a ≥ 3d, then the warrant has been met an another traffic signal. Otherwise, the warra		within 300 ft (91 n Warrant N						
Step 4: Estimate pedestrian delay.								
Pedestrian crossing distance, curb to curb (ft), L								
Pedestrian walking speed (ft/s), S _p								
Pedestrian start-up time and end clearance t	me (s), t _s	4c	3					
Critical gap required for crossing pedestrian	s), $t_c = (L/S_p) + t_s$ OR $[(4a/4b) + 4c)]$	4d	18.7					
Major road volume, total both approaches or island is present during peak hour (veh/h)		4e	324					
Major road flow rate (veh/s), v = V _{maj-d} /3600	4f	0.09						
Average pedestrian delay (s/person), $d_p = (e^{-\frac{\pi}{2}})^{\frac{1}{2}}$	$(t^{c} - v t_{c} - 1) / v \text{ OR } [(e^{4f \times 4d} - 4f \times 4d - 1) / v]$	′4f] 4g	30					
Total pedestrian delay (h), $D_p = (d_p \times V_p)/3,6$ (this is estimated delay for all pedestrians of treatment – assumes 0% compliance). This total pedestrian delay measured at the site	00 OR $[(4g\times2a)/3600]$ rossing the major roadway without a crossing calculated value can be replaced with the a	g ctual 4h	0.167					
Step 5: Select treatment based upon total pede	strian delay and expected motorist compliar	ice.						
Expected motorist compliance at pedestrian	crossings in region, Comp = high or low	<i>5</i> a	high					
Total Pedestrian Delay, D _p (from <i>4h</i>) and Motorist Compliance, Comp (from <i>5a</i>)	Treatment Category (see Descriptions of Sample Treatments f	or examples)						
$D_p \ge 21.3 \text{ h (Comp = high or low)}$ OR $5.3 \text{ h} \le D_p < 21.3 \text{ h and Comp = low}$	RED							
$1.3 \text{ h} \leq D_p < 5.3 \text{ h} \text{ (Comp = high or low)}$ ACTIVE OR								
$5.3 \text{ h} \leq D_p < 21.3 \text{ h} \text{ and Comp} = \text{high}$ ENHANCED								
$D_p < 1.3 \text{ h (Comp = high or low)}$ CROSSWALK								

Figure A-2. Worksheet 1.

