Site Stormwater H&H

Normal Depth Swale Calculations

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Monday, Jun 3 2019

INLET SWALE TO SE BASIN FOR FV

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_											

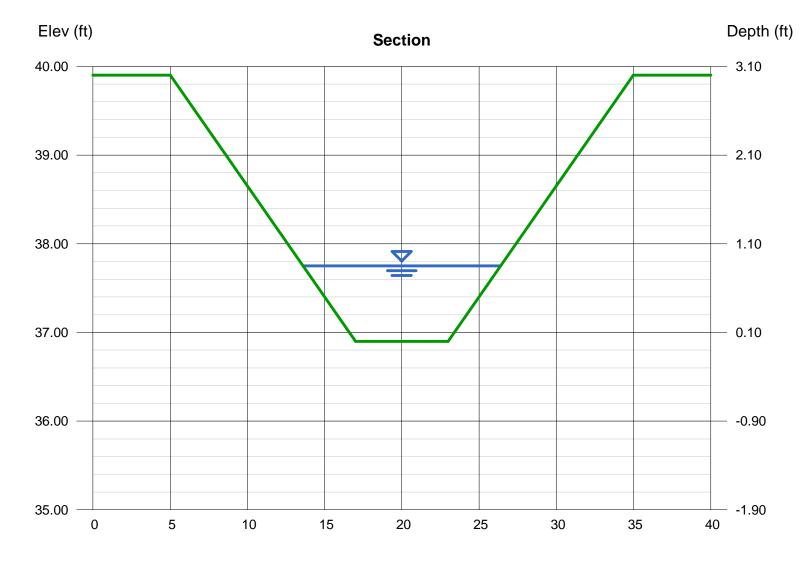
Bottom Width (ft) = 6.00 Side Slopes (z:1) = 4.00, 4.00 Total Depth (ft) = 3.00 Invert Elev (ft) = 36.90 Slope (%) = 0.53 N-Value = 0.040

Calculations

Compute by: Known Q Known Q (cfs) = 15.60

Highlighted

Depth (ft) = 0.85Q (cfs) = 15.60Area (sqft) = 7.99Velocity (ft/s) = 1.95Wetted Perim (ft) = 13.01Crit Depth, Yc (ft) = 0.53Top Width (ft) = 12.80EGL (ft) = 0.91



Daach (ft)

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Monday, Jun 3 2019

INLET SWALE TO SW BASIN FOR FV

Trapezoidal	
Dattom Widt	L

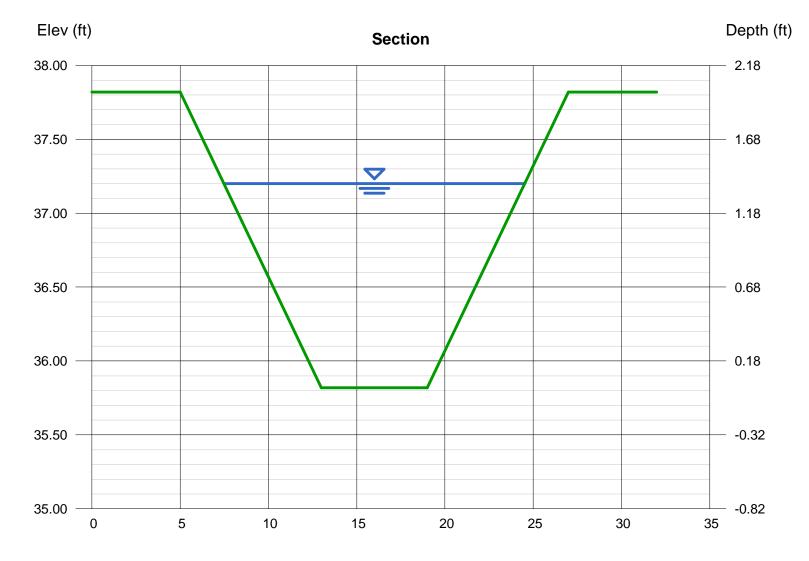
Bottom Width (ft) = 6.00 Side Slopes (z:1) = 4.00, 4.00 Total Depth (ft) = 2.00 Invert Elev (ft) = 35.82 Slope (%) = 0.50 N-Value = 0.040

Calculations

Compute by: Known Q Known Q (cfs) = 39.23

Highlighted

Depth (ft) = 1.38Q (cfs) = 39.23Area (sqft) = 15.90Velocity (ft/s) = 2.47Wetted Perim (ft) = 17.38Crit Depth, Yc (ft) = 0.90Top Width (ft) = 17.04EGL (ft) = 1.47



Dooch /ft)

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Monday, Jun 3 2019

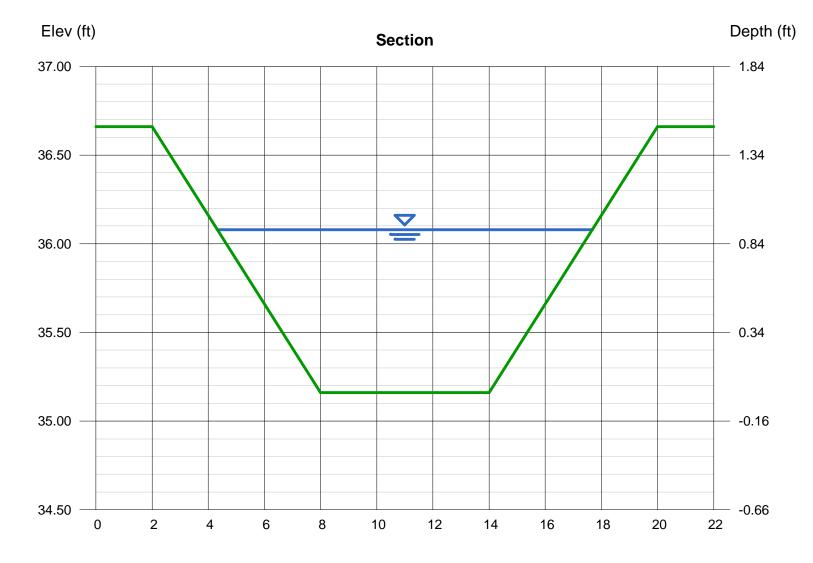
OUTFALL SWALE AT END OF SYSTEM FOR FV

Trapezoidal	
Bottom Width (ft)	= 6.00
Side Slopes (z:1)	= 4.00, 4.00
Total Depth (ft)	= 1.50
Invert Elev (ft)	= 35.16
Slope (%)	= 0.50
N-Value	= 0.040

Calculations

Compute by: Known Q Known Q (cfs) = 17.50

Highlighted		
Depth (ft)	=	0.92
Q (cfs)	=	17.50
Area (sqft)	=	8.91
Velocity (ft/s)	=	1.97
Wetted Perim (ft)	=	13.59
Crit Depth, Yc (ft)	=	0.57
Top Width (ft)	=	13.36
EGL (ft)	=	0.98



Daach (ft)

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Monday, Jun 3 2019

ROADSIDE SWALE, CATCHMENT 4A FOR FV

Trapezoidal

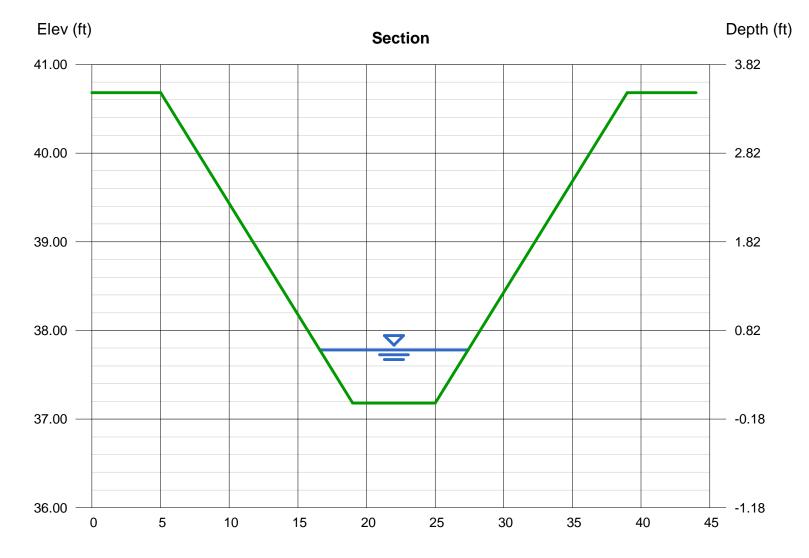
Bottom Width (ft) = 6.00 Side Slopes (z:1) = 4.00, 4.00 Total Depth (ft) = 3.50 Invert Elev (ft) = 37.18 Slope (%) = 0.42 N-Value = 0.040

Calculations

Compute by: Known Q Known Q (cfs) = 7.20

Highlighted

Depth (ft) = 0.60Q (cfs) = 7.200Area (sqft) = 5.04Velocity (ft/s) = 1.43Wetted Perim (ft) = 10.95Crit Depth, Yc (ft) = 0.33Top Width (ft) = 10.80EGL (ft) = 0.63



Dooch (ft)

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Monday, Jun 3 2019

= 0.62

ROADSIDE SWALE, CATCHMENT 4B FOR FV

Trapezoidal
Bottom Width (ft) = 6.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.25
Invert Elev (ft) = 34.71

Invert Elev (ft) = 34.71 Slope (%) = 0.42 N-Value = 0.040

Calculations

Compute by: Known Q Known Q (cfs) = 7.01

 Highlighted

 Depth (ft)
 = 0.59

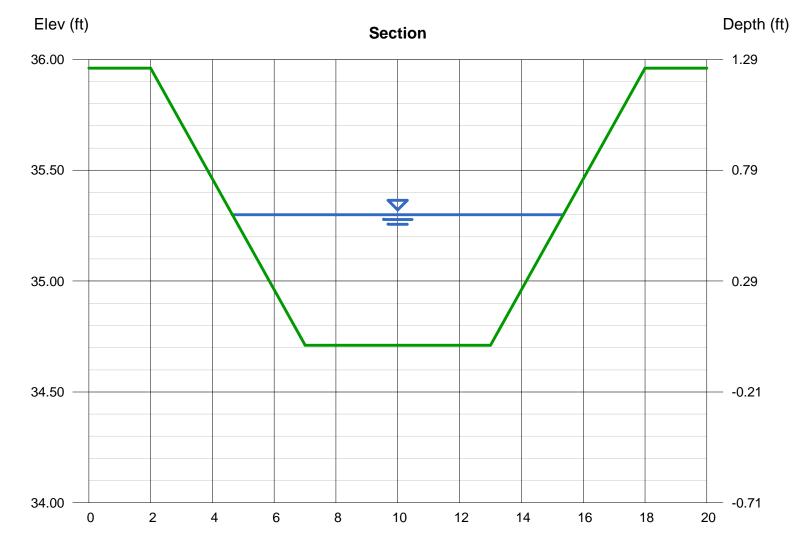
 Q (cfs)
 = 7.010

 Area (sqft)
 = 4.93

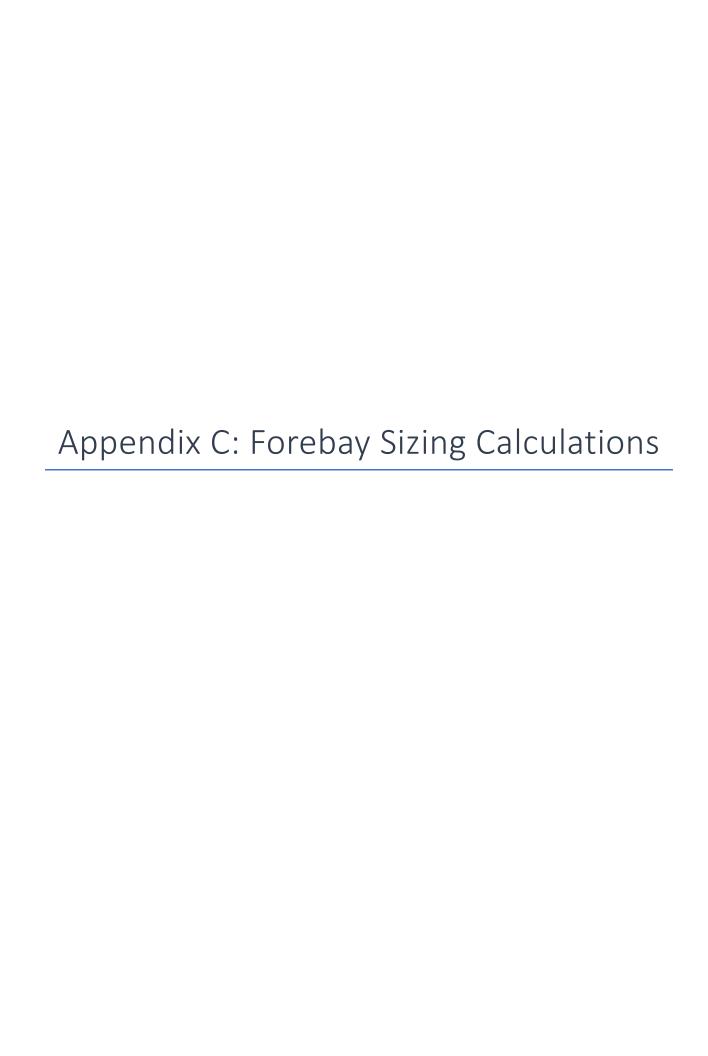
 Velocity (ft/s)
 = 1.42

Velocity (ft/s) = 1.42Wetted Perim (ft) = 10.87Crit Depth, Yc (ft) = 0.33Top Width (ft) = 10.72

EGL (ft)



Dooch /ft)



Forebay Sizing Calculations

NW INFILTRATION BASIN FOREBAY SIZING

RPV RUNOFF ENTERING FOREBAY = 4.844 AC*4,3560*0.35"/12 = 5702 CU-FT (DIRECT RUNOFF, NOT TREATED BY AN UPSTREAM BMP FROM SUMMARY FOR POND NW BMP FORBAY IN HYDROCAD)

10% RPV = 570.2 CU-FT

STORAGE PROVIDED IN FOREBAY AT ELEVATION OF WEIR (39.10') = 3,000 CU-FT

SW INFILTRATION BASIN FOREBAY SIZING

RPV RUNOFF ENTERING FOREBAY = 11.526 AC*4,3560*0.57"/12 = 23848 CU-FT (DIRECT RUNOFF, NOT TREATED BY AN UPSTREAM BMP FROM SUMMARY FOR POND NW BMP FORBAY IN HYDROCAD)

10% RPV = 2384.8 CU-FT

STORAGE PROVIDED IN FOREBAY AT ELEVATION OF WEIR (37.30') = 8,500 CU-FT





June 6, 2019

REPORT ON INFILTROMETER TESTING AT 24778 DUPONT BOULEVARD, GEORGETOWN, DE 19947

SUSSEX PARCEL ID: 133-6.00-123.00

The scope of work for this project included a soil profile to the depth of groundwater, and infiltrometer testing to allow engineering design for SWM to proceed.

Nine (9) infiltration tests to varying depths based on proposed stormwater features were constructed. Groundwater levels indicated that approximately six to seven (6-7) feet of unsaturated material exists on the site.

The infiltrometers were 4" in diameter, constructed of sch. 10 pvc and driven into the bottom of the auger depth 3". 24" or the max amount of water allowable was initially added to each infiltrometer. After each one (1) hour period the depth of water remaining was measured and then returned to the 24" depth for a minimum of three hours. A pre-wetting period of either one hour, or a 12" water level drop was used to simulate the saturated conditions of a stormwater facility.

Deep auger borings within the test areas were used to obtain the soil profile. Infiltrometers were then constructed nearby the profile hole to run the infiltration tests. This soil data is shown in the attachments. NRCS has the soil in the area identified as Downer loamy sand. The profiles show a well-drained soil, and extensive depth of structure. No confining layers were observed during the borings.

A summary table of the results has been provided below.



Summary Table:

Test #	Longitude	Latitude	Test Depth	Elevation	Design
			Inches	Feet	Infiltration
	20				Rate
1	75°21'29.19"W	38°38'30.74"N	16"	40.00	0.532 in/hr
2	75°21'26.23"W	38°38'26.70"N	37.5"	36.90	14.408 in/hr
3	75°21'27.03"W	38°38'26.11"N	31"	36.90	2.246 in/hr
4	75°21'32.15"W	38°38'26.59"N	39"	35.80	10.8 in/hr
5	75°21'29.59"W	38°38'25.87"N	42"	39.44	15 in/hr
6	75°21'33.94"W	38°38'27.92"N	38"	35.80	15 in/hr
7	75°21'36.90"W	38°38'32.49"N	59"	37.10	2.8 in/hr
8	75°21'33.74"W	38°38'32.06"N	36	39.00	1.464 in/hr

Submitted by:

Kyle a. Kowalczyk

Kyle Kowalczyk, Environmental Scientist.



APPENDIX 1: TEST LOCATION SITE PLAN





APPENDIX 2: SOIL BORING LOGS

Name of Tester: Kyle Kowalczyk
Test Method: Single Ring
Temperature: 65°
Precipitation: 0"
Boring Diameter: 4.25"
Casing Diameter: 4.25"
Depth of Penetration: 3"

Date	2.5.19
Profile #	-

Remarks				gravel	gravel					tacky	tacky	tacky, wet	tacky, wet
Structure F	2msbk	2msbk	1msbk	2csbk (2csbk (1msbk	1msbk	2msbk	2msbk	2msbk	2msbk	2msbk	2msbk
Texture	SI	SI	Is	ls S	sl	SI	IS	SI	S	S	S	S	S
Description Texture Structure Remarks					f2d,m2d	m2d	m2p	m2d	m2d	m2d	m2d,c2d	m2p	7.5yr 6/8 m2d,m2d
					10yr 6/1						7.5yr 6/8		7.5yr 6/8
Mottles					5yr 5/8	10yr 6/1	2.5y 7/1	7.5yr 6/8	2.5y 7/2	2.5y 7/2	5y 7/1	2.5y 7/4	5y 7/1
Matrix	10yr 4/2	2.5y 6/4	2.5y 6/3	2.5y 6/3	2.5y 6/3	2.5y 6/3	2.5y 7/3	5y 7/2	10yr 7/6	10yr 7/6	2.5y 7/4	2.5y 7/1	2.5y 7/4
Thickness	12	9	80	4	80	4	80	10	4	4	12	8	4
	12	18	26	30	38	42	50	09	64	89	80	88	92
Depth (in)	0	12	18	26	30	38	42	50	09	64	89	80	88

	% Slope			0-2%			
Landscape	Feature:	Summit	Shoulder	Sideslope	Footslope	Toeslope	Depression

Hillside	
Element:	
Divergent	
Convergent	
Level	*

Kyle Kowalczyk Single Ring 51° 0.2" 4.25" 3"

Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:

	Remarks				2msbk gravel, wet		2csbk H2O at 74"
	Structure	2msbk	2msbk	2msbk	2msbk	1msbk	2csbk
	Texture	SI	sl	S	IS	hsl	IS
	Description Texture Structure Remarks		m2p	m2p,f1d	m2p	m2p,m2d	m2d,f1d
				10yr 5/8		7.5yr 6/8	5y 8/1 7.5yr 7/8
	Mottles		2.5y 7/2	2.5y 7/2	2.5y 7/2	2.5y 7/1	5y 8/1
	Matrix	10yr 4/2	2.5y 6/4	2.5y 6/4	2.5y 7/4	2.5y 6/4	2.5y 7/4
Date 2.7.19	Thickness	12	80	18	18	18	4
		12	20	38	56	74	78
Profile #	Depth (in)	0	12	20	38	56	74

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	0-2%
Footslope	
Toeslope	
Depression	

				*
Hillside	Element:	Divergent	Convergent	Level
	į.			

Kyle Kowalczyk Single Ring 65° 0" 4.25" 3"

Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:
Depth of Penetration:

Profile #

Date 2.5.19

	Remarks				wet	wet	wet, heavy	wet, heavy	3msbk H2O at 82"
	Structure	1msbk	2msbk	2csbk	1msbk	3msbk	3msbk	3msbk	3msbk
	Texture	sl	sl	sl	sl	sl	sl	sl	SI
	Description Texture Structure Remarks		m2p	m2p, f1d		m2p, m2d	m3d, m2d	m2d,m2d	c2d, c2d
				7.5yr 5/8		7.5yr 5/8	7.5yr 5/8	5y 7/1	2.5y 6/4
	Mottles		2.5y 7/2	2.5y 7/2		2.5y 7/2	5y 7/1	10yr 6/6	5yr 6/8
	Matrix	10yr 4/2	2.5y 6/4	2.5y 6/4	2.5y 7/4	2.5y 7/4	2.5y 7/4	5yr 5/8	5y 7/1
	Thickness	14	14	16	4	4	5	7	18
		14	28	44	48	52	57	64	82
,	Depth (in)	0	14	28	44	48	52	57	64

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	0-2%
Footslope	
Toeslope	
Depression	

				*
Hillside	Element:	Divergent	Convergent	Level

Kyle Kowalczyk Single Ring 51° 0.2" Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:
Depth of Penetration:

Profile #

Date 2.7.19

		Modulic	001410M		Docorintion Toyture Structure Bemarks	Toxfiiro	Stricting	Pomarke
Inickness	SS	Matrix	Mottles		Describion	aınıxaı	amanne	Vellial
10		10yr 5/2				IS	1msbk	
8		2.5y 6/4				SI	1msbk	
10		2.5y 6/2				sl	1msbk	gravel
12		2.5y 6/2				sl	1csbk	gravel
7		2.5y 6/2	7.5yr 6/8		c1d	S	1csbk	
19		2.5y 6/2	7.5yr 6/8	5y 8/1	m2d,c2d	sl/ls	1fsbk	
17		2.5y 6/2	7.5yr 6/8	5y 8/1	5y 8/1 f1d,m2d,f1d	sl	2csbk	
4		9.5 N	7.5yr 6/8	5G 3/1	f1d	sl	2csbk	H2O at 8
	1							

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	0-2%
Footslope	
Toeslope	
Depression	

				*
Hillside	Element:	Divergent	Convergent	Level

Name of Tester: Test Method:

Kyle Kowalczyk Single Ring 51° 0.2"

Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:

Depth of Penetration:

Date	2.7.19
Profile #	2

	Remarks			gravel	gravel	gravel	gravel, wet at 58"	2msbk H2O at 79"
,	Structure	1msbk	2msbk	2msbk	2csbk	2csbk	2csbk	2msbk
ı	Texture	IS	sl	sl	sl	sl/ls	SI	sl
	Description Texture Structure Remarks			f1d			f1d,f1d	c2d
							5y 8/1	
	Mottles			7.5yr 5/8			7.5yr 6/8	7.5yr 6/8
;	Matrix	10yr 4/2	2.5y 6/4	2.5y 6/4	2.5y 6/2	2.5y 6/2	2.5y 6/2	5y 8/1
i	Thickness	12	9	8	8	10	16	19
		12	18	26	34	44	09	79
	Depth (in)	0	12	18	26	34	44	. 09

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	0-2%
Footslope	
_oeslobe	
Depression	

				*
Hillside	Element:	Divergent	Convergent	l evel

Kyle Kowalczyk Single Ring 51° 0.2"

Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:

Profile #

Date 2.7.19

Remarks			gravel		3msbk H2O at 79"	
Structure	massive	1msbk	1csbk	2msbk	3msbk	
Texture	S	S	sl/ls	sl	sl	
Description Texture Structure Remarks				7.5yr 7/8 m3d,m2d	f1d	
				7.5yr 7/8		
Mottles				5y 8/1	7.5yr 7/6	
Matrix	10yr 4/3	2.5y 6/4	2.5y 7/3	2.5y 7/3	N/6	
Thickness	12	12	10	24	21	
	12	24	34	58	79	
Depth (in)	0	12	24	34	28	

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	
Footslope	
Toeslope	0-5%
Denression	

				*
Hillside	Element:	Divergent	Convergent.	Level

Kyle Kowalczyk Single Ring 51° 0.2" Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:
Depth of Penetration:

Profile # 7		Date 2.7.19							
Depth (in)		Thickness	Matrix	Mottles		Description Texture Structure Remarks	Texture	Structure	Remarks
0	11	11	10yr 4/2				IS	2msbk	
11	30	19	2.5y 6/3				sl	2msbk	heavy
30	40	10	2.5y 6/3	7.5yr 6/8	10yr 7/1	f1d,c1d	sl	3msbk	gravel, heav
40	99	16	2.5y 7/2	2.5y 6/3	7.5yr 6/8	m3p,m3d	IS	2csbk	
56	64	8	5y 7/1	7.5yr 6/8		c1d	IS	2csbk	
64	74	10	5v 7/1	5G 3/1	7.5vr 6/8	f1d.f1d	S	1csbk	1csbk H2O at 74"

3msbk gravel, heavy
2csbk
1csbk H2O at 74"

Landscape		1
Feature:	edols %	
Summit		
Shoulder		
Sideslope		
Footslope		
Toeslope	0-2%	
Depression		

Hillside Element: Divergent	
Element: Divergent	
Divergent	
Convergent	
* Fevel	

Name of Tester: Test Method:

Kyle Kowalczyk Single Ring 51° 0.2"

Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:
Depth of Penetration:

Profile #

Remarks			gravel	heavy			heavy	576	2msbk H2O at 84"
Structure	1msbk	1msbk	2msbk	3msbk	3msbk	3msbk	2msbk	2fsbk	2msbk
Texture	sl	SI	IS	IS	scl	scl	IS	SI	IS
Description Texture Structure Remarks			f1d	m2d	m2d	c1d,m2p	c2d,m2p		f1d,f1d
		(3)				2.5y 7/1	2.5y 7/1		7.5yr 7/6
Mottles			7.5yr 5/8	7.5yr 5/8	7.5yr 5/8	7.5yr 6/8	7.5yr 6/8		5G 3/1
Matrix	10yr 5/2	10yr 6/3	10yr 6/3	2.5y 6/2	2.5y 6/2	2.5y 6/4	2.5y 6/4	2.5y 6/4	5y 8/1
Thickness	12	8	12	4	5	9	6	13	15
	12	20	32	36	41	47	99	69	84
Depth (in)	0	12	20	32	36	41	47	56	69

Landscape	
Feature:	% Slope
Summit	
Shoulder	12
Sideslope	
Footslope	0-2%
Toeslope	
Depression	

				*
Hillside	Element:	Divergent	Convergent	Level

Kyle Kowalczyk Single Ring 32° 0" 4.25"

Name of Tester:
Test Method:
Temperature:
Precipitation:
Boring Diameter:
Casing Diameter:
Depth of Penetratic

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tion:	
etrat	
- Pen	
0	
pth	

Date	26.10

6

Profile #

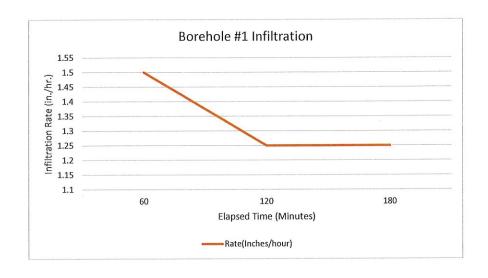
emarks				gravel at 40"			H2O at 88"
Structure R	1msbk	1msbk	2msbk	2msbk gr	1msbk	1msbk	1msbk H
Texture	IS	IS	IS	ls	IS	S	S
Description Texture Structure Remarks			f1d,c1p	f1d,f1d	c2d	c1d	f2d,c2d
			10yr 6/1	10yr 7/1			10yr 8/4
Mottles			7.5yr 5/8	7.5yr 5/8	7.5yr 5/8	10yr 8/4	7.5yr 7/8
Matrix	10yr 4/2	10yr 5/4	10yr 5/6	2.5y 6/4	10yr 7/1	5y 8/1	N/6
Thickness	6	12	8	17	12	20	10
	6	21	29	46	58	78	88
Depth (in)	0	6	21	29	46	58	78

Landscape	
Feature:	% Slope
Summit	
Shoulder	
Sideslope	0-2%
Footslope	
Toeslope	
Depression	

ø	ıt:	ţ	ent	*
Hillside	Element:	Divergent	Convergent	Level

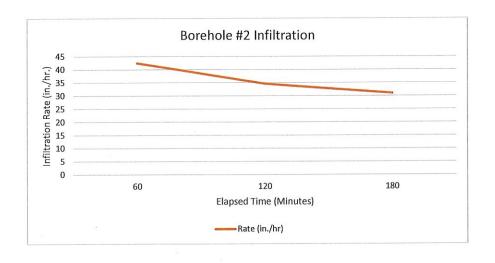


APPENDIX 3: INFILTRATION RATES AND GRAPHS



		INFILTE	RATION TEST				
		16" DEEP I	NFILTROMETER				
Start Tme	Start Tme End Time Duration Infiltration Rate (inch/hour						
6:12	7:12	1:00	1.5	1.5			
6:12	6:27	0:15	0.375				
6:27	6:42	0:15	0.375				
6:42	6:57	0:15	0.375				
6:57	7:12	0:15	0.375				
7:13	8:13	1:00	1.25	1.25			
7:13	7:28	0:15	0.3125				
7:28	7:43	0:15	0.3125				
7:43	7:58	0:15	0.3125				
7:58	8:13	0:15	0.3125				
8:13	9:13	1:00	1.25	1.25			
8:13	8:28	0:15	0.3125				
8:28	8:43	0:15	0.3125				
8:43	8:58	0:15	0.3125				
8:58	9:13	0:15	0.3125				
Average				1.33			

^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 0.532 IN/HR



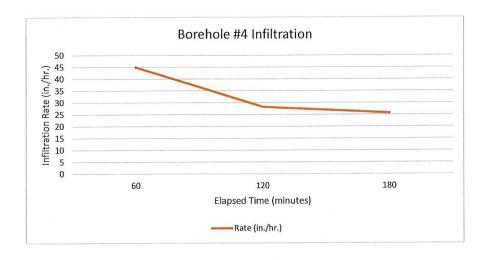
		INFILTE	RATION TEST	
	3	7.5" DEEP	INFILTROMETER	
Start Time	End Time	Duration	Infiltration	Rate (inch/hour)
12:25	1:18	53	37.5	42.452
12:25	12:40	0:15	10.613	
12:40	12:55	0:15	10.613	1
12:55	1:10	0:15	10.613	
1:10	1:18	0:08	5.66	
1:19	2:24	1:05	37.5	34.6
1:19	1:34	0:15	8.654	
1:34	1:49	0:15	8.654	
1:49	2:04	0:15	8.654	
2:04	2:19	0:15	8.654	
2:19	2:24	0:05	2.885	
2:26	3:26	1:00	31	31
2:26	2:41	0:15	7.75	
2:41	2:56	0:15	7.75	
2:56	3:11	0:15	7.75	
3:11	3:26	0:15	7.75	
Average				36.02

^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 14.408 IN/HR



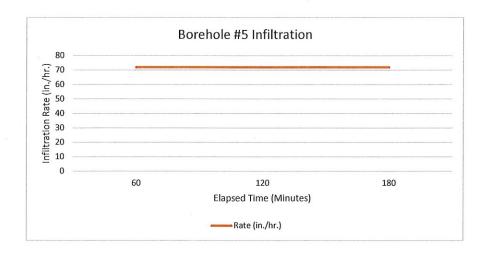
		INFILTE	RATION TEST	
		31" DEEP I	NFILTROMETER	
Start Time	End Time	Duration	Infiltration	Rate (inch/hour)
1:17	2:17	1:00	5.5"	5.5
1:17	1:32	0:15	1.375	
1:32	1:47	0:15	1.375	
1:47	2:02	0:15	1.375	
2:02	2:17	0:15	1.375	
2:22	3:22	1:00	5.70"	5.7
2:22	2:37	0:15	1.425	
2:37	2:52	0:15	1.425	
2:52	3:07	0:15	1.425	
3:07	3:22	0:15	1.425	
3:23	4:23	1:00	5.65"	5.6
3:23	3:38	0:15	1.4125	,
3:38	3:53	0:15	1.4125	
3:53	4:08	0:15	1.4125	
4:08	4:23	0:15	1.4125	
Average				5.61

^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 2.246 IN/HR



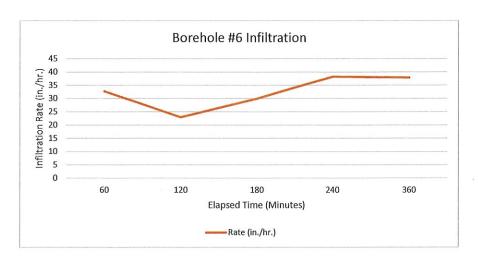
		INFILTE	RATION TEST	
		39" DEEP I	NFILTROMETER	
Time Start	Time End	Duration	Infiltration	Rate (inches/hour)
11:33	11:49	0:16	12"	45
11:33	11:48	0:15	11.25	
11:48	11:49	0:01	0.75	
11:50	12:16	0:26	12.25"	28.27
11:50	12:05	0:15	7.067	
12:05	12:16	0:11	5.183	
12:17	12:45	0:28	12"	25.71
12:17	12:32	0:15	6.429	
12:32	12:45	0:13	5.571	
AVERAGE				27"

^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 10.8 IN/HR



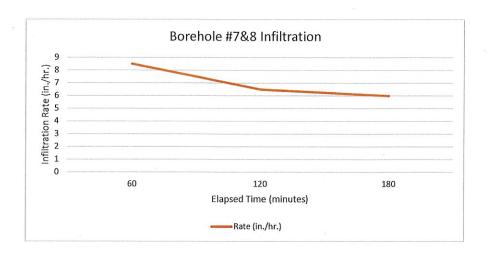
		INFILTE	RATION TEST		
		42" DEEP I	NFILTROMETER		
Time Start	Time End	Duration	Iniltration	Rate (/hr)	
11:38	11:53	0:15	>18"		>72"
11:53	12:08	0:15	>18"		>72"
12:09	12:22	0:13	>18"		>72"
Average	-				75.67

*USING A FACTOR OF SAFETY, THE DEIGN INFILTRATION RATE SHALL BE NO GREATER THAN 15 IN/HR



		INFILTE	RATION TEST	
		38" DEEP I	NFILTROMETER	
Start Time	End Time	Duration	Infiltration (inches)	Rate (inch/hour)
11:42	12:04	0:22	12	32.72
11:42	11:57	0:15	8.182	
11:57	12:04	0:07	3.818	
12:04	12:38	0:34	13	22.94
12:04	12:19	0:15	5.735	
12:19	12:34	0:15	5.735	
12:34	12:38	0:04	1.529	
12:39	1:05	0:26	13	30
12:39	12:54	0:15	7.5	
12:54	1:05	0:11	5.5	
1:09	1:29	0:20	12.75	38.25
1:09	1:24	0:15	9.563	
1:24	1:29	0:05	3.188	
1:33	1:52	0:19	12	37.89
1:33	1:48	0:15	9.474	
1:48	1:52	0:04	2.526	
Average				32.56

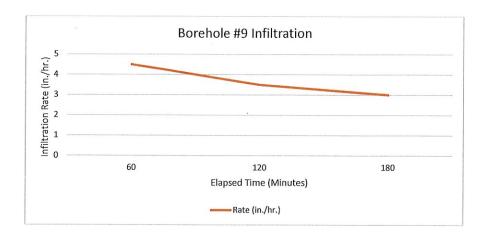
^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 15 IN/HR $\,$



		INFILTE	RATION TEST				
		59" DEEP I	NFILTROMETER				
Start Time	End Time Duration Infiltration (inches) Rate (/hour)						
8:25	9:25	1:00	8.5	8.5			
8:25	8:40	0:15	2.125				
8:40	8:55	0:15	2.125				
8:55	9:10	0:15	2.125	58			
9:10	9:25	0:15	2.125				
9:27	10:27	1:00	6.5	6.5			
9:27	9:42	0:15	1.625				
9:42	9:57	0:15	1.625				
9:57	10:12	0:15	1.625				
10:12	10:27	0:15	1.625				
10:28	11:28	1:00	6	6			
10:28	10:43	0:15	1.5				
10:43	10:58	0:15	1.5				
10:58	11:13	0:15	1.5				
11:13	11:28	0:15	1.5				
Average				7			

Please Note* Infiltration test performed approximately equidistant between profiles 7 and 8.

*USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 2.8 IN/HR



		INFILTE	RATION TEST				
		36" DEEP I	NFILTROMETER				
Start Time	Start Time End Time Duration Infiltration (inches) Rate (/hour)						
11:42	12:42	1:00	4.5	4.5			
11:42	11:57	0:15	1.125				
11:57	12:12	0:15	1.125				
12:12	12:27	0:15	1.125				
12:27	12:42	0:15	1.125				
12:42	1:42	1:00	3.5	3.5			
12:42	12:57	0:15	0.875				
12:57	1:12	0:15	0.875				
1:12	1:27	0:15	0.875				
1:27	1:42	0:15	0.875				
1:42	2:42	1:00	3	3			
1:42	1:57	0:15	0.75				
1:57	2:12	0:15	0.75				
2:12	2:27	0:15	0.75				
2:27	2:42	0:15	0.75	*			
Average				3.66			

^{*}USING A FACTOR OF SAFETY OF 2.5, THE DESIGN INFILTRATION RATE SHALL BE NO GREATER THAN 1.464 IN/HR



APPENDIX 12

PRELIMINARY STORMWATER MANAGEMENT PLANS

DAGSBORO HUNDRED 24778 DU PONT BLVD. **TAX PARCEL NUMBER 113-6-123.00** SUSSEX COUNTY, DELAWARE

TITLE	Τ ¯	SHEET
11116	R	JMBER
STORMWATER COVER SHEET	000	006-C-SW000
SWM KEY	/001	006-C-SW001
PRE DEVELOPMENT SUBAREA LOD AREA	010	006-C-SW010
PRE CONSTRUCTION SWM PLAN	/011	006-C-SW011
CONSTRUCTION SWM PLAN	012	006-C-SW012
BMP CONTRIBUTING AREA PLAN	013	006-C-SW013
CONSTRUCTION SITE DETAILS AND NOT	020	006-C-SW020
CONSTRUCTION SITE DETAILS AND NOT	/021	006-C-SW021
CONSTRUCTION SITE DETAILS AND NOT	022	006-C-SW022
CONSTRUCTION SITE DETAILS AND NOT	023	006-C-SW023
CONSTRUCTION SITE DETAILS AND NOT	024	006-C-SW024
CONSTRUCTION SITE DETAILS AND NOT	025	006-C-SW025
CONSTRUCTION SITE SITE DETAILS AND N	026 C	006-C-SW026
POST CONSTRUCTION SITE SWM PLAI	030	006-C-SW030
POST CONSTRUCTION SITE SWM PLAN	/031	006-C-SW031
POST CONSTRUCTION SITE SWM PLAN	032	006-C-SW032
POST CONSTRUCTION SITE SWM PLAN	033	006-C-SW033
POST CONSTRUCTION SITE SWM PLAN	034	006-C-SW034
PIPE SCHEDULE AND DETAILS	040	006-C-SW040
POST CONSTRUCTION SITE DETAILS	/041	006-C-SW041
POST CONSTRUCTION SITE DETAILS	042	006-C-SW042

CB006-C-SW043

1. PROPERTY IS LOCATED AT DISTRICT 133, TAX MAP 6.00, PARCEL 123.00

POST CONSTRUCTION SITE DETAILS 3

- 2. DEED REFERENCE: BOOK 7542 PAGE 0
- 3. PARCEL LOCATED OUTSIDE MUNICIPALITY BOUNDARIES OF THE TOWN OF
- 4. ZONING IS RESIDENTIAL/AGRICULTURAL
- 5. CURRENT LAND USE IS AGRICULTURAL
- 6. PROPOSED CONDITIONAL LAND USE -- GENERAL INDUSTRIAL (I -1)
- 7. PARCEL IS LOCATED IN FLOOD ZONE X (AREA OF MINIMAL FLOOD HAZARD) FIRM PANEL 10005C0325K EFFECTIVE MARCH 16, 2015

6 MW POWER GENERATION FACILITY

DESIGNER DATA:

106 N HARRISON STREET

EASTON, MD. 21601

LAND DEVELOPER DATA: **REVIEW AGENCY DATA:**

PARCEL DATA:

PLUS #: 2017-08-04

TAX MAP #: 133-6.00-123.00

CONDITIONAL USE #: 2589

LOCATION: 20.07 ACRES

CLEANBAY BIOFUELS LLC. 726 SECOND STREET. UNIT 3B

ANNAPOLIS, MD. 21403

OWNER DATA:

410 514 6488

DNR SEDIMENT AND STORMWATER PROGRAM #:

LATITUDE/LONGITUDE: 38.64204° N 75.35889° W

EXISTING SITE AREA: 19.244 ACRES PROPOSED SITE AREA: 19.244 ACRES

EXISTING WETLAND AREA: 2.61 ACRES

PROPOSED CONDITION: INDUSTRIAL

ADDRESS: 24778 DU PONT BLVD, GEORGETOWN, DE.

WATERSHED: COW BRIDGE BRANCH, INDIAN RIVER

PROPOSED DISCHARGE LOCATION: HORSE POND SWAMP DITCH

PROPOSED TOTAL LIMIT OF DISTURBANCE PER DISCHARGE

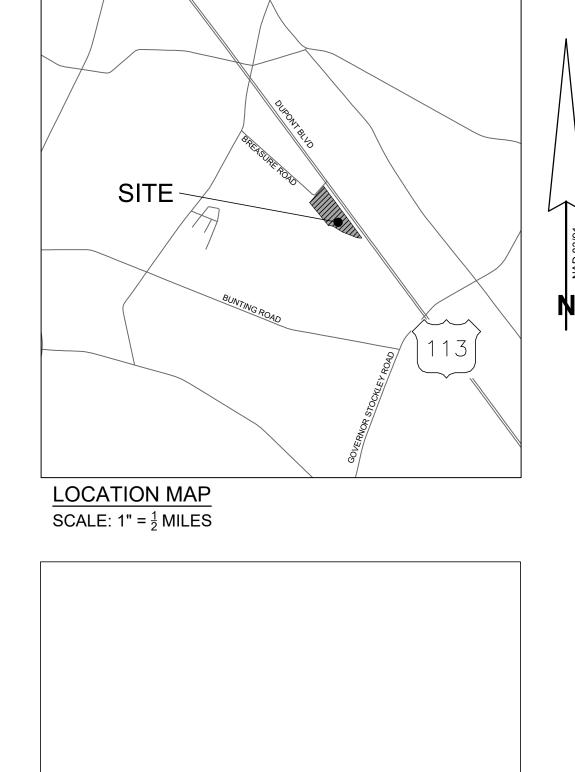
SUSSEX CONSERVATION DISTRICT 23818 SHORTLY ROAD **GEORGETOWN DE 19947** 302 856 7219

DAYS PRIOR TO COMMENCING WITH CONSTRUCTION. FAILURE TO DO SO CONSTITUTES A VIOLATION O APPROVED SEDIMENT AND STORMWATER MANAGEMENT PLAN.

RAUCH INC

410 770 9081

- CONTRACTOR FROM HIS OR HER RESPONSIBILITIES FOR COMPLIANCE WITH THE REQUIREMENTS OF THE SEDIMENT AND STORMW ATER REGULATIONS, NOR SHALL IT RELIEVE THE CONTRACTOR FROM ERRORS OR
- 3. IF THE APPROVED PLAN NEEDS TO BE MODIFIED, ADDITIONAL SEDIMENT AND STORMWATER CONTROL MEASURES MAY BE REQUIRED AS DEEMED NECESSARY BY DNREC.
- 4. FOLLOWING SOIL DISTURBANCE OR REDISTURBANCE. PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED FOR ALL PERIMETER SEDIMENT CONTROLS, SOIL STOCKPILES, AND ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE WITHIN 14 CALENDAR DAYS UNLESS MORE RESTRICTIVE FEDERAL
- 5. ALL EROSION AND SEDIMENT CONTROL PRACTICES SHALL COMPLY WITH THE DELAWARE EROSION AND SEDIMENT CONTROL HANDBOOK, LATEST EDITION.
- 6. AT ANY TIME A DEWATERING OPERATION IS USED, IT SHALL BE PREVIOUSLY APPROVED BY THE AGENCY CONSTRUCTION SITE REVIEWER FOR A NON-EROSIVE POINT OF DISCHARGE, AND A DEWATERING PERMIT SHALL BE APPROVED BY THE DNREC WELL PERMITTING BRANCH.
- 7. APPROVED PLANS REMAIN VALID FOR 3 YEARS FROM THE DATE OF APPROVAL.
- 8. POST CONSTRUCTION VERIFICATION DRAWINGS ARE TO BE SUBMITTED TO THE DISTRICT WITHIN 60-DAYS OF STORMWATER MANAGEMENT FACILITY COMPLETION.
- 9. APPROVAL OF A SEDIMENT AND STORMWATER PLAN DOES NOT GRANT OR IMPLY A RIGHT TO DISCHARGE STORMWATER RUNOFF. THE OWNER/DEVEWPER IS RESPONSIBLE FOR ACQUIRING ANY AND ALL AGREEMENTS. EASEMENTS, ETC., NECESSARY TO COMPLY WITH STATE DRAINAGE AND OTHER APPLICABLE LAWS.
- 10. THE NOTICE OF INTENT FOR STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY UNDER A NPDES GENERAL PERMIT FOR THIS PROJECT IS# . AT ANY TIME THE OWNERSHIP FOR THIS PROJECT CHANGES, A TRANSFER OF AUTHORIZATION OR A CO-PERMITTEE APPLICATION MUST BE SUBMITTED TO DNREC. THE PERMITTEE OF RECORD SHALL NOT BE RELIEVED OF THEIR RESPONSIBILITIES UNTIL A NOTICE OF TERMINATION HAS BEEN PROCESSED BY DNREC.
- 11. THE OWNER SHALL BE FAMILIAR WITH AND COMPLY WITH ALL ASPECTS OF THE NPDES CONSTRUCTION GENERAL PERMIT ASSOCIATED WITH THE PROJECT, INCLUDING, BUT NOT LIMITED TO, PERFORMING WEEKLY SITE INSPECTIONS DURING CONSTRUCTION AND AFTER RAIN EVENTS, AND MAINTAINING WRITTEN LOGS OF
- 12. THE CONTRACTOR SHALL AT ALL TIMES PROTECT AGAINST SEDIMENT OF DEBRIS LADEN RUNOFF OR WIND FROM LEAVING THE SIDE. PERIMETER CONTROLS SHALL BE CHECKED DAILY AND ADJUSTED AND/OR REPAIRED TO FULLY CONTAIN AND CONTROL SEDIMENT FROM LEAVING THE SITE. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT HAS REACHED HALF OF THE EFFECTIVE CAPACITY OF THE CONTROL. IN ADDITION, THE CONTRACTOR MAY NEED TO ADJUST OR ALTER MEASURES IN TIMES OF ADVERSE WEATHER CONDITIONS, OR AS DIRECTED BY THE AGENCY SITE REVIEWER.
- 13. BEFORE ANY EARTHWORK OR EXCAVATION TAKES PLACE, THE CONTRACTOR SHALL CALL MISS UTILITY AT 811 OR 1.800.282.8555 AT LEAST 48 HOURS PRIOR TO CONSTRUCTION, TO HAVE ALL EXISTING UTILITIES MARKED ONSITE.
- 14. BEST AVAILABLE TECHNOLOGY (BAT) SHALL BE EMPLOYED TO MANAGE TURBID DISCHARGES IN ACCORDANCE WITH REQUIREMENTS OF 7. DEL C. CH 60, REGULATIONS GOVERNING THE CONTROL OF WATER POLLUTION, SECTION 9.1.02, KNOWN AS SPECIAL CONDITIONS FOR STORMWATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITIES AND DEPARTMENT POLICIES, PROCEDURES, AND GUIDANCE.
- 15. DOCUMENTATION OF SOIL TESTING AND MATERIALS USED FOR TEMPORARY OR PERMANENT STABILIZATION INCLUDING BUT NOT LIMITED TO SOIL TEST RESULTS, SEED TAGS, SOIL AMENDMENT TAGS, ETC, SHALL BE PROVIDED TO THE DEPARTMENT OR DELEGATED AGENCY TO VERIFY THAT THE PERMANENT OR TEMPORARY STABILIZATION HAS BEEN COMPLETED IN ACCORDANCE WITH THE APPROVED PLAN AND THE STANDARDS AND SPECIFICATIONS OF THE DELAWARE EROSION AND SEDIMENT CONTROL HANDBOOK, LATEST EDITION.
- 16. THE DEPARTMENT OF THE DELEGATED AGENCY SHALL HAVE THE DISCRETION TO REQUIRE ADDITIONAL SOIL TESTING AND REAPPLICATION OF PERMANENT OR TEMPORARY STABILIZATION IN ACCORDANCE WITH THE SPECIFICATION PROVIDED WITHIN THE DELAWARE EROSION AND SEDIMENT CONTROL HANDBOOK, LATEST EDITION.



OWNER'S CERTIFICATION:

"I, THE UNDERSIGNED, CERTIFY THAT ALL LAND CLEARING, CONSTRUCTION AND DEVELOPMENT SHALL BE DONE PURSUANT TO THE APPROVED PLAN AND THAT RESPONSIBLE PERSONNEL (I.E., BLUE CARD HOLDER) INVOLVED IN THE LAND DISTURBANCE WILL HAVE A CERTIFICATION OF TRAINING PRIOR TO INITIATION OF THE PROJECT, AT A DNREC SPONSORED OR APPROVED TRAINING COURSE FOR THE CONTROL OF EROSION AND SEDIMENT DURING CONSTRUCTION. IN ADDITION, I GRANT THE DNREC SEDIMENT AND STORMW ATER PROGRAM AND/OR THE RELEVANT DELEGATED AGENCY THE RIGHT TO CONDUCT ON-SITE REVIEWS, AND I UNDERSTAND MY RESPONSIBILILLES UNDER THE NPDES CONSTRUCTION GENERAL PERMIT, AS REFERENCED ON THIS

TOM SPANGLER CLEANBAY BIOFUELS LLC. 726 SECOND STREET. UNIT 3B ANNAPOLIS, MD. 21403 410 514 6488

WETLAND CERTIFICATION:

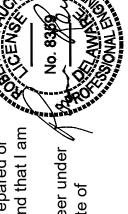
THIS PROPERTY, TAX MAP 113-6.00, HAS BEEN EXAMINED BY RAUCH INC. FOR THE PRESENCE OF WATERS OF THE UNITED STATES, INCLUDING WETLANDS (SECTION 404 AND SECTION 10), STATE SUBAQUEOUS LANDS AND STATE REGULATED WETLANDS AS ESTABLISHED BY THE REVIEWING AGENCIES IN THE FORM OF MANUALS, POLICIES AND PROCEDURES IN PLACE AT THE TIME THAT THE INVESTIGATION WAS CONDUCTED. THE WETLAND INFORMATION CONTAINED IN THIS PLAN SET IS IN ACCORDANCE WITH THIS CRITERIA PER STATE JD #XXXX AND/ OR ARMY CORPS JD #XXXX [AS APPLICABLE].

KYLE KOWALCYZK RAUCH INC 106 N HARRISON STREET EASTON, MD. 21601 410 770 9081

SITE DESIGNER'S CERTIFICATION:

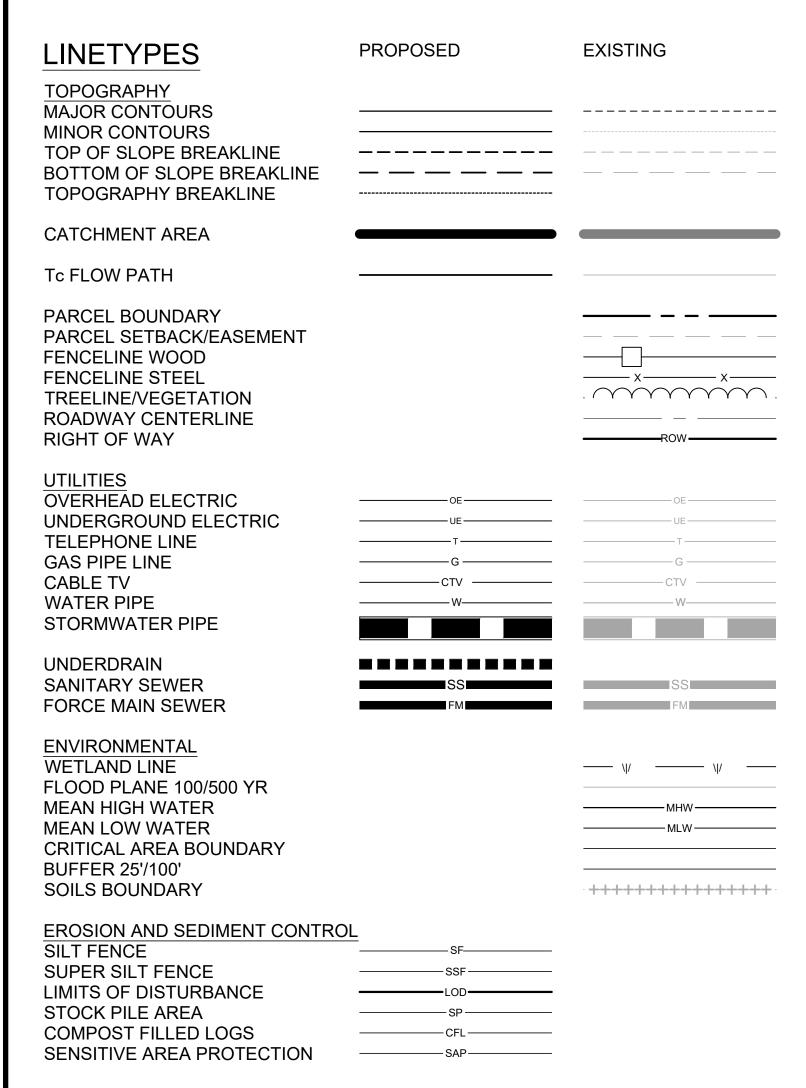
I HEREBY CERTIFY THAT THIS PLAN HAS BEEN PREPARED UNDER MY SUPERVISION AND TO THE BEST OF MY KNOWLEDGE COMPLIES WITH THE APPLICABLE STATE AND LOCAL REGULATIONS AND ORDINANCES.

ROBERT D RAUCH RAUCH INC 106 N HARRISON STREET EASTON, MD. 21601 410 770 9081



4/MAR/19 DATF. SCALE: DESIGNED BY: WJR APPROVED BY: SHEET NO.:

CB006-C-SW000



SYMBOLS	PROPOSED	EXISTING
FLARED END SECTION		
YARD INLET	YI	YI
CURB INLET	CI	
CUSTOM INLET	Cul	Cul
NYLOPLAST INLET	$\overline{\mathbb{Q}}$	
STORM DRAIN MANHOLE		
SANITARY SEWER MANHOLE	S	S
CLEANOUT		
GATE VALVE	GV ⋈	GV ⋈
SIGN		
WATER METER	<i>WM</i> O	WM
FIRE HYDRANT	b	b8⊲
WELL	W	W
BOLLARD	•	•
ELECTRIC TRANSFORMER	☐ ELEC	ELEC
TELEPHONE PEDESTAL	T	T
UTILITY POLE		
GUY WIRE	> —	>—
CONIFEROUS TREE	*	
DECIDUOUS TREE	5 C C C C C C C C C C C C C C C C C C C	
SPOT ELEVATION	X 39.01	× 39.00
LAND SLOPE	-0.41%	-0.41%
BENCHMARK	ELEV: 123	

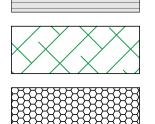
BOREHOLE/MONITORING WELL +

HATCHES BRICK PAVING EXISTING,TO BE DEMOLISHED STEEP SLOPES

ASPHALT	
GRAVEL	44 4
CONCRETE	
RIPRAP/ROCK	
ESD/LID/BMP BED BOTTOM	+ + + + + + + + + + + + + + + + + + +
PRETREATMENT FILTER STRIP	
ROOFTOP/NON-ROOFTOP DISCONNECT	
BRICK PAVING	

IMPERVIOUS AREA
WOODED/FOREST





ABBREVIATIONS

			ADMINISTRATION
ADA	AMERICANS WITH DISABILITIES ACT	MUTCD	MANUAL ON UNIFORM TRAFFIC
AGIP	AT GRADE INLET PROTECTION		CONTROL DEVICES
@	AT	N	NORTH
BC	BACK OF CURB	NO	NUMBER
ВМ	BENCHMARK	PC	POINT OF CURVATURE
BRL	BUILDING RESTRICTION LINE	PE	POLYETHYLENE
CES	CONCRETE END SECTION	PERF	PERFORATED
C/O	CLEAN OUT	PR	PROPOSED
CHRD		PT	POINT OF TANGENCY
CI	CURB INLET	PVC	POLYVINYL CHLORIDE
CIP	CURB INLET PROTECTION	PVI	POINT OF VERTICAL INTERSECTION
CL	CENERLINE	Q	DISCHARGE
CMP	CORRUGATED METAL PIPE	R	
Cul	CUSTOM INLET		RADIUS
		RCP	REINFORCED CONCRETE PIPE
CY	CUBIC YARD	RCN	RUNOFF CURVE NUMBER
DEL D		ROW	RIGHT-OF-WAY
DI	TRANSPORTATION	RT	RIGHT
DI	DUCTILE IRON	S	SOUTH
Ε	EAST	SCE	STABILIZED CONSTRUCTION
EL/ELE			ENTRANCE
EP	EDGE OF PAVE	SD	STORM DRAIN
ESC	EROSION AND SEDIMENT CONTROL	SDMH	STORM DRAIN MANHOLE
EX	EXISTING	Sf	FRICTION SLOPE
FES	FLARED END SECTION	SF	SILT FENCE
FFE	FINISHED FLOOR ELEVATION	SHA	STATE HIGHWAY ADMINISTRATION
FH	FIRE HYDRANT	SIP	STANDARD INLET PROTECTION
FL	FLOW LINE	SQ	SQUARE
FT	FOOT	SS	SANITARY SEWER
GFA	GROSS FLOOR AREA	SSF	SUPER SILT FENCE
GPD	GALLONS PER DAY	SSMH	SANITARY SEWER MANHOLE
GV	GATE VALVE	STA	STATION
HDPE	HIGH DENSITY POLYETHYLENE	STD	STANDARD
1	INLET	SWM	STORM WATER MANAGEMENT
INV	INVERT	TAN	TANGENT
KV	KILOVOLTS	ТВ	THRUST BLOCK
L	ARC LENGTH	TBR	TO BE REMOVED
LF	LINEAR FEET	TC	TOP OF CURB
LOD	LIMITS OF DISTURBED AREA	Tc	TIME OF CONCENTRATION
LT	LEFT	TP	TAX PARCEL
MAX	MAXIMUM	TYP	TYPICAL
MBR	MICRO-BIORETENTION	UE	UNDERGROUND ELECTRIC
MD	MARYLAND	UL	UNDERGROUND LIGHT CABLE
MEP	MECHANICAL, ELECTRICAL, AND	UT	UNDERGROUND TELEPHONE
1VIL-1	PLUMBING	V	
МН	MANHOLE		VELOCITY
MHW	MEAN HIGH WATER	W M	WEST
MIN	MINIMUM	W/	WITH
(VIII N	IVIII VIIVI OIVI		



MEAN LOW WATER

MARYLAND STATE HIGHWAY

ANBAY

4/MAR/2019 DATE: SCALE: DRAWN BY: WJR

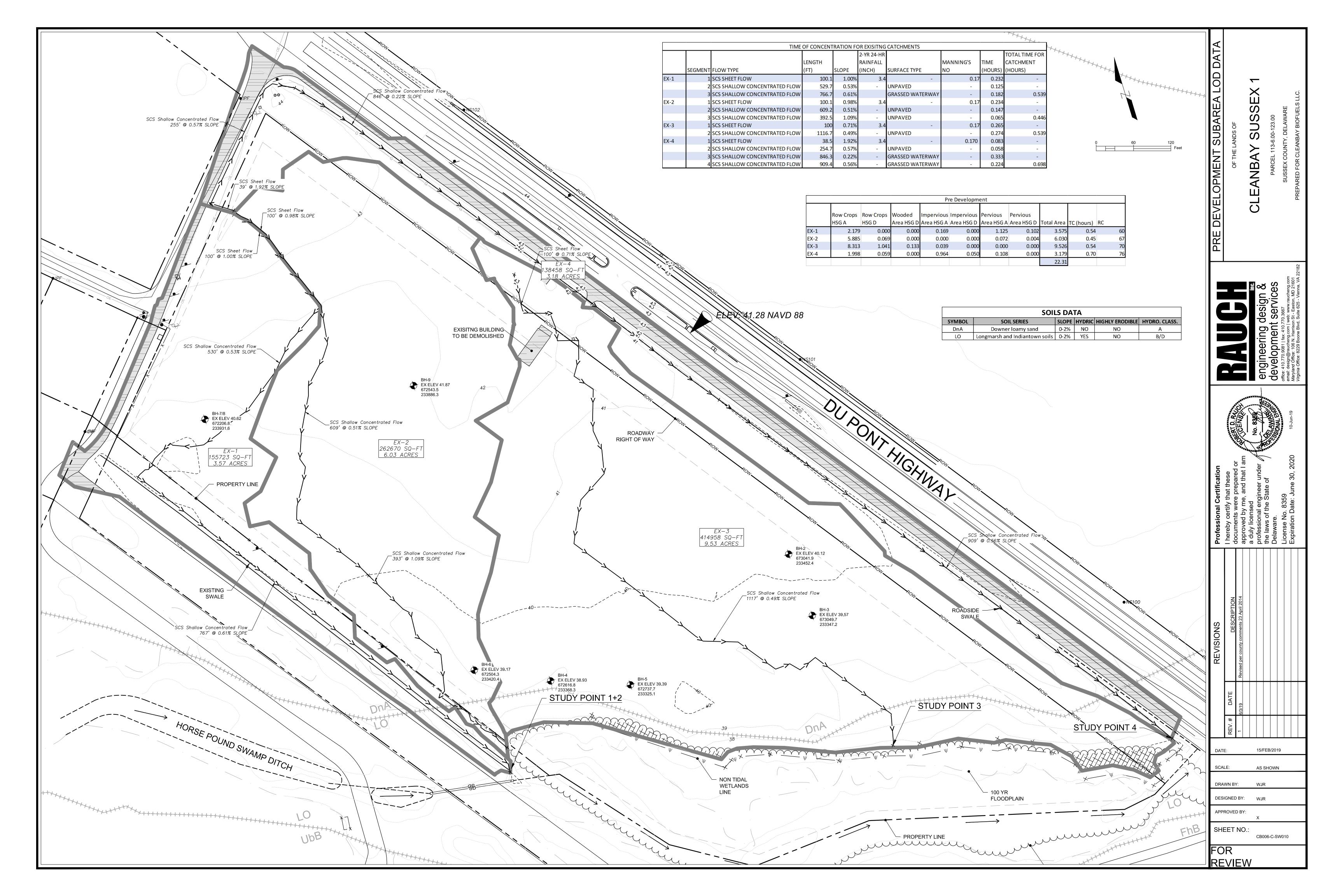
DESIGNED BY: WJR

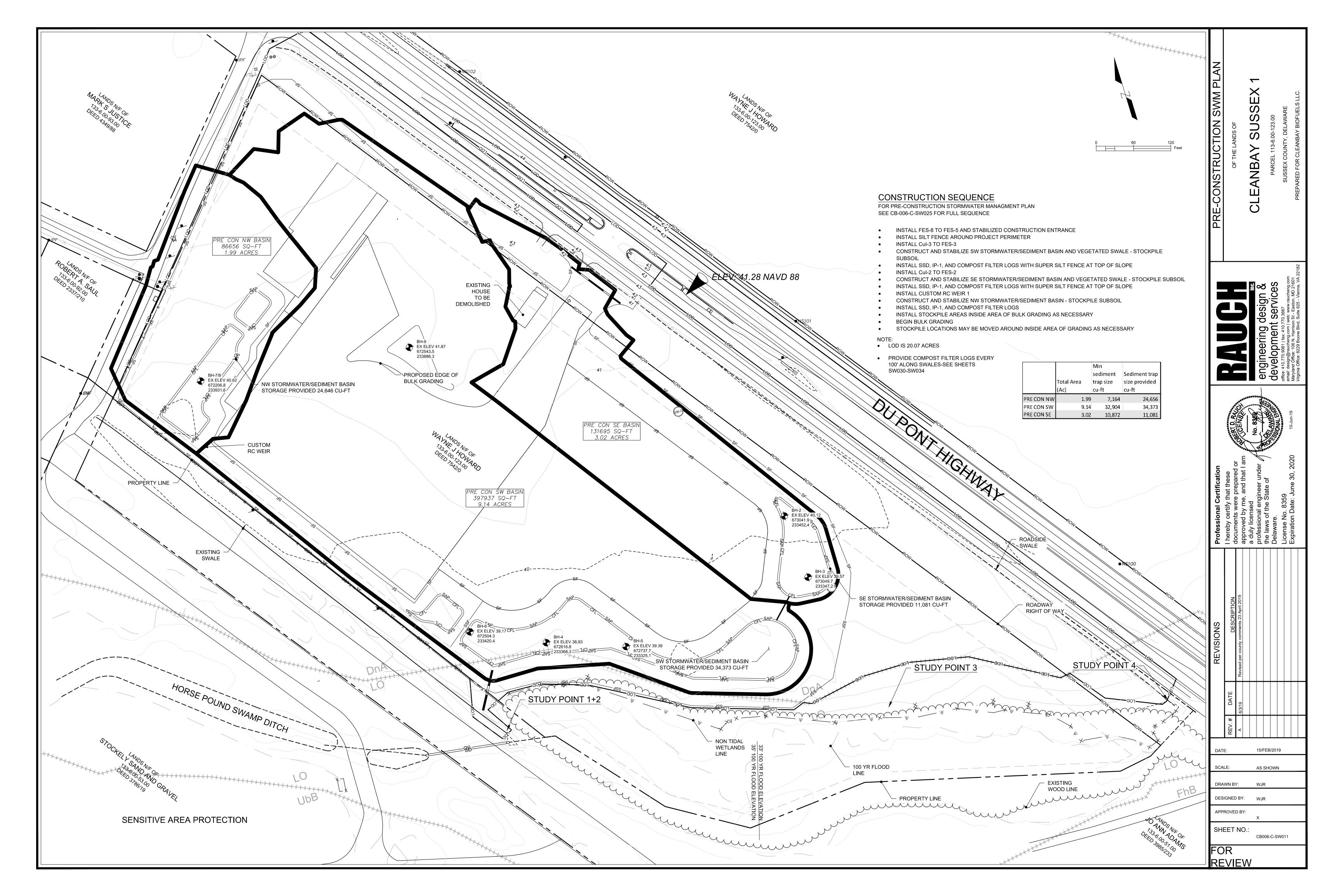
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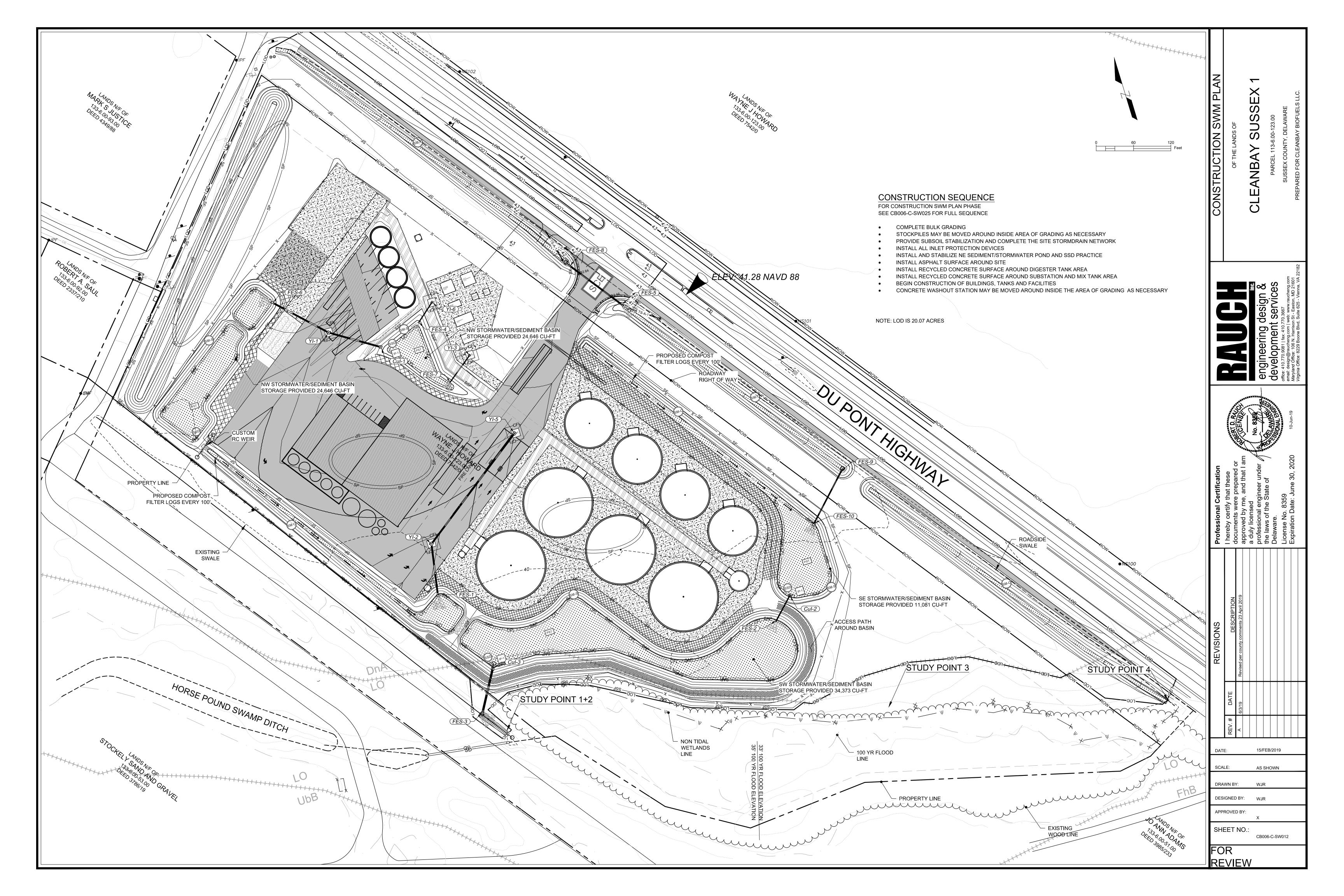
APPROVED BY:

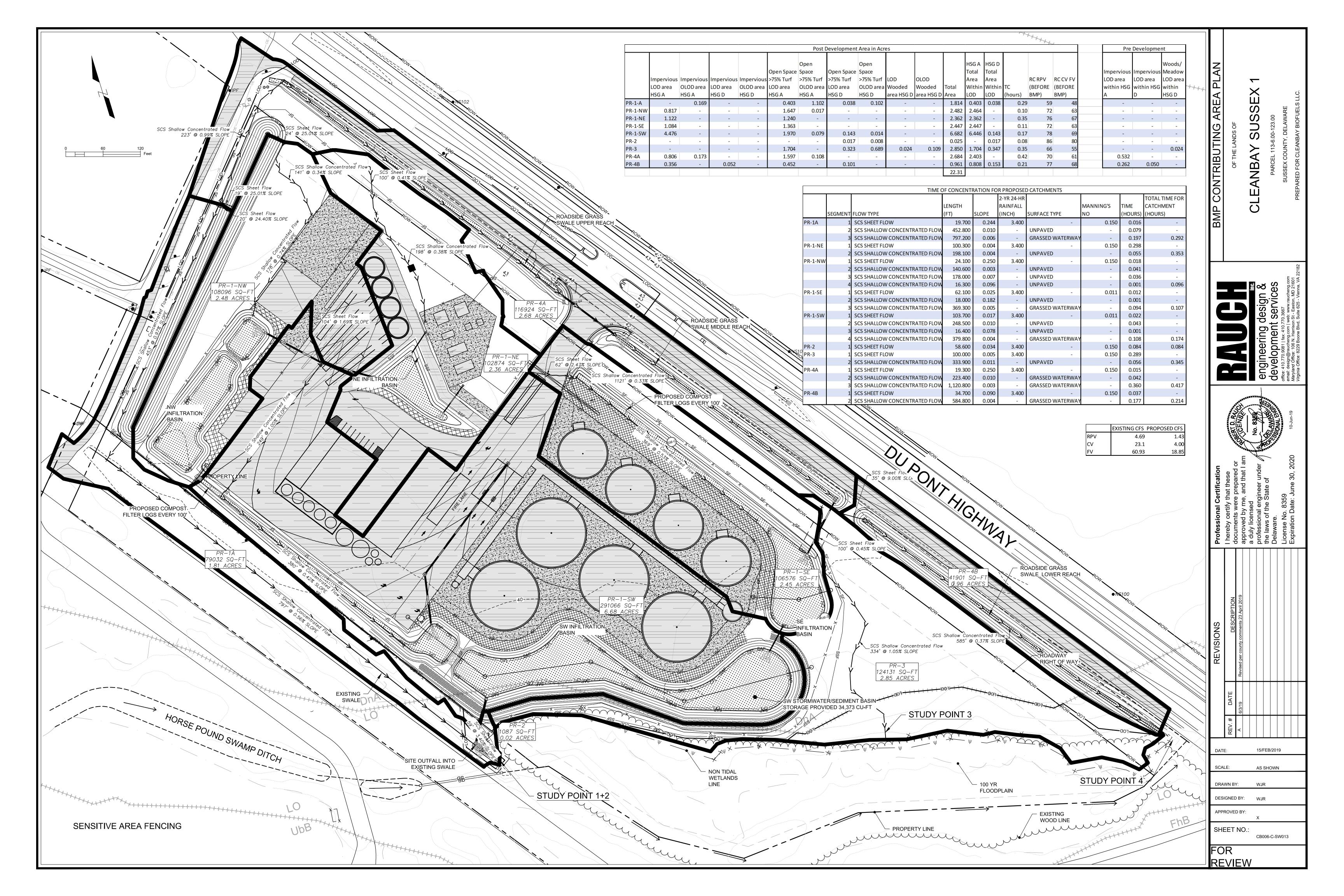
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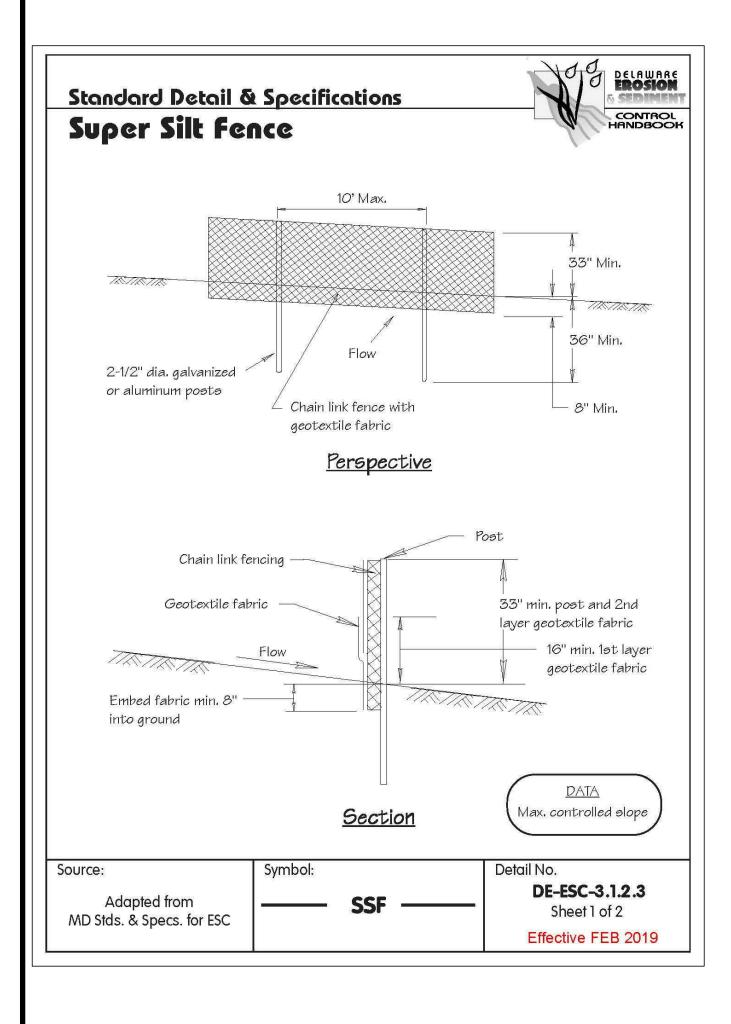
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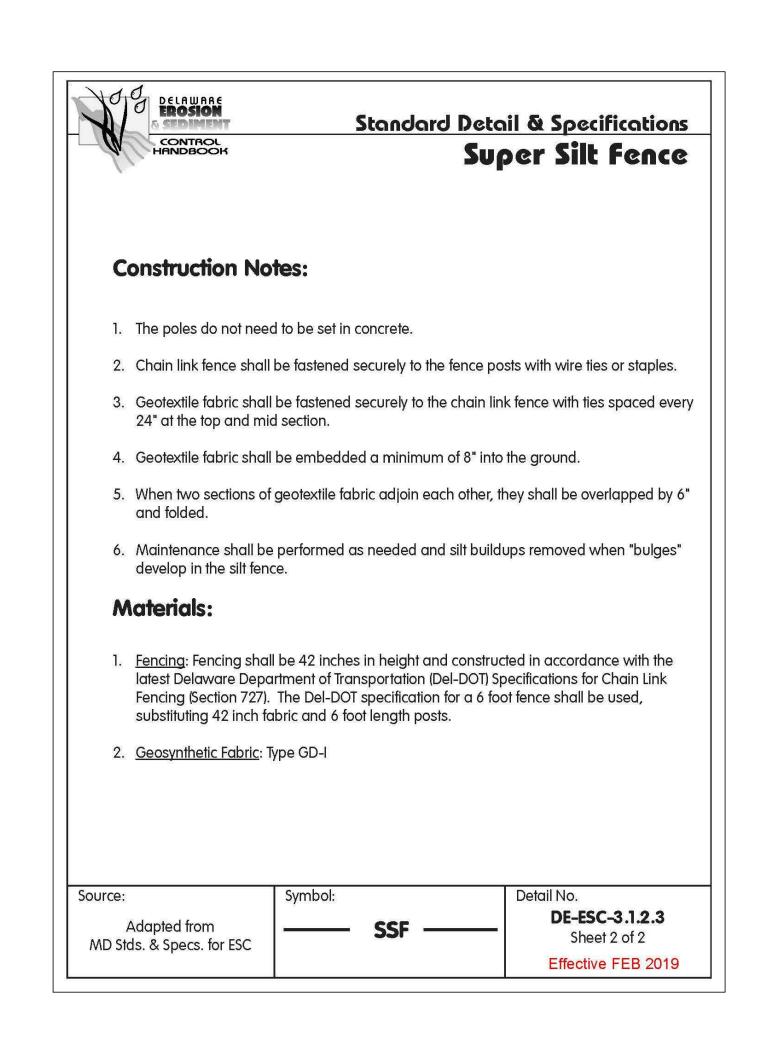


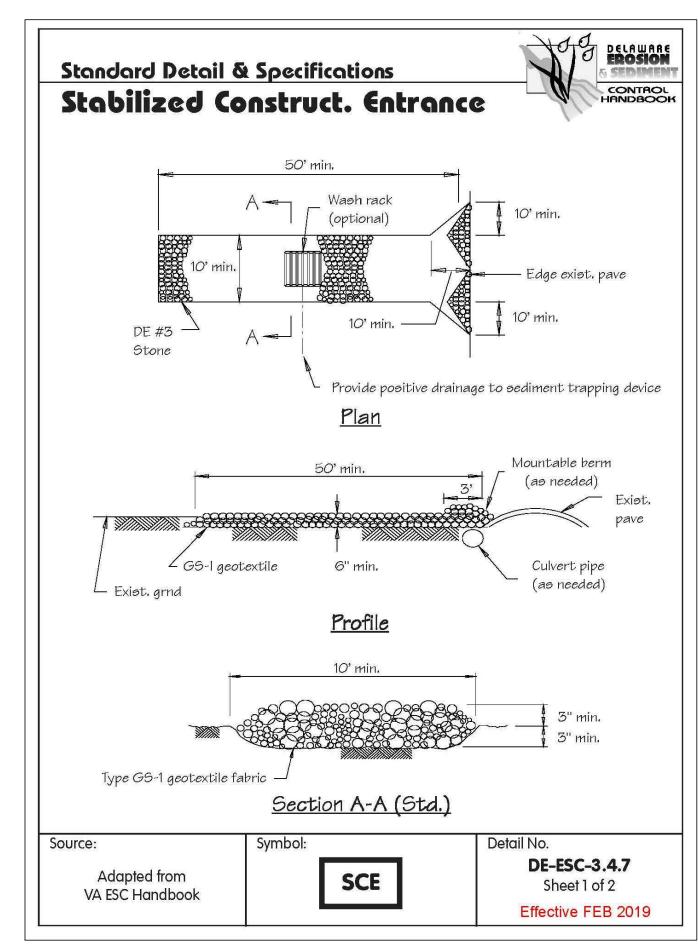


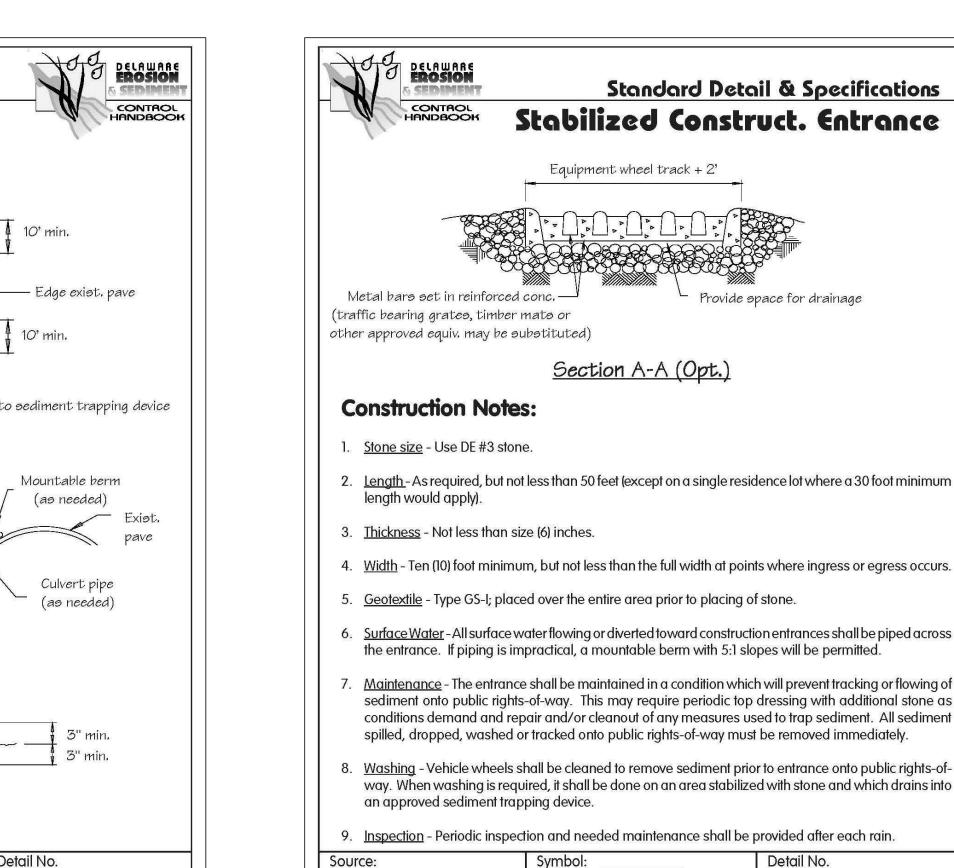


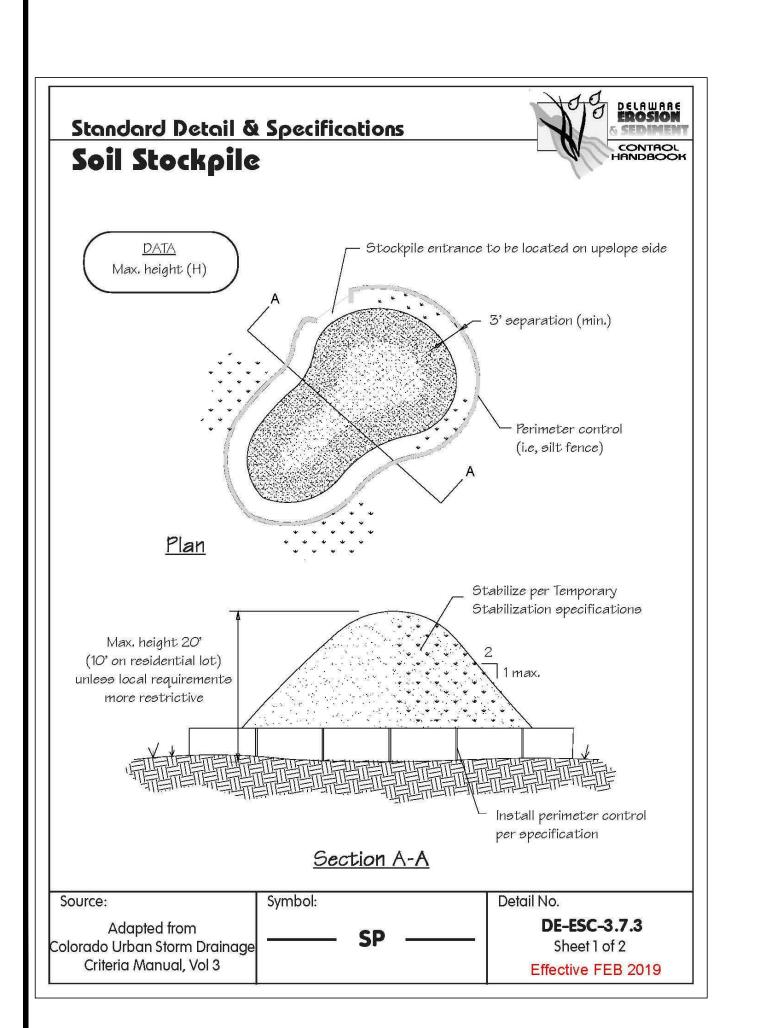


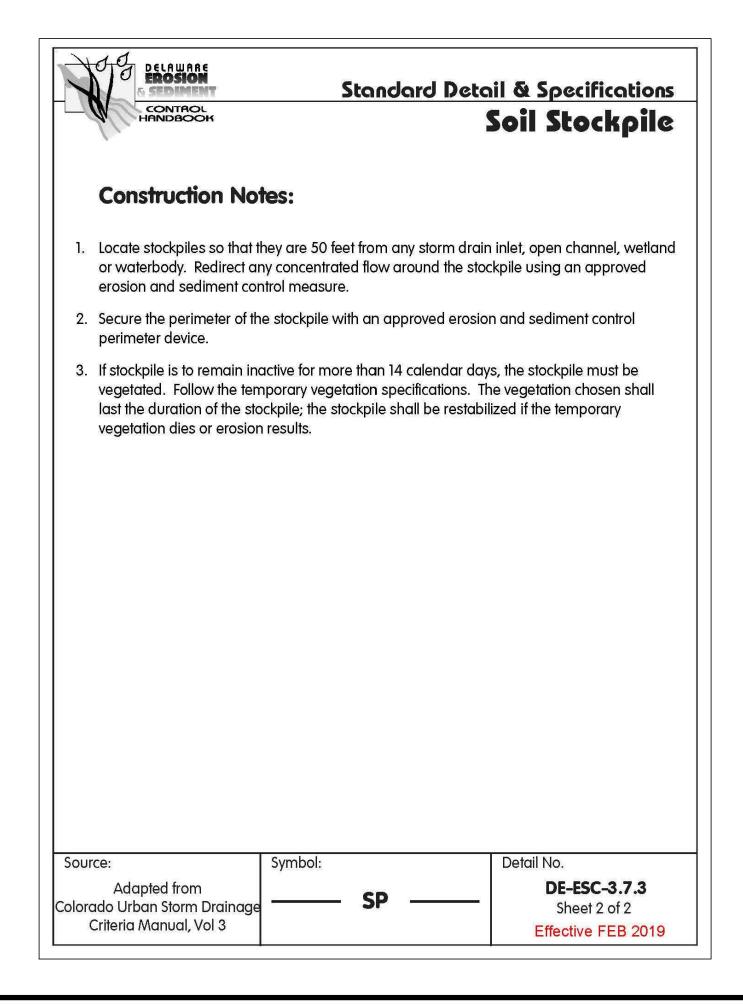


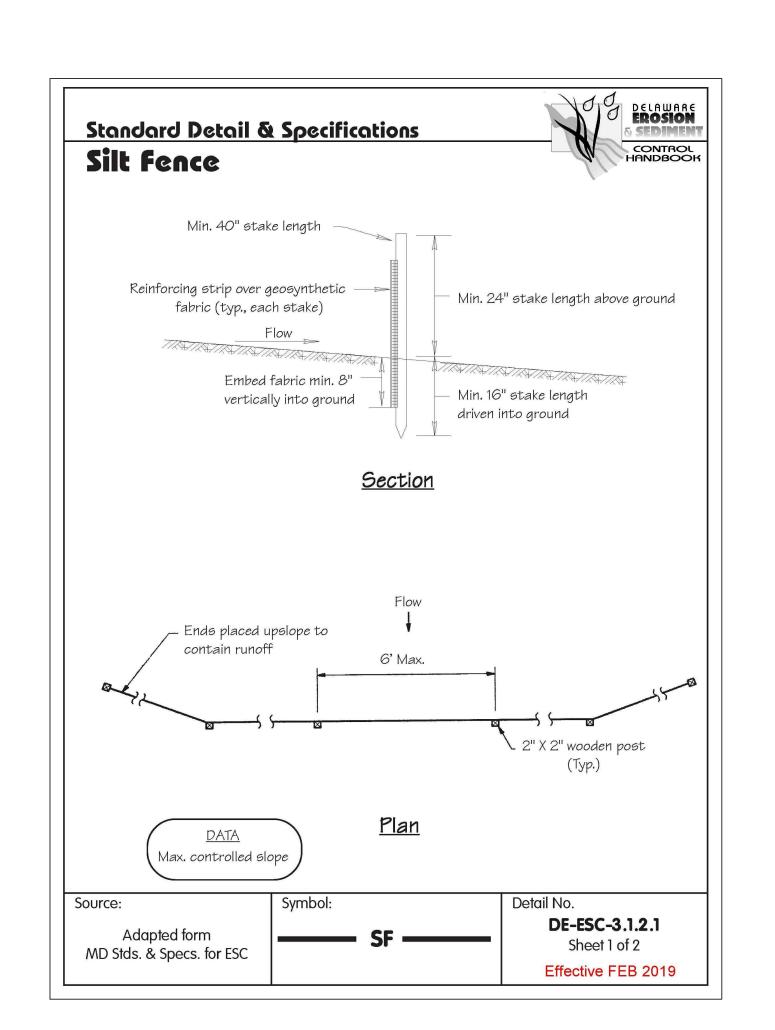


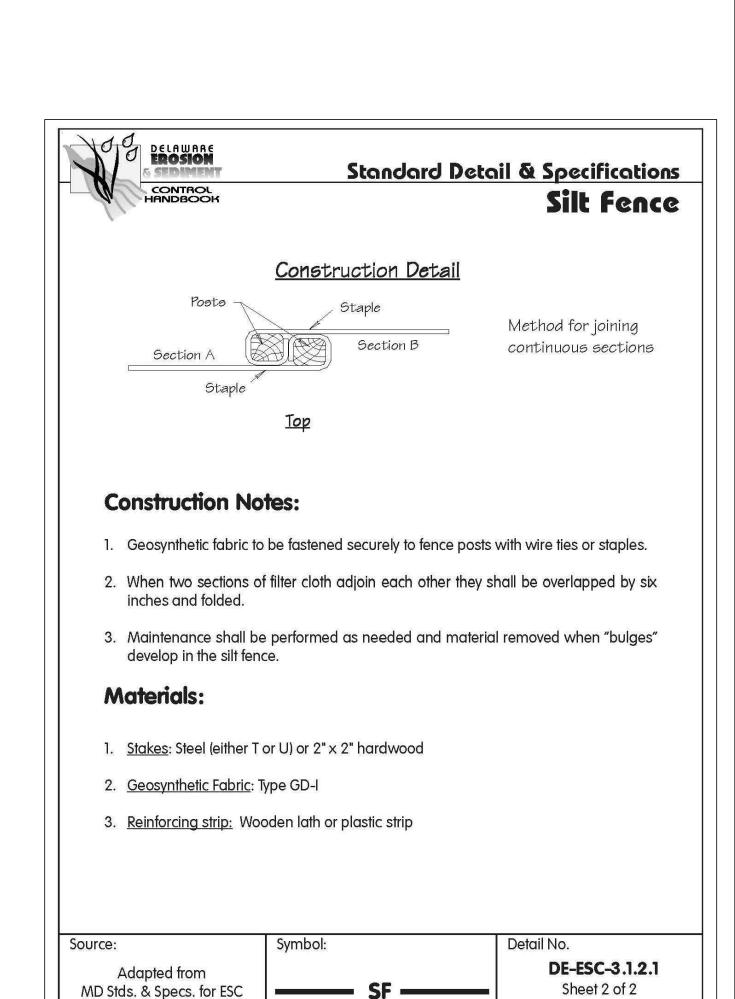


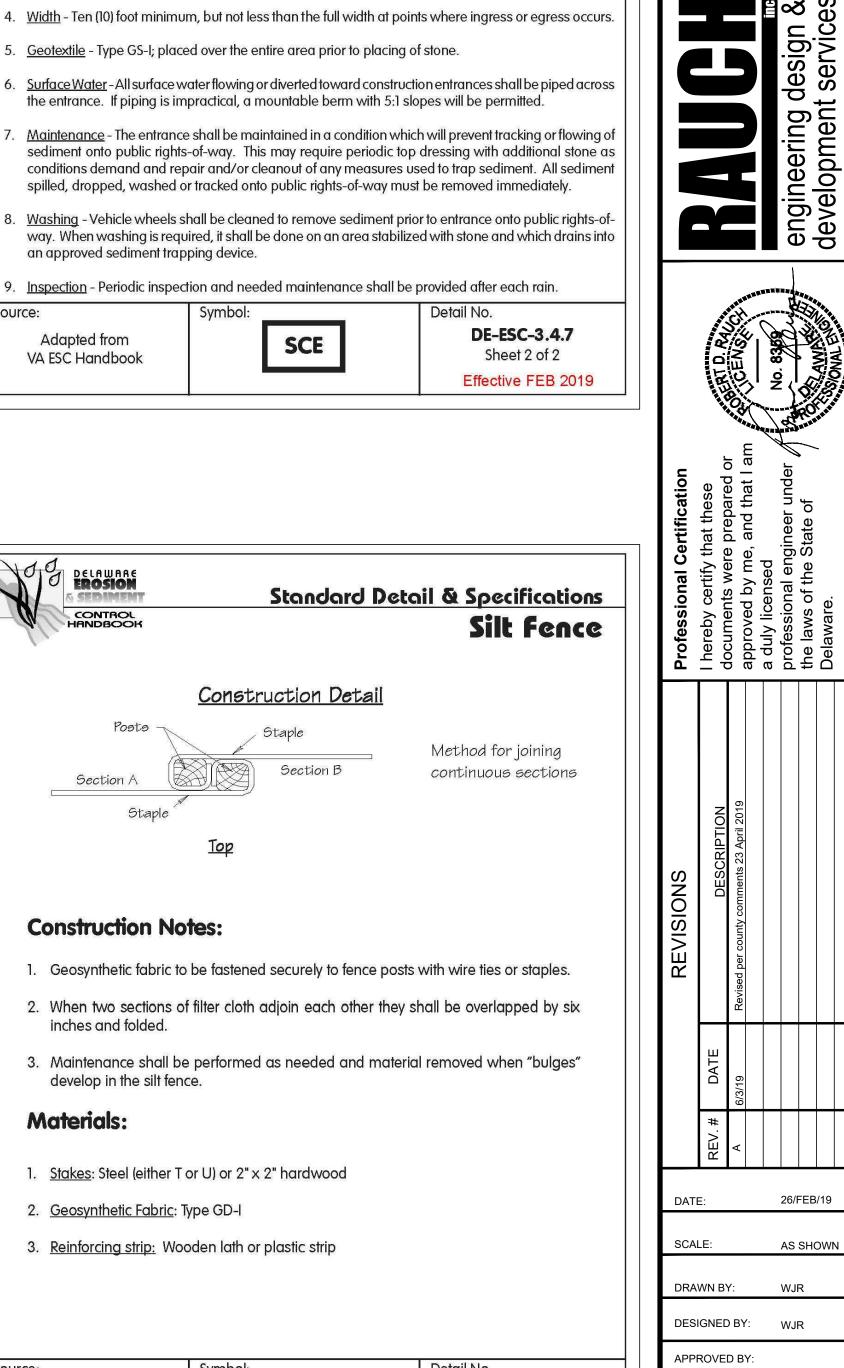












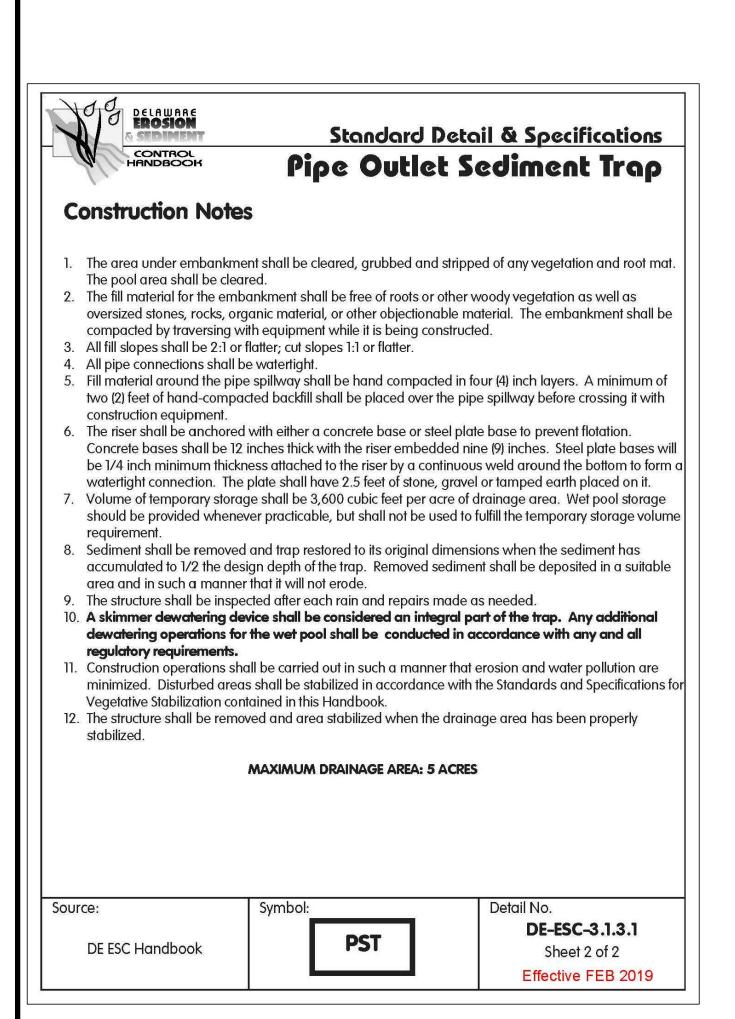
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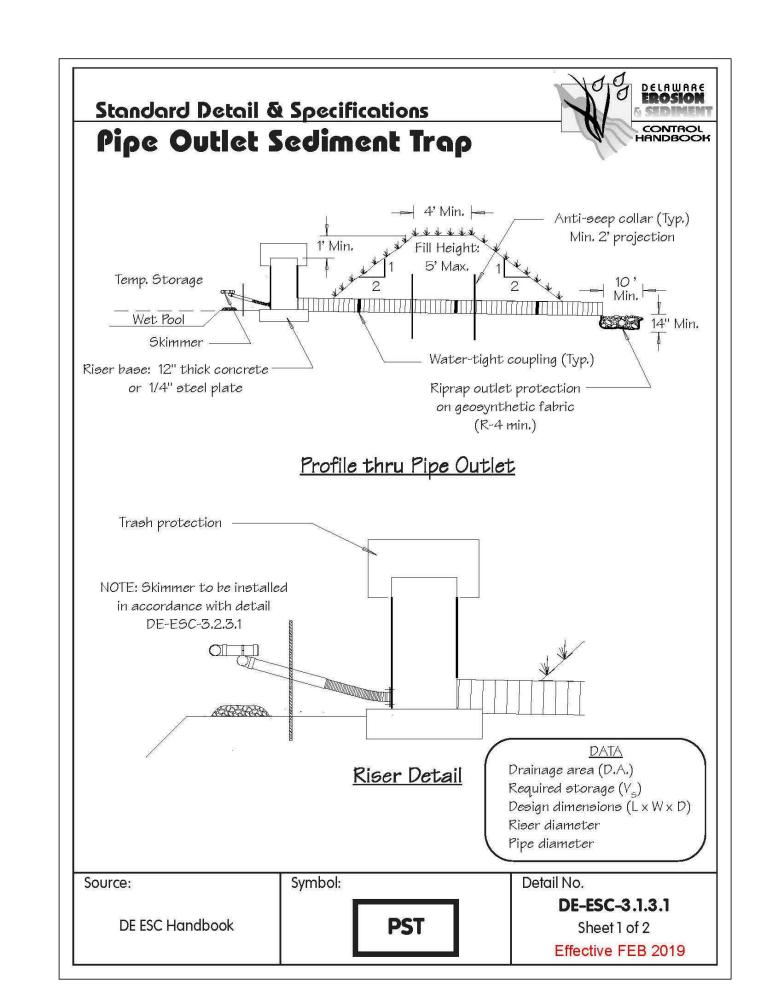
SHEET NO .:

REVIEW

Effective FEB 2019

CB006-C-SW020





Construction Notes:

Materials:

against it.

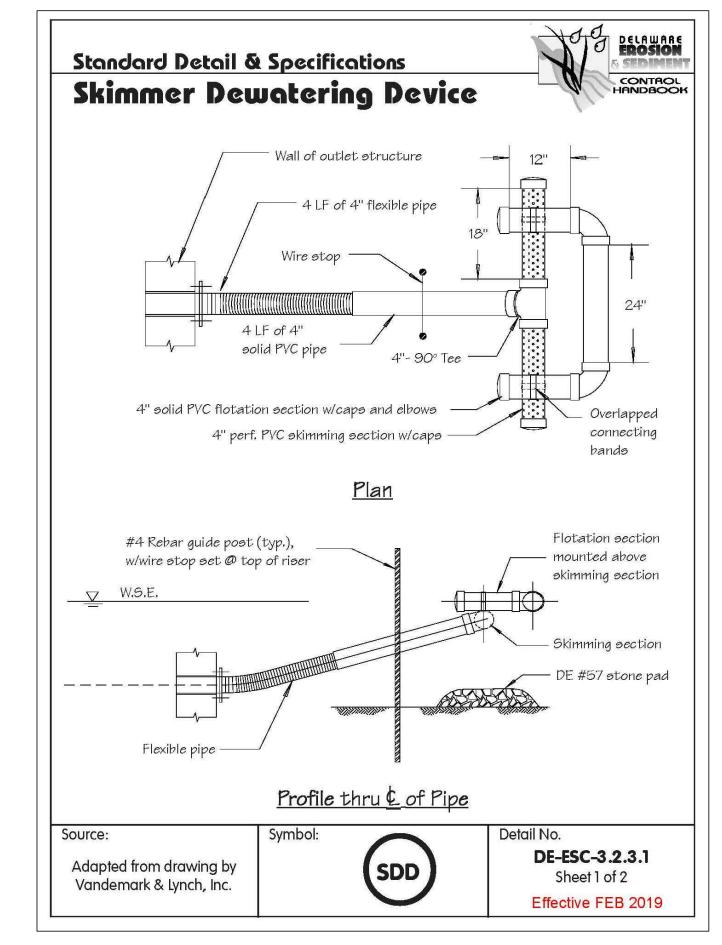
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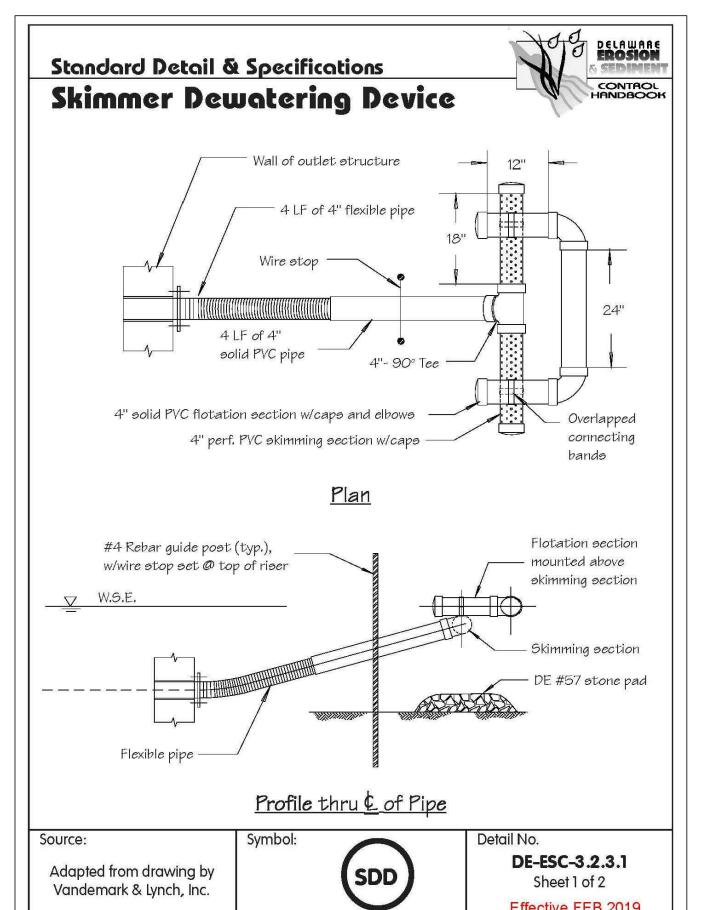
Erosion Draw Manual

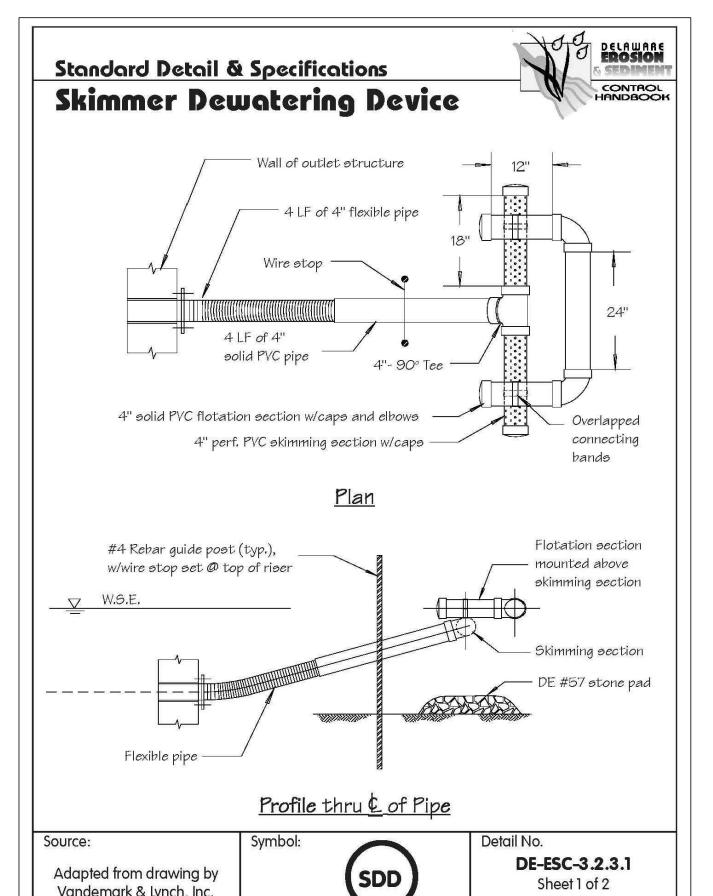
J. McCullah & Assoc.

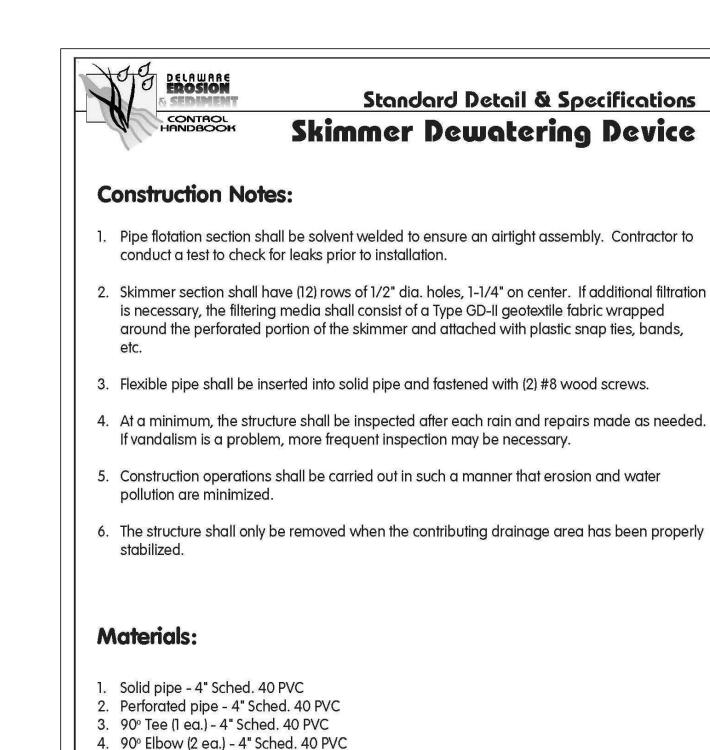
Source:

3. Geotextile fabric: Type GD-II









5. Cap (4 ea.) - 4" Sched. 40 PVC, solid

Source:

Delaware ESC Handbook

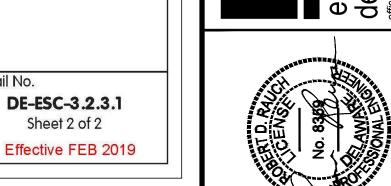
DELAWARE EROSION

Construction Notes:

impede normal flow.

6. Flexible pipe - 4" corrugated plastic tubing (non-perforated)

SDD



DE-ESC-3.2.3.1

Sheet 2 of 2

Standard Detail & Specifications

Veg. Channel - Triang./Trap.

4. All earth removed and not needed in construction shall be spread or disposed of so that it will not interfere with the functioning of the waterway. Stabilization shall be done in accordance with the appropriate Standard and Specifications for Vegetative Stabilization and Stabilization Mat.

a. It is recommended that, when conditions permit, temporary diversions or other means

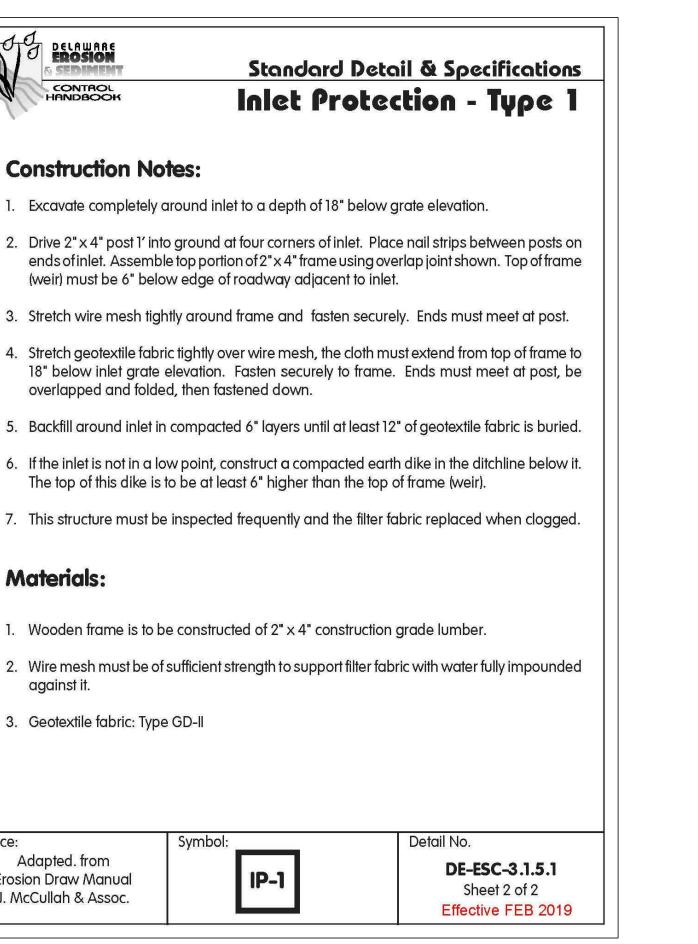
The channel shall be excavated or shaped to line, grade, and cross section as required to meet

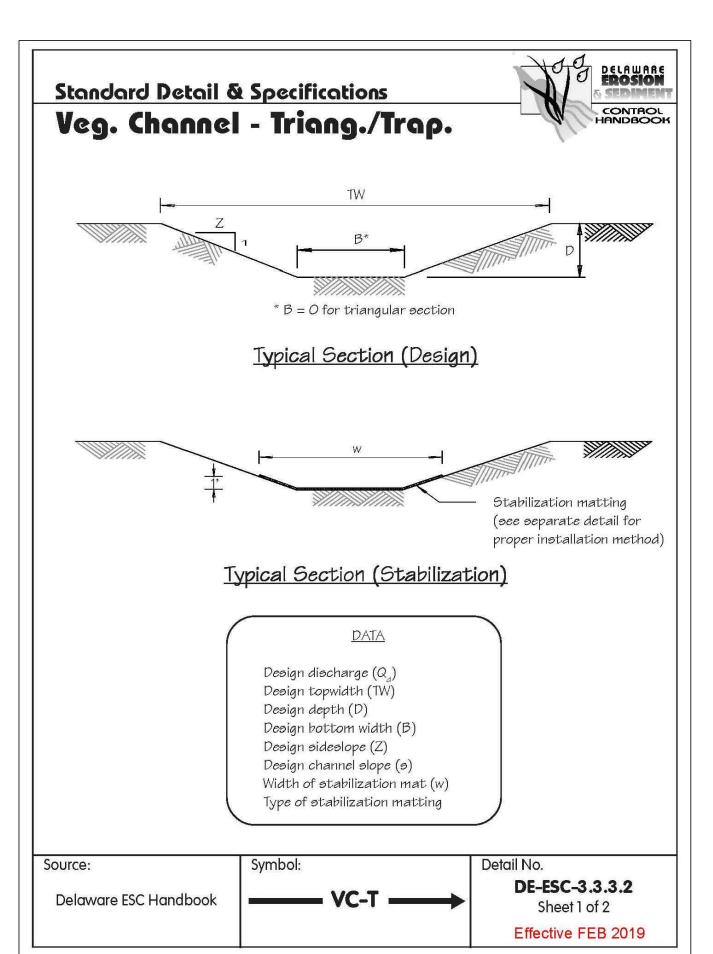
disposed of so as not to interfere with the proper functioning of the waterway.

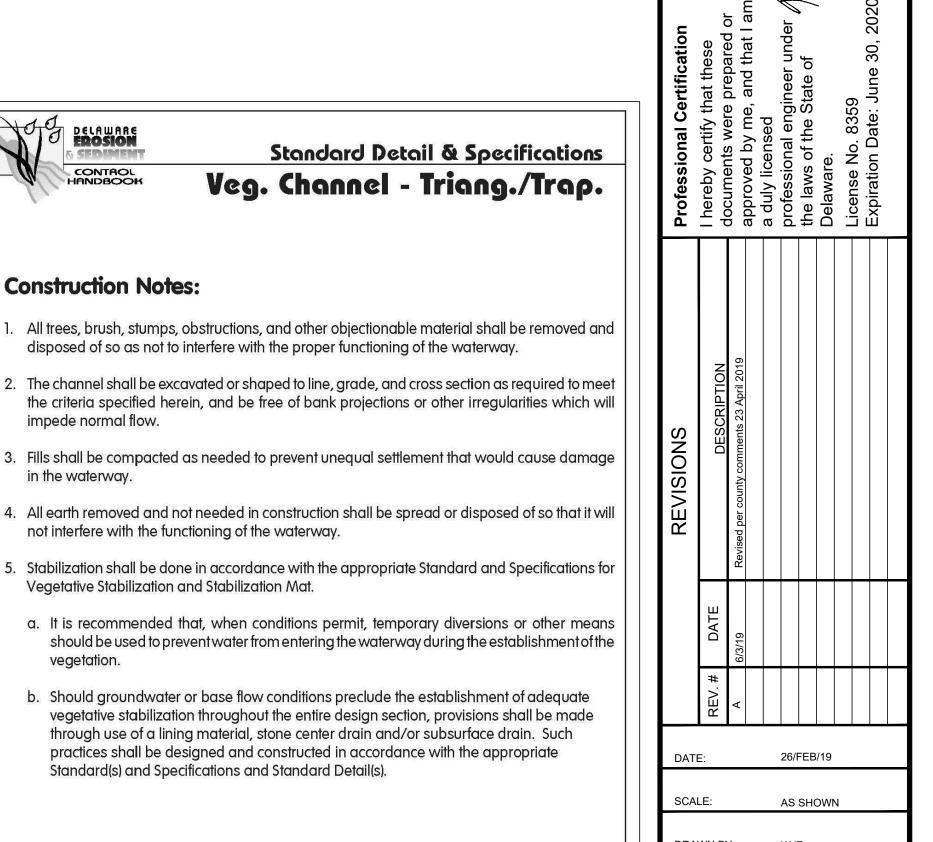
should be used to prevent water from entering the waterway during the establishment of the

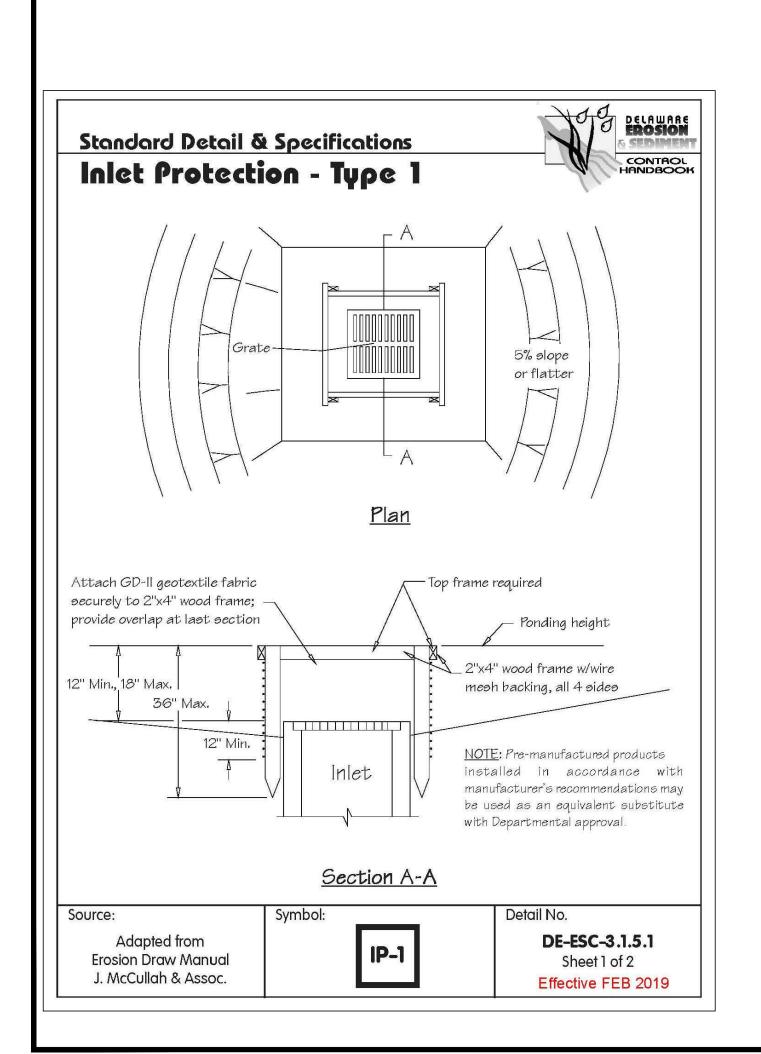
b. Should groundwater or base flow conditions preclude the establishment of adequate vegetative stabilization throughout the entire design section, provisions shall be made through use of a lining material, stone center drain and/or subsurface drain. Such practices shall be designed and constructed in accordance with the appropriate Standard(s) and Specifications and Standard Detail(s).

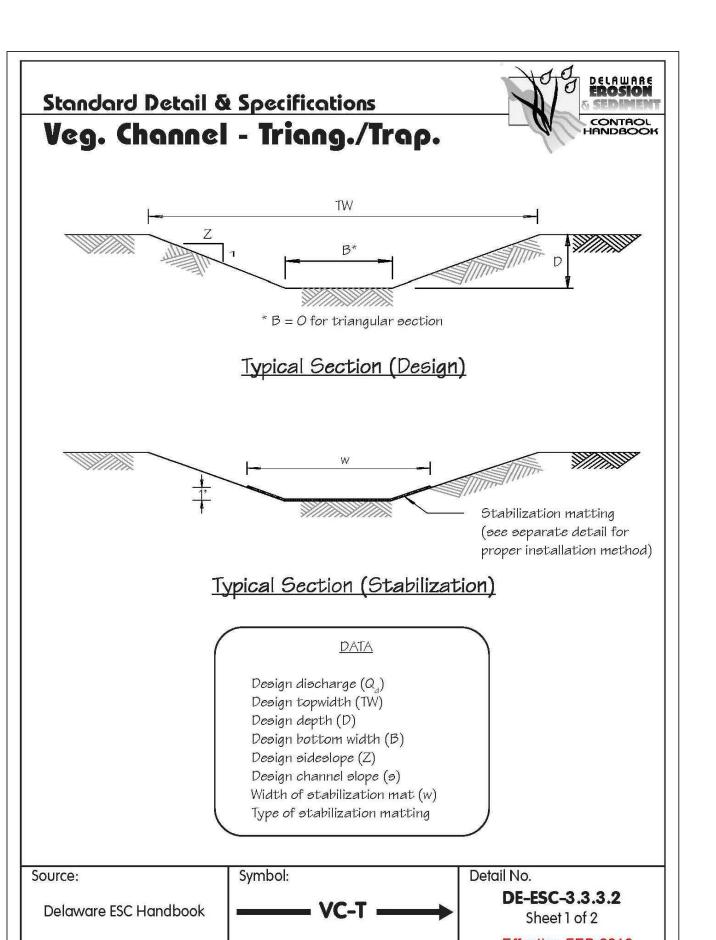
Source: Symbol: Detail No. DE-ESC-3.3.3.2 Delaware ESC Handbook Sheet 2 of 2 Effective FEB 2019







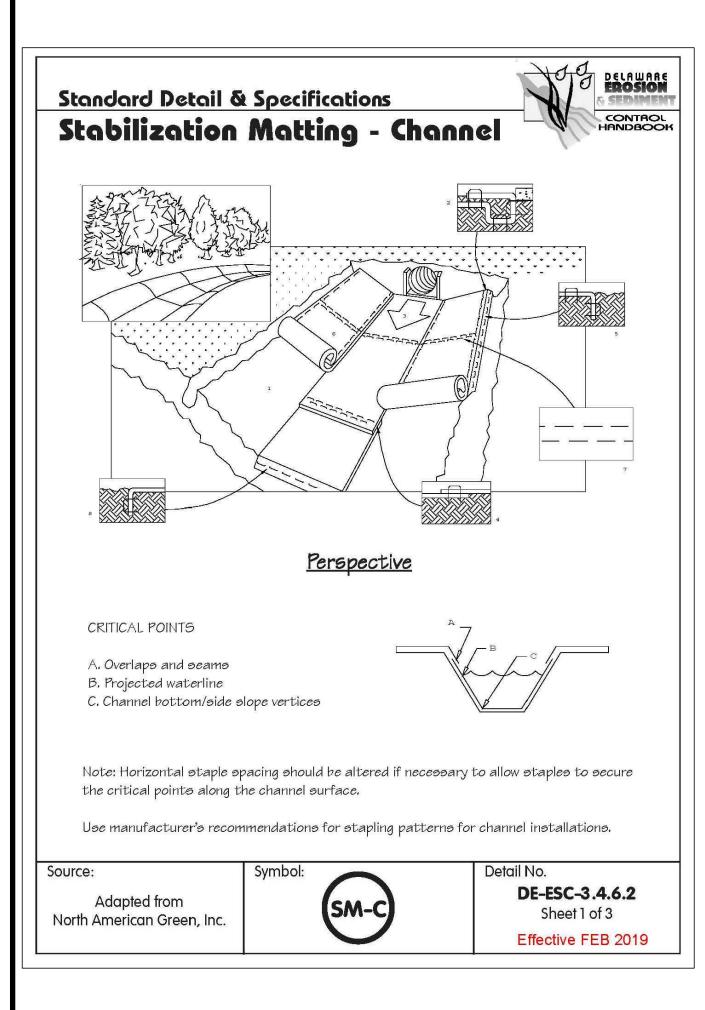


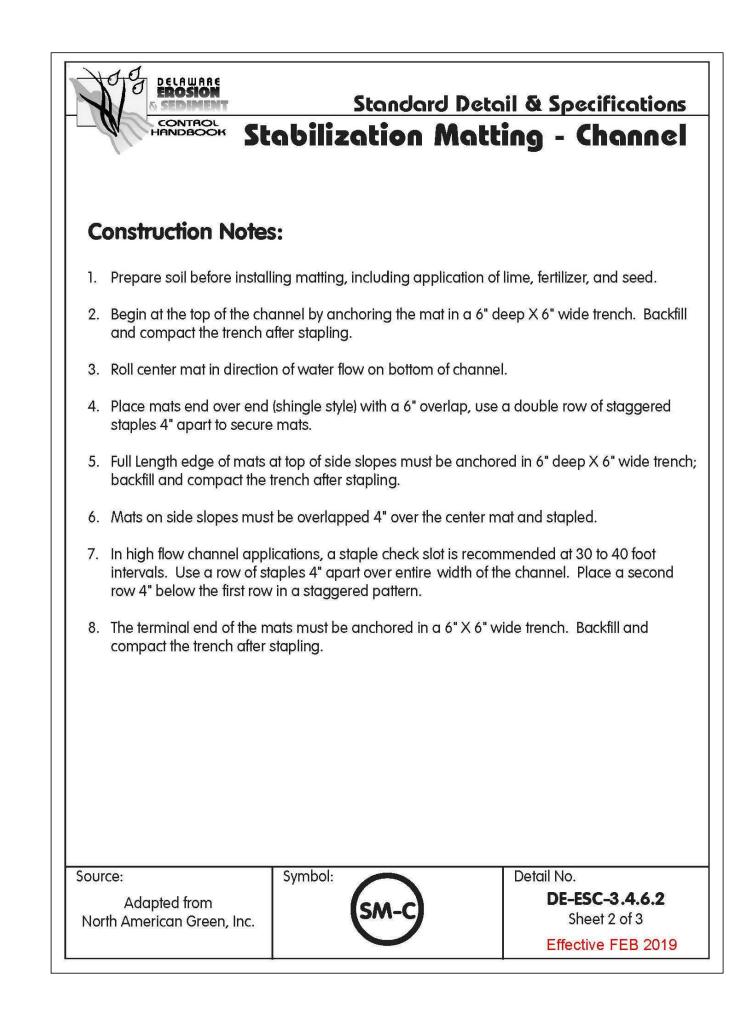


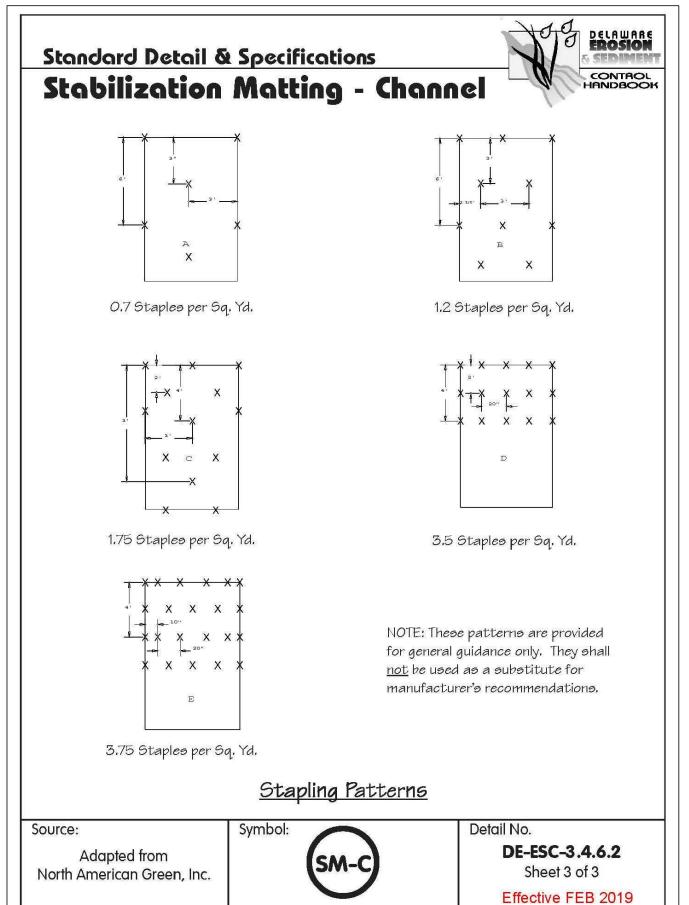


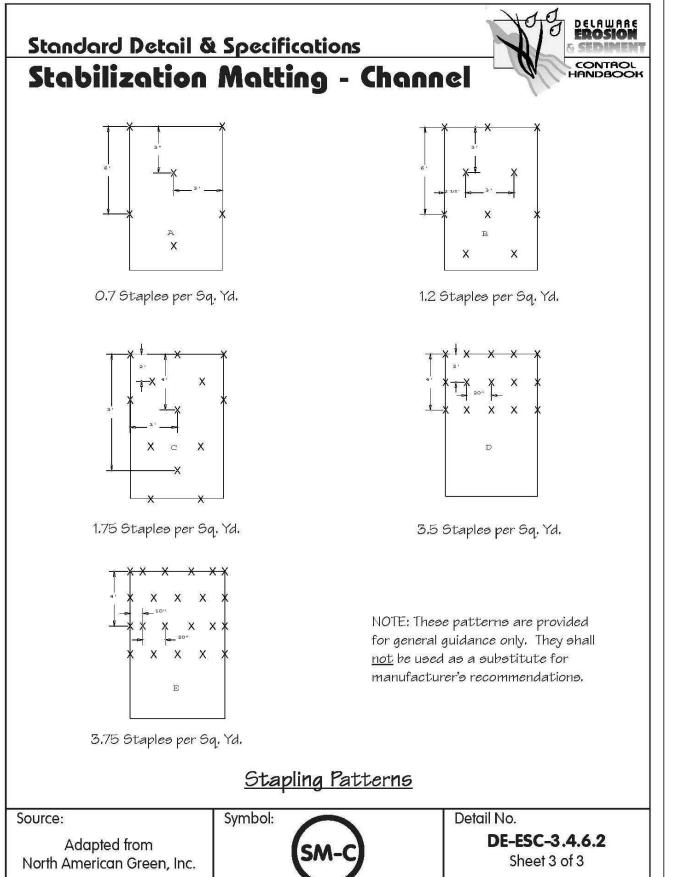
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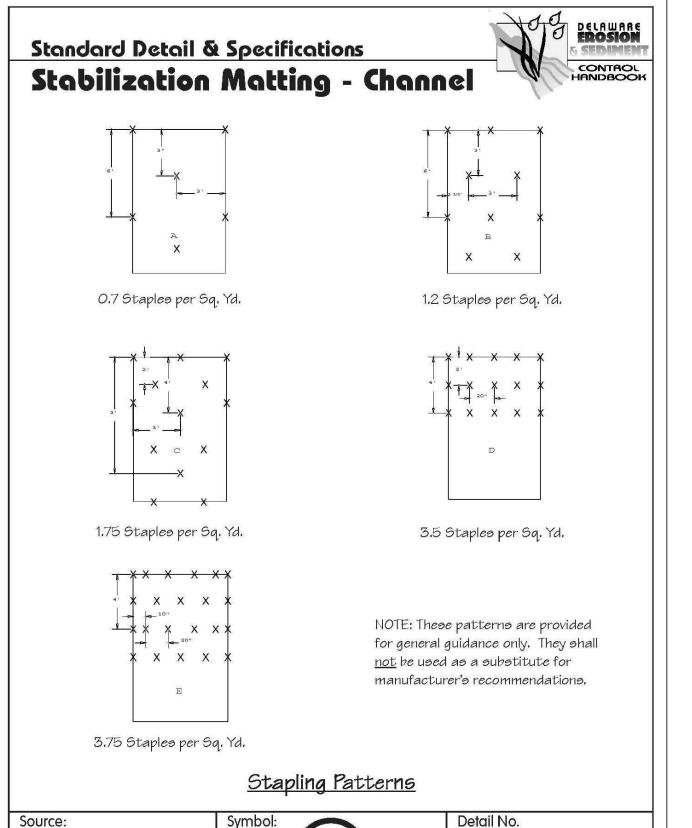
APPROVED BY: SHEET NO .: CB006-C-SW021

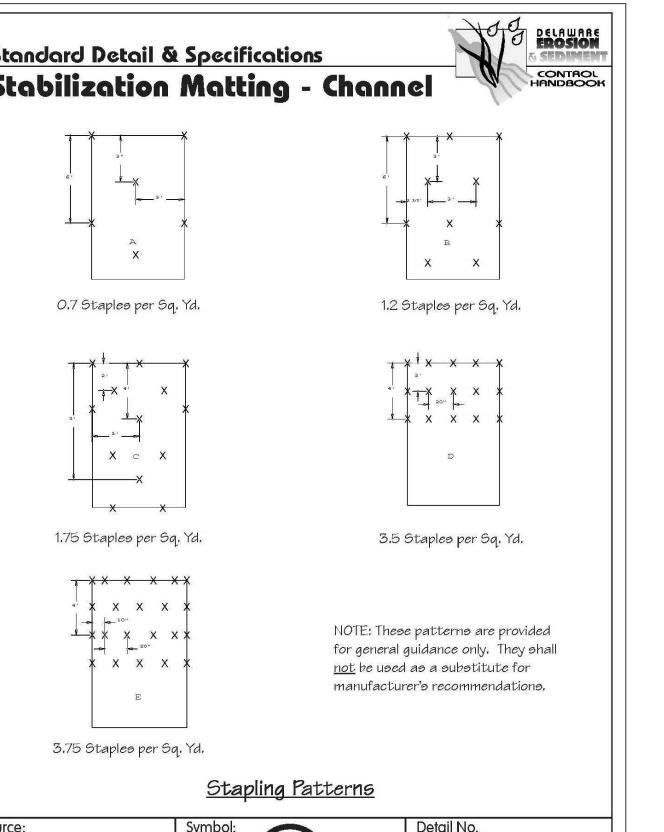












Source: Symbol: Detail No. **DE-ESC-3.4.5** Delaware ESC Handbook Sheet 1 of 3 & Filtrexx™ International Effective FEB 2019

a. Straw - Straw shall be unrotted small grain straw applied at the rate of 1-1/2 to 2 tons per acre, or 70 to 90

b. Wood chips - Apply at the rate of approximately 6 tons per acre or 275 pounds per 1,000 square feet when

c. Hydraulically applied mulch-The following conditions apply to hydraulically applied mulch:

available and when feasible. These are particularly well suited for utility and road rights-of-way. If wood

chips are used, increase the application rate of nitrogen fertilizer by 20 pounds of N per acre (200 pounds

a. Wood fiber mulch shall consist of specially prepared wood that has been processed to a uniform state, is packaged for sale as a hydraulic mulch for use with hydraulic seeding

b. Blended fiber mulch shall consist of any hydraulic mulch that contains greater than 30% paper fiber. The paper component must consist of specially prepared paper that has been processed

c. A bonded fiber matrix (BFM) consists of long strand, specially prepared wood fibers that have

d. Refer to Figure 3.4.5a for conditions and limitations of use for each of the above categories of

ii. All components of the hydraulically applied mulches shall be pre-packaged by the manufacturer to

iii. Hydraulic mulches shall be applied with a viable seed and at manufacturer's recommended rates.

iv. Hydraulically applied mulches and additives shall be mixed according to manufacturers

iv. Materials within this category shall only be used when hydraulically applied mulch has been specified for use on the approved Sediment and Stormwater Plan, or supplemental approval from the plan

assure material performance. Field mixing of the mulch components is acceptable, but must be done

equipment, and consists of a minimum of 70% virgin or recycled wood fiber combined with

to a uniform fibrous state and is packaged for sale as a hydraulic mulch for use with hydraulic

been processed to a uniform state held together by a water resistant bonding agent. BFMs

shall contain no paper (cellulose) mulch but may contain small percentages of synthetic fibers

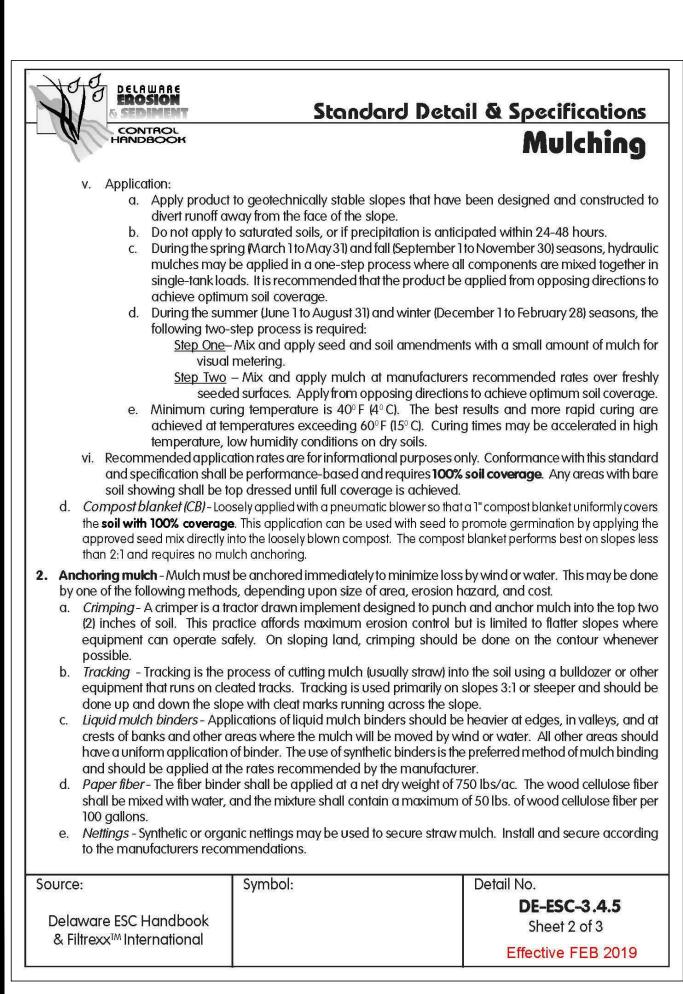
feet sections and place 70-90 pounds (two bales) of mulch in each section.

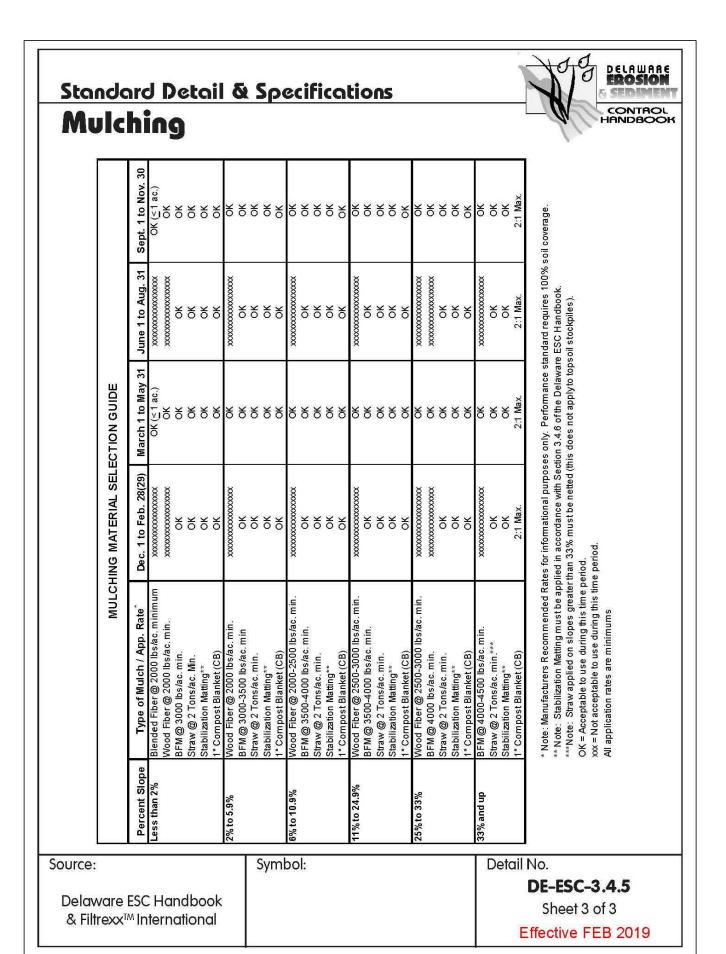
per manufacturers recommendations to ensure the proper results.

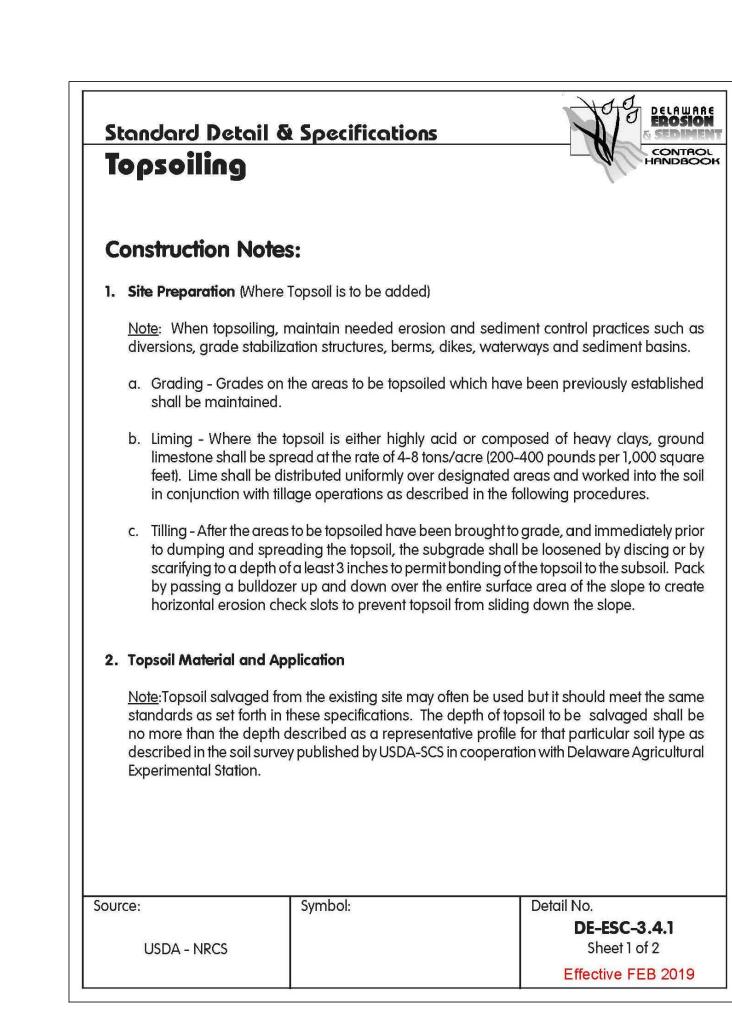
approval agency has been obtained in writing for a specific area.

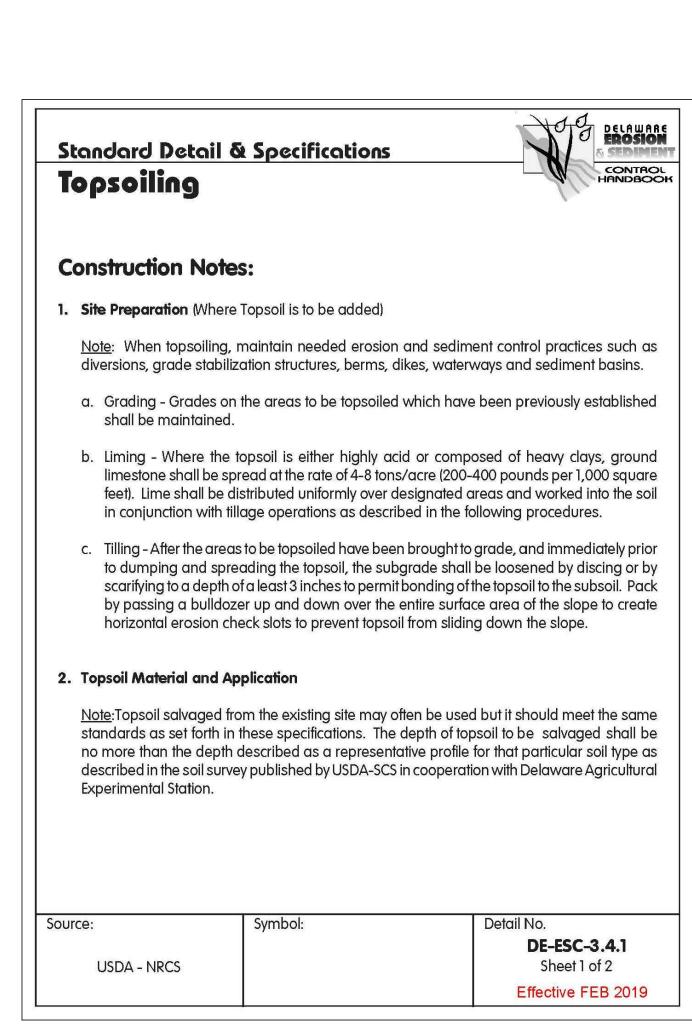
Increased rates may be necessary based on site conditions.

pounds (two bales) per 1,000 square feet. Mulch materials shall be relatively free of weeds and shall be free of noxious weeds such as; thistles, Johnsongrass, and quackgrass. Spread mulch uniformly by hand or mechanically. For uniform distribution of hand spread mulch, divide area into approximately 1,000 square











Standard Detail & Specifications Topsoiling

Construction Notes (cont.)

Standard Detail & Specifications

of 10-10-10 or 66 pounds of 30-0-0 per acre).

seeding equipment.

to enhance performance.

30% paper fiber and additives.

Mulching

Materials and Amounts

a. Materials - Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand or other soil as approved by an agronomist or soil scientist. It shall not have a mixture of contrasting textured subsoil and contain no more than 5 percent by volume of cinders, stones, slag, coarse fragment, gravel, sticks, roots, trash or other extraneous materials larger than 1-1/2 inches in diameter. Topsoil must be free of plants or plant parts of bermudagrass, quackgrass, Johnsongrass, nutsedge, poison ivy, thistles, or others as specified. All topsoil shall be tested by a reputable laboratory for organic matter content, pH and soluble salts. A pH of 6.0 to 7.5 and an organic content of not less than 1.5 percent by weight is required. If pH value is less than 6.0 lime shall be applied and incorporated with the topsoil to adjust the pH to 6.5 or higher. Topsoil containing soluble salts greater than 500 parts per million shall not be used.

Note: No sod or seed shall be placed on soil which has been treated with soil sterilants or chemicals used for weed control until sufficient time has elapsed to permit dissipation of toxic materials.

b. Grading - The topsoil shall be uniformly distributed and compacted to a minimum of four (4) inches. Spreading shall be performed in such a manner that sodding or seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or water pockets. Topsoil shall not be placed while in a frozen or muddy condition, when the subgrade is excessively wet, or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.

Note: Topsoil substitutes or amendments as approved by a qualified agronomist or soil scientist, may be used in lieu of natural topsoil. Compost material used to improve the percentage of organic matter shall be provided by a certified supplier.

Compost amendments that are intended to meet specific post-construction stormwater management goals shall further meet the requirements of Appendix 3.06.2 Post Construction Stormwater Management BMP Standards and Specifications, Section 14.0 Soil Amendments.

Source:	Symbol:	Detail No.
		DE-ESC-3.4.1
USDA - NRCS		Sheet 2 of 2
		Effective FEB 2019

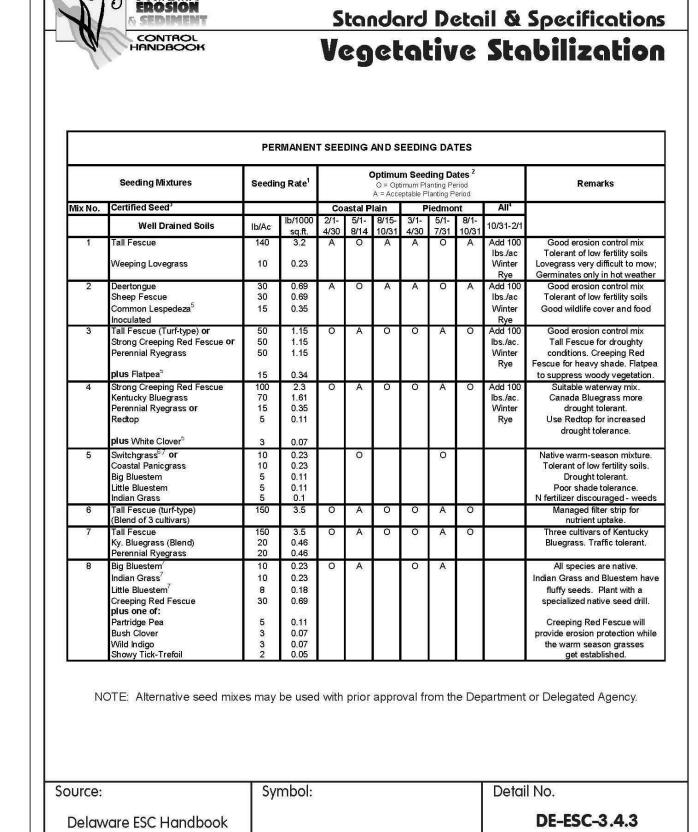
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26/FEB/19 SCALE: AS SHOWN DRAWN BY: WJR DESIGNED BY: WJR APPROVED BY: SHEET NO .: CB006-C-SW022

	TEM	IPORARY	SEEDING	BY F	RATES	, DEP	THS AN	ND DA	TES		
Mix #	Mix # Species ⁶ Seeding Rate Optimum Seeding Dates 1 O = Optimum Planting Period; A = Acceptable Planting Period								Planting Depth [©]		
			1	Co	astal P	lain	Р	iedmoi	nt	All	
	Certified Seed	lb/Ac ⁵	lb/1000 sq.ft.	2/1- 4/30	² 5/1- 8/14	8/15- 10/31	3/1-4/30	² 5/1- 7/31	8/1- 10/31	10/31- 2/1	
1	Barley	125	4	0	Α	0	0	Α	0		1-2 inches 2-3" sandy soils
2	Oats	125	4	0	Α	Α	0	Α	Α		1-2 inches 2-3" sandy soils
3	Rye	125	4	0	Α	0	0	Α	0	Α	1-2 inches 2-3" sandy soils
4	Perennial Ryegrass	125	4	0	Α	0	0	Α	0		0.5 inches 1-2" sandy soils
5	Annual Ryegrass	125	4	0	Α	0	0	Α	0	Α	0.5 inches 1-2" sandy soils
6	Winter Wheat	125	4	0	Α	0	0	Α	0	Α	1-2 inches 2-3" sandy soils
7	Foxtail Millet	30 PLS	0.7		0			0			0.5 inches 1-2" sandy soils
8	Pearl Millet	20 PLS	0.5		0			0			0.5 inches 1-2" sandy soils

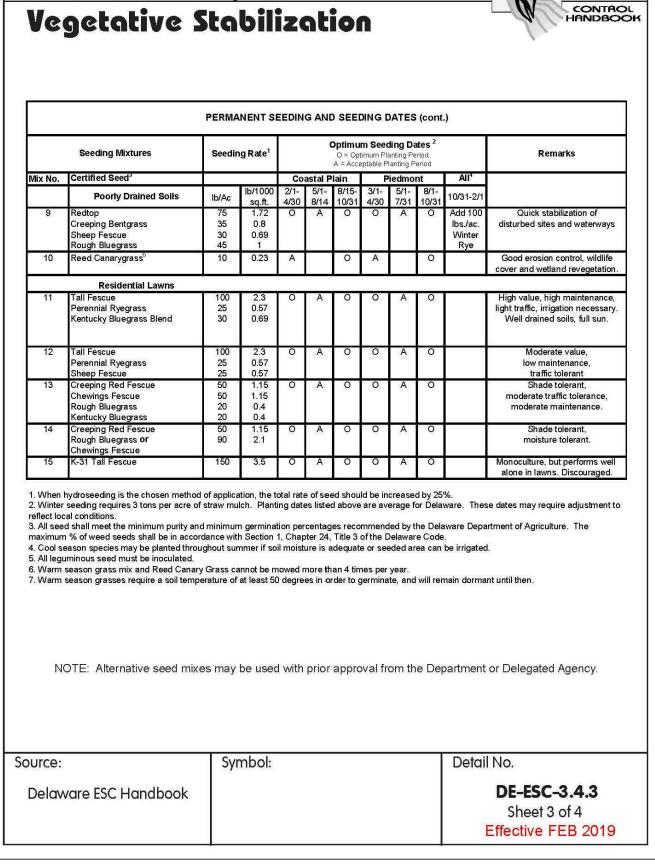
- 1. Winter seeding requires 3 tons per acre of straw mulch for proper stabilization.
- 2. May be planted throughout summer if soil moisture is adequate or seeded area can be irrigated.
- 4. Fifty pounds per acre of Annual Lespedeza may be added to 1/2 the seeding rate of any of the above species.
- 5. Use varieties currently recommended for Delaware. Contact a County Extension Office for information.
- 6. Warm season grasses such as Millet or Weeping Lovegrass may be used between 5/1 and 9/1 if desired. Seed at 3-5 lbs. per acre. Good on low fertility and acid areas. Seed after frost through summer at a depth of 0.5".
- NOTE: Alternative seed mixes may be used with prior approval from the Department or Delegated Agency.

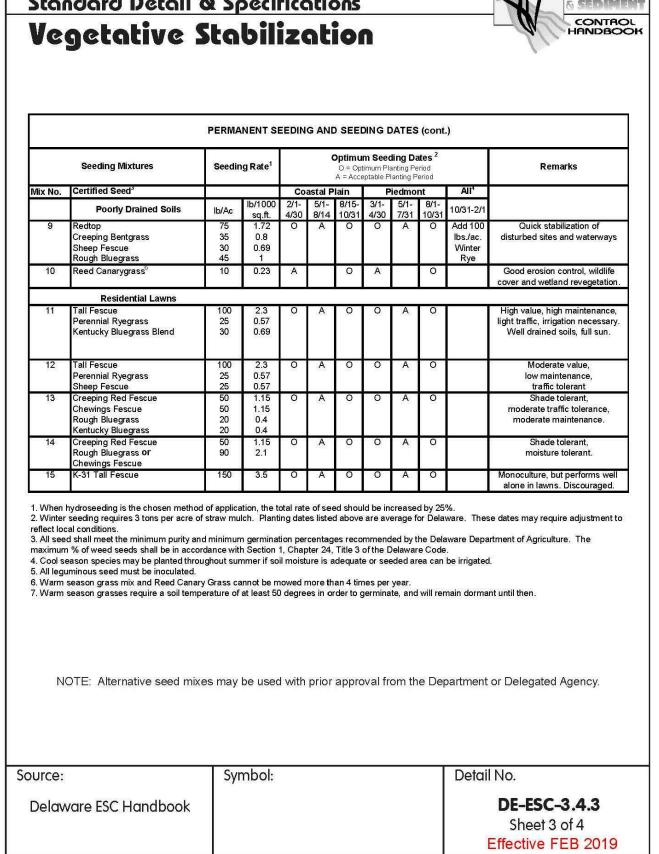
Source:	Symbol:	Detail No.
Delaware ESC Handbook		DE-ESC-3.4.3 Sheet 1 of 4
		Effective FEB 2019

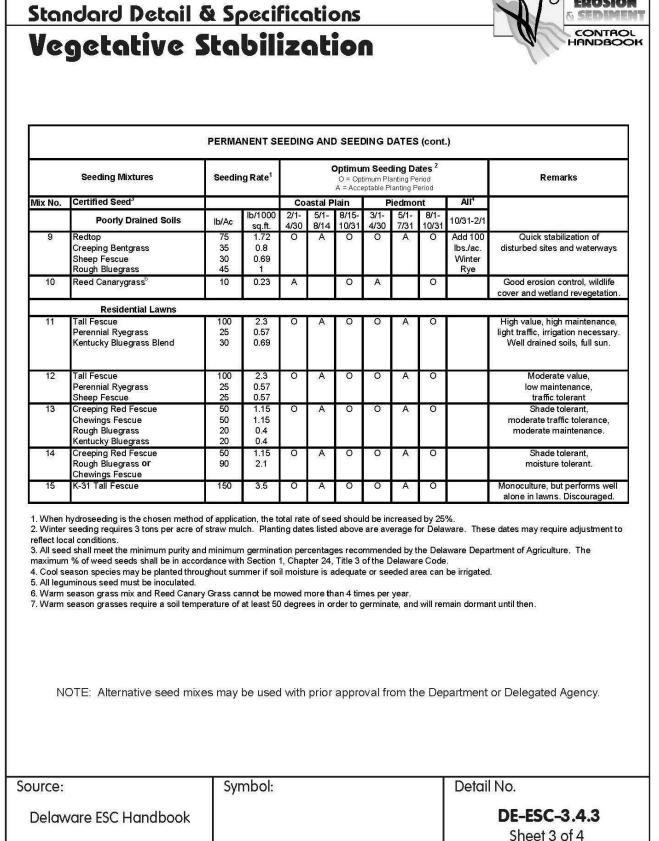


Sheet 2 of 4

Effective FEB 2019









Standard Detail & Specifications Vegetative Stabilization

Construction Notes:

- 1. Site Preparation
- a. Prior to seeding, install needed erosion and sediment control practices such as diversions, grade stabilization structures, berms, dikes, grassed waterways, and sediment basins.
- b. Final grading and shaping is not necessary for temporary seedings.
- 2. Seedbed Preparation

It is important to prepare a good seedbed to insure the success of establishing vegetation. The seedbed should be well prepared, loose, uniform, and free of large clods, rocks, and other objectionable material. The soil surface should not be compacted or crusted.

Soil Amendments

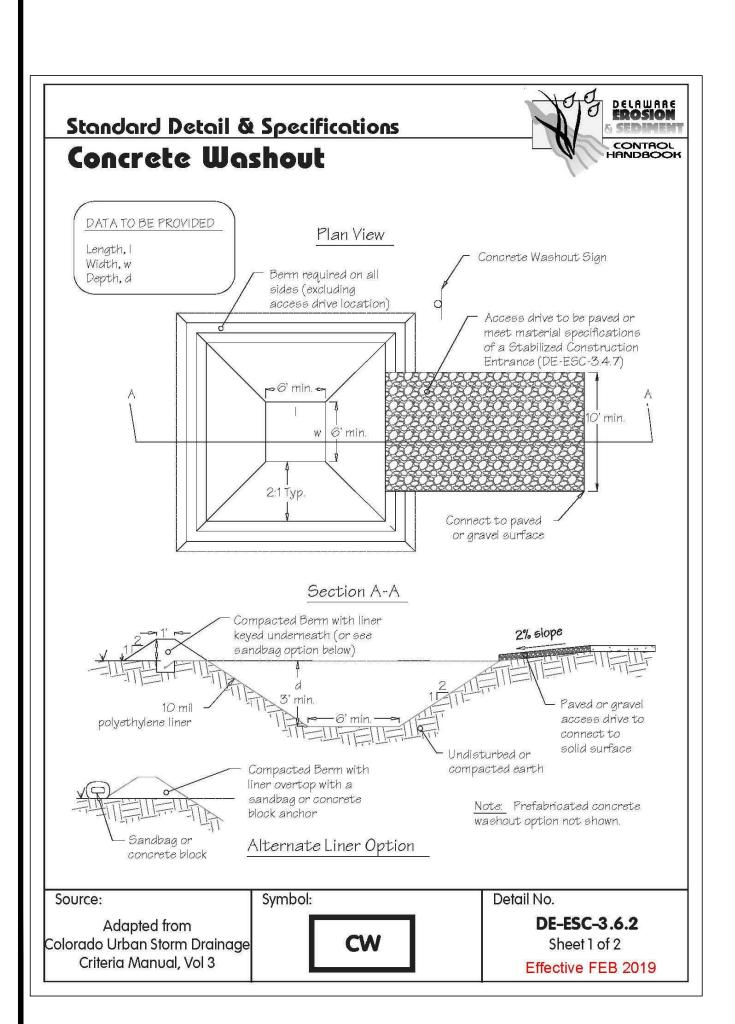
- a. Lime Apply liming materials based on the recommendations of a soil test in accordance with the approved nutrient management plan. If a nutrient management plan is not required, apply dolomitic limestone at the rate of 1 to 2 tons per acre. Apply limestone uniformly and incorporate into the top 4 to 6 inches of soil.
- b. Fertilizer Apply fertilizer based on the recommendations of a **soil test** in accordance with the approved nutrient management plan. If a nutrient management plan is not required, apply a formulation of 10-10-10 at the rate of 600 pounds per acre. Apply fertilizer uniformly and incorporate into the top 4 to 6 inches of soils.

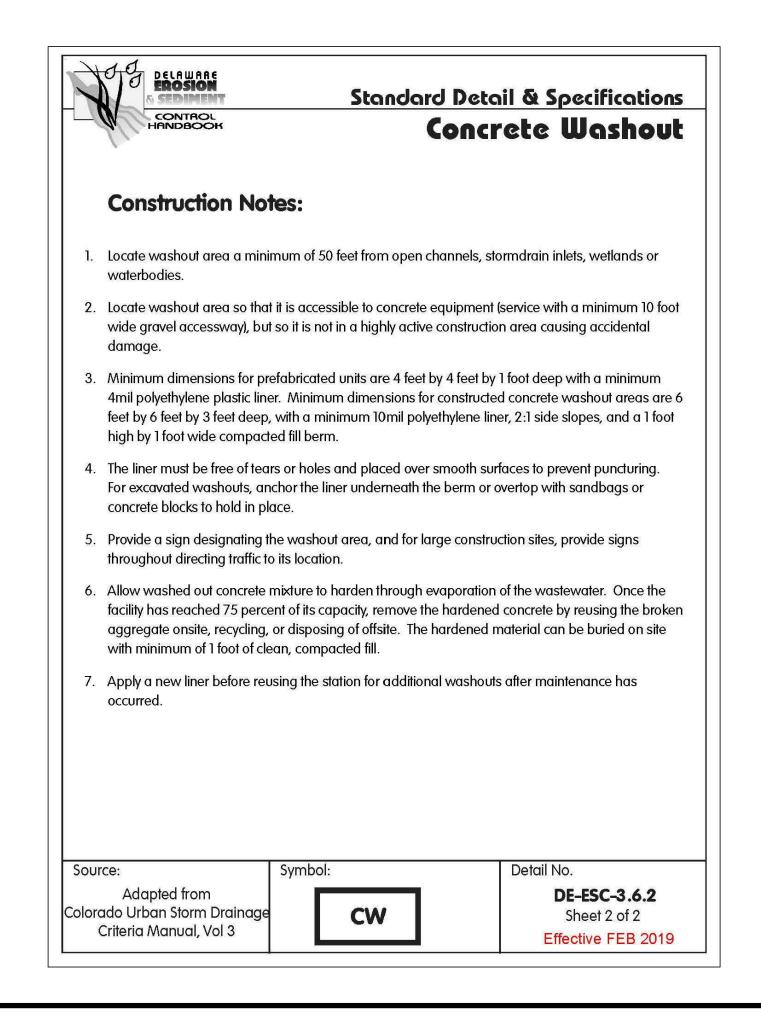
- a. For temporary stabilization, select a mixture from Sheet 1. For a permanent stabilization, select a mixture from Sheet 2 or Sheet 3 depending on the conditions. Alternative seed mixes may be used with prior approval from the Department or Delegated Agency.
- b. Apply seed uniformly with a broadcast seeder, drill, cultipacker seeder or hydroseeder. All seed will be applied at the recommended rate and planting depth.
- c. Seed that has been broadcast should be covered by raking or dragging and then lightly tamped into place using a roller or cultipacker. If hydroseeding is used and the seed and fertilizer is mixed, they will be mixed on site and the seeding shall be done immediately and without interruption.

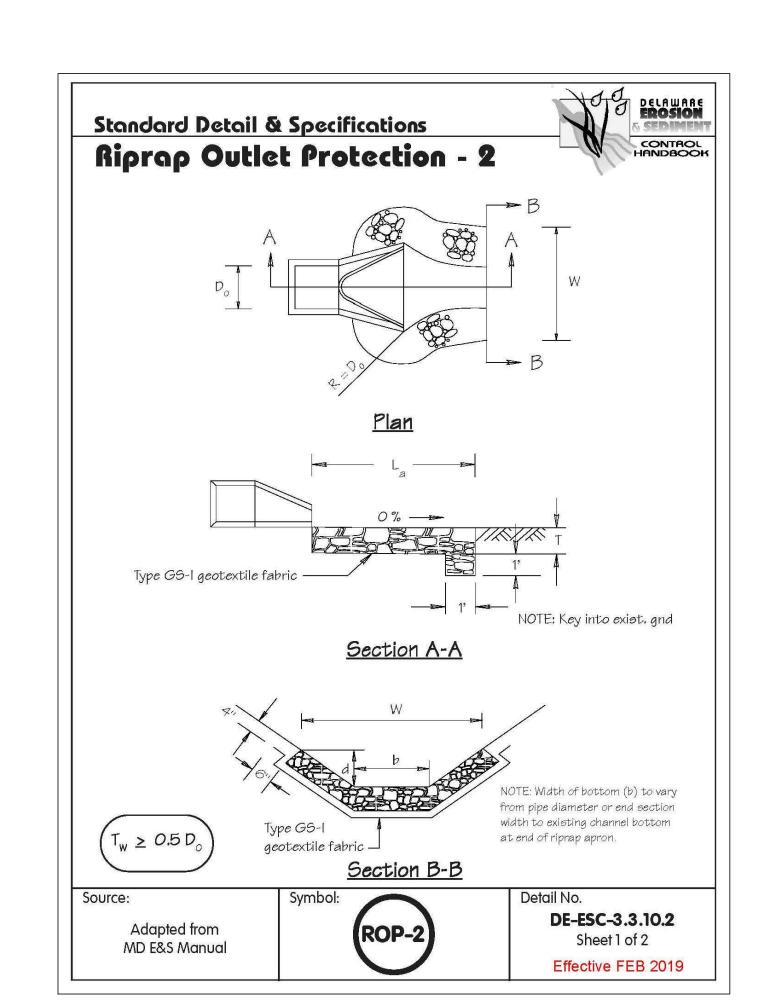
All mulching shall be done in accordance with detail **DE-ESC-3.4.5**.

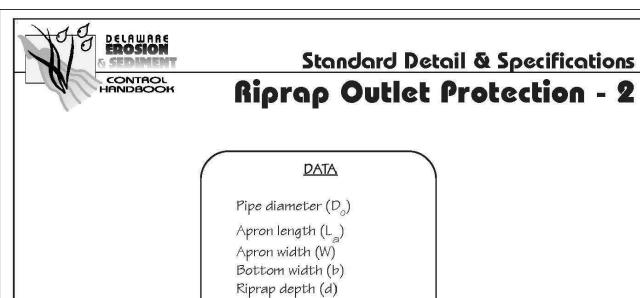
Source:	Symbol:	Detail No.
Delaware ESC Handbook		DE-ESC-3.4.3
		Sheet 4 of 4
		Effective FEB 2019

Standard Detail & Specifications









Construction Notes:

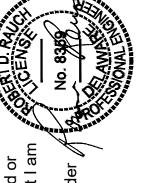
- The subgrade for the riprap shall be prepared to the required lines and grades as shown on the plan. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding undisturbed material.
- 2. The riprap shall conform to the grading limits as shown on the plan.

Riprap size (R No.) Riprap thickness (T)

- 3. Filter cloth shall be protected from punching, cutting or tearing. Any damage other than an occasional small hole shall be repaired by placing another piece of cloth over the damaged area. All connecting joints should overlap a minimum of 1 ft. If the damage is extensive, replace the entire filter cloth.
- 4. Stone for the riprap or gabion outlets may be placed by equipment. Riprap shall be placed in a manner to prevent damage to the filter cloth. Hand placement will be required to the extent necessary to prevent damage to the conduits, structures, etc.

Source:	Symbol:	Detail No.
Adapted from MD E&S Manual	ROP-2	DE-ESC-3.3.10.2 Sheet 2 of 2
MID EX3 Mariodi		Effective FEB 2019

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26/FEB/19 SCALE: AS SHOWN DRAWN BY: DESIGNED BY: APPROVED BY: SHEET NO .:

CB006-C-SW023



Pollution Prevention - Spill Prevention

- 1. Fueling should only take place in signed designated areas, away from downstream drainage facilities and watercourses.
- 2. Fueling must be with nozzles equipped with automatic shut-off to control drips. Do not top off.
- 3. Protect the areas where equipment or vehicles are being repaired, maintained, fueled or parked from storm water run-on and runoff.
- 4. Use barriers such as berms to prevent storm water run-on and runoff, and to contain spills.
- 5. Place a "Fueling Area" sign next to each fueling area.
- 6. Store hazardous materials such as fuel, solvents, oil and chemicals in secondary containment. 7. Inspect vehicles and equipment for leaks on each day of use. Repair fluid and oil leaks
- immediately. 8. Absorbent spill clean-up materials and spill kits must be available in fueling areas and on fuel
- 9. If fueling is to take place at night, make sure the fueling area is sufficiently illuminated. 10. Properly dispose of used oil, fluids, lubricants and spill clean-up materials. CLEAN UP SPILLS
- 1. If it is safe to do so, immediately contain and clean up any chemical and/or hazardous material
- 2. Properly dispose of used oil, fluids, lubricants and spill clean-up materials.
- 3. Do not bury spills or wash them down with water.
- LEAKS AND DRIPS
- 1. Use drip pans or absorbent pads at all times. Place under and around leaky equipment.
- 2. Do not allow oil, grease, fuel or chemicals to drip onto the ground.
- 3. Have spill kits and clean up material on-site.
- 4. Repair leaky equipment promptly or remove problem vehicles and equipment from the site. Clean up contaminated soil immediately.
- 5. Store contaminated waste in sealed containers constructed of suitable material. Label these containers properly.
- 6 Clean up all spills and leaks Promptly dispose of waste and spent clean up materials

Source:	Symbol:	Detail No.
Delaware ESC Handbook		DE-ESC-3.6.1 Sheet 2 of 5
		Effective FEB 2019

Standard Detail & Specifications

Construction Site Waste Mgt & Spill Control

The Construction Site Pollution Prevention Plan should include the following elements:

1. Material Inventory

Document the storage and use of the following materials:

- a. Concrete
- b. Detergents
- c. Paints (enamel and latex)
- d. Cleaning solvents
- e. Pesticides f. Wood scraps
- g. Fertilizers
- h. Petroleum based products

2. Good housekeeping practices

- a. Store only enough product required to do the job.
- b. All materials shall be stored in a neat, orderly manner in their original labeled containers and covered.
- c. Substances shall not be mixed.
- d. When possible, all of a product shall be used up prior to disposal of the container.
- e. Manufacturers' instructions for disposal shall be strictly adhered to.
- f. The site foreman shall designate someone to inspect all BMPs daily.

3. Waste management practices

- a. All waste materials shall be collected and stored in securely lidded dumpsters in a location that does not drain to a waterbody.
- b. Waste materials shall be salvaged and/or recycled whenever possible.
- c. The dumpsters shall be emptied a minimum of twice per week, or more if necessary. The licensed trash hauler is responsible for cleaning out dumpsters.

Source:	Symbol:	Detail No.
Adapted from USEPA Pub. 840-B-92-002	~	DE-ESC-3.6.1 Sheet 3 of 5
		Effective FEB 2019



Standard Detail & Specifications Construction Site Waste Mgt & Spill Control

Notes (cont.)

- d. Trash shall be disposed of in accordance with all applicable Delaware laws.
- e. Trash cans shall be placed at all lunch spots and littering is strictly prohibited. Recycle bins shall be placed near the construction trailer.
- f. If fertilizer bags can not be stored in a weather-proof location, they shall be kept on a pallet and covered with plastic sheeting which is overlapped and anchored.

4. Equipment maintenance practices

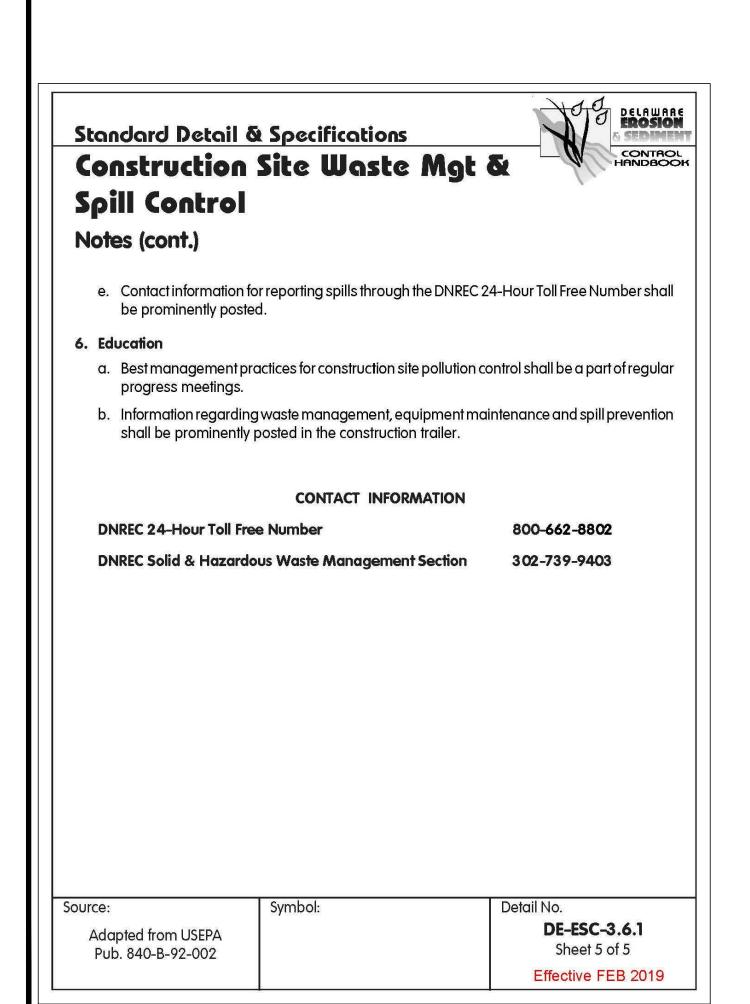
- a. If possible, equipment should be taken to off-site commercial facilities for washing and
- b. If performed on-site, vehicles shall be washed with high-pressure water spray without detergents in an area contained by an impervious berm.
- c. Drip pans shall be used for all equipment maintenance.
- d. Equipment shall be inspected for leaks on a daily basis.
- e. Washout from concrete trucks shall be disposed of in a temporary pit for hardening and proper disposal.
- f. Fuel nozzles shall be equipped with automatic shut-off valves.
- g. All used products such as oil, antifreeze, solvents and tires shall be disposed of in accordance with manufacturers' recommendations and local, state and federal laws and regulations.

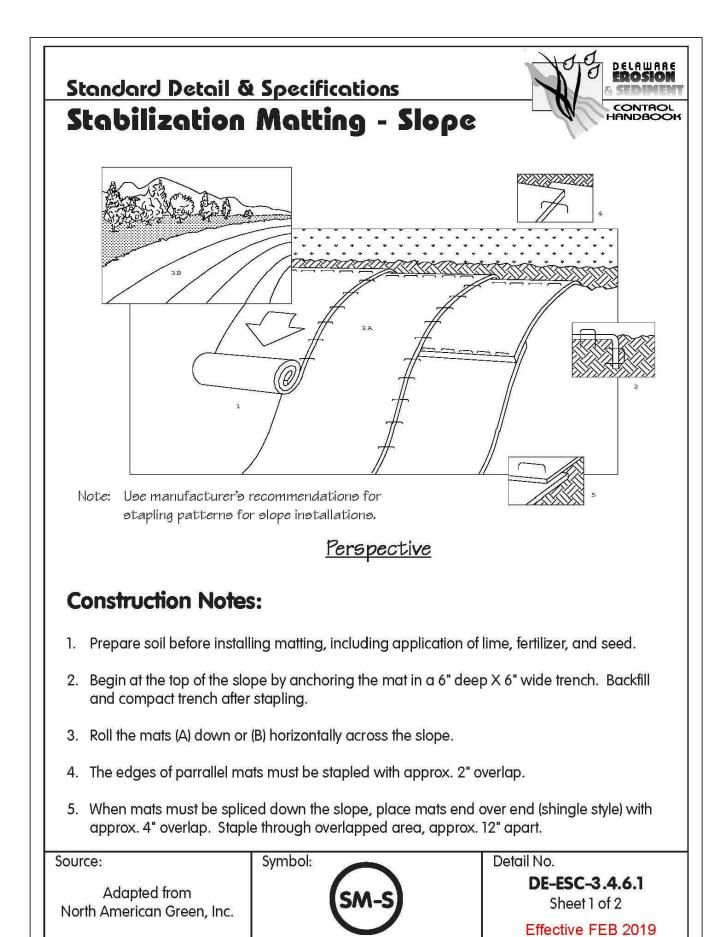
5. Spill prevention practices

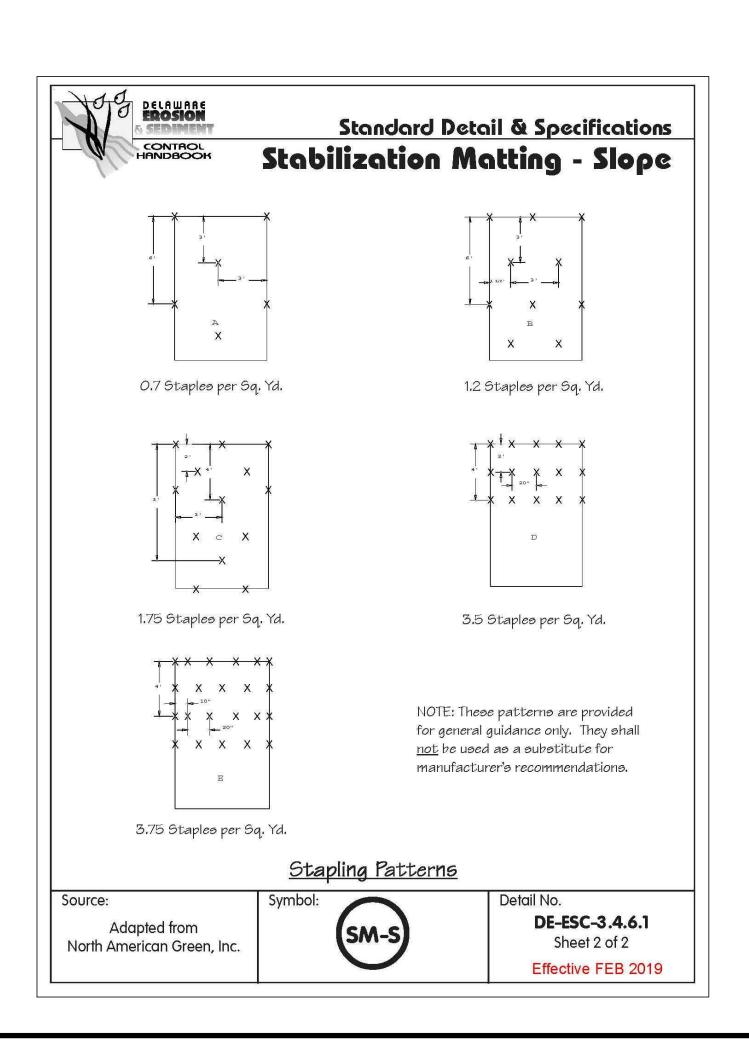
- a. Potential spill areas shall be identified and contained in covered areas with no connection to the storm drain system.
- b. Warning signs shall be posted in hazardous material storage areas.
- c. Preventive maintenance shall be performed on all tanks, valves, pumps, pipes and other equipment as necessary.
- d. Low or non-toxic substances shall be prioritized for use.

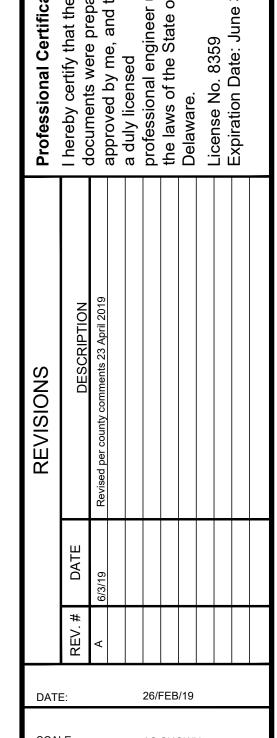
Source:	Symbol:	Detail No.
Adapted from USEPA		DE-ESC-3.6.1
Pub. 840-B-92-002		Sheet 4 of 5
		Effective FEB 2019

NOTE: ALL ESC MAT SHALL BE BIODEGRADABLE





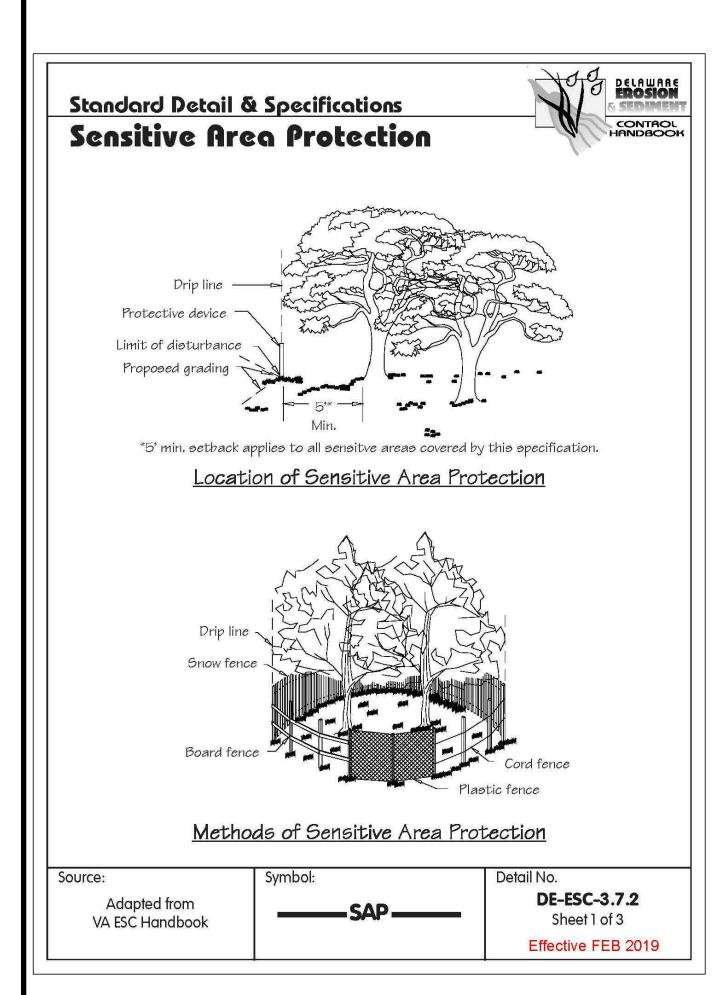


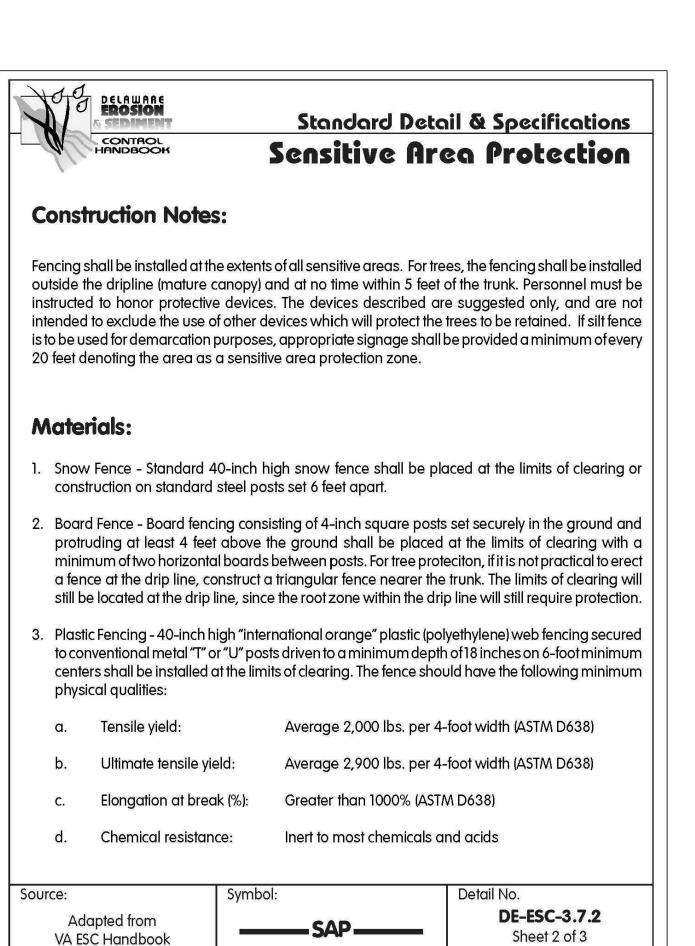


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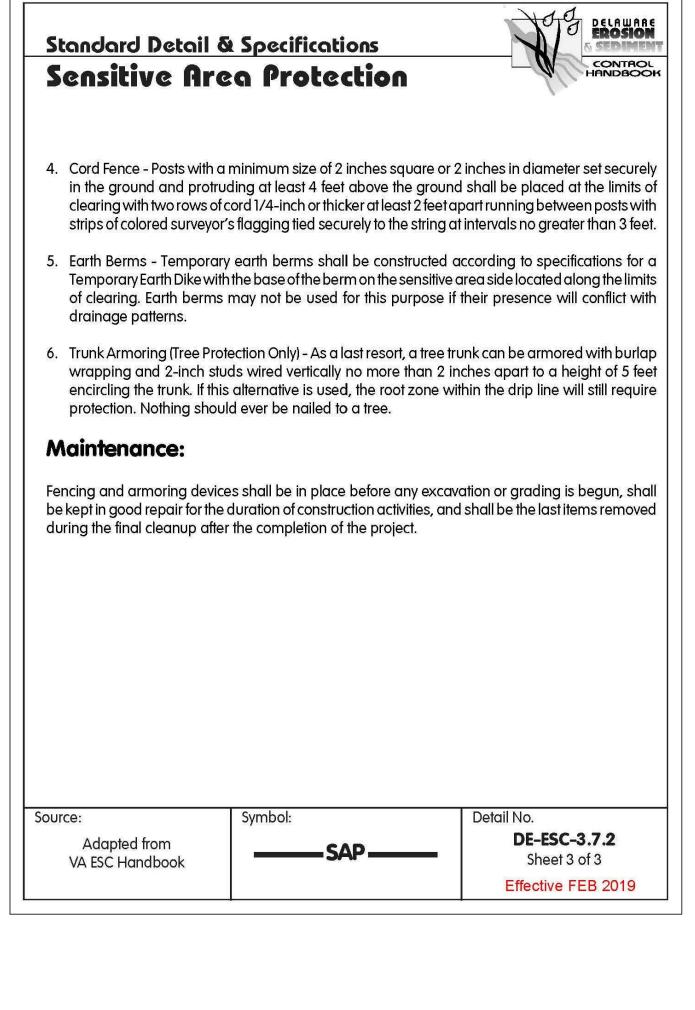
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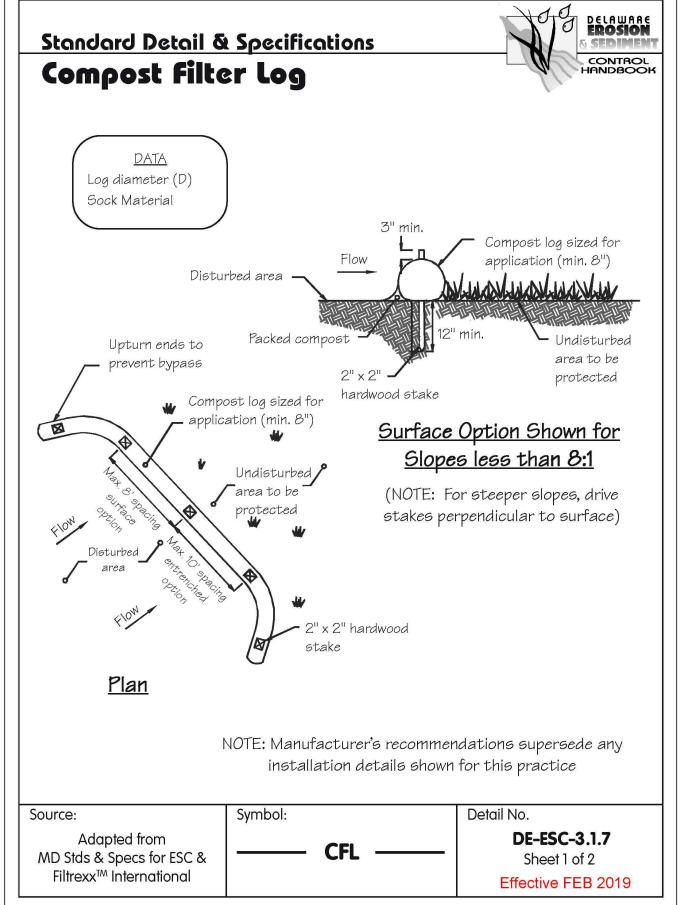
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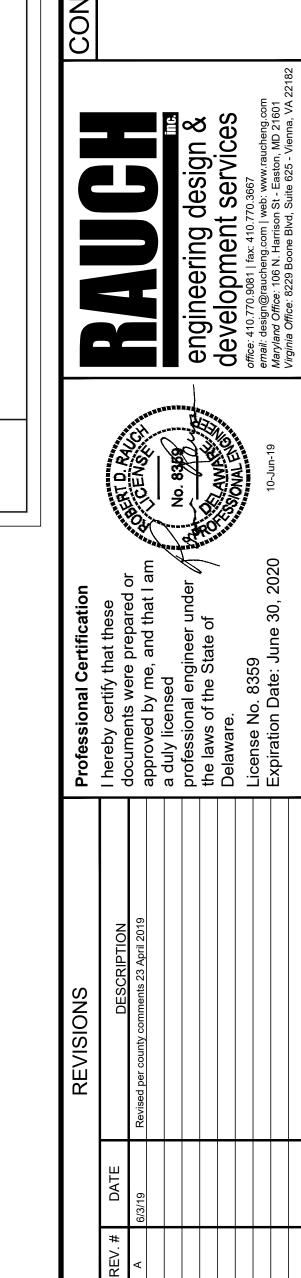




Effective FEB 2019







26/FEB/19

AS SHOWN

CB006-C-SW025

SHEET NO.:

REVIEW

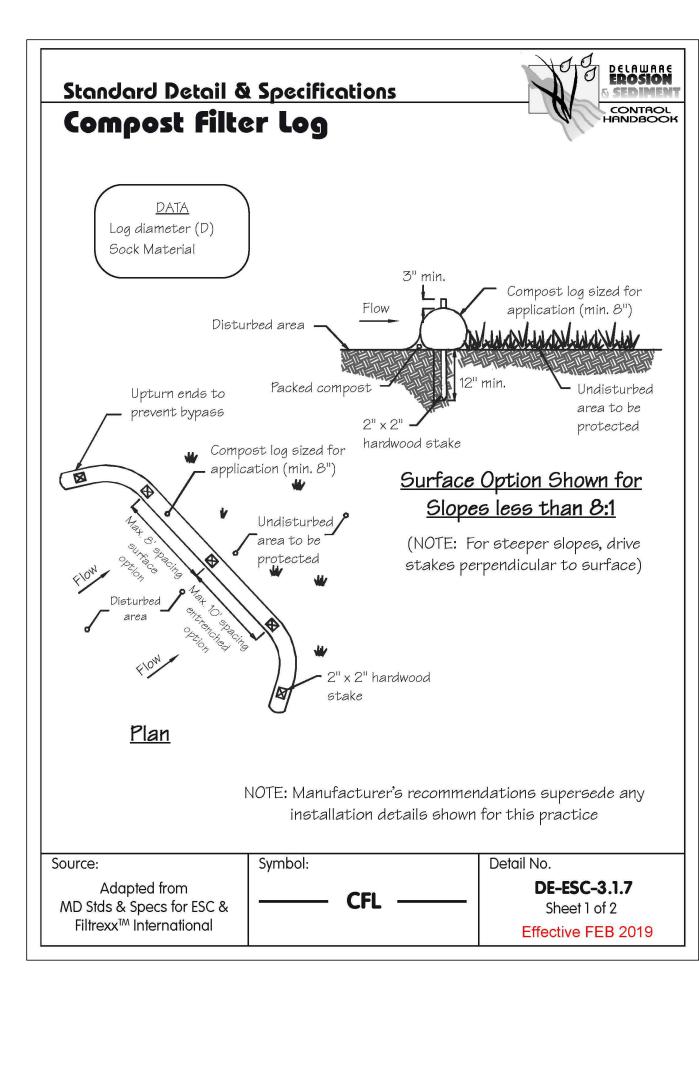
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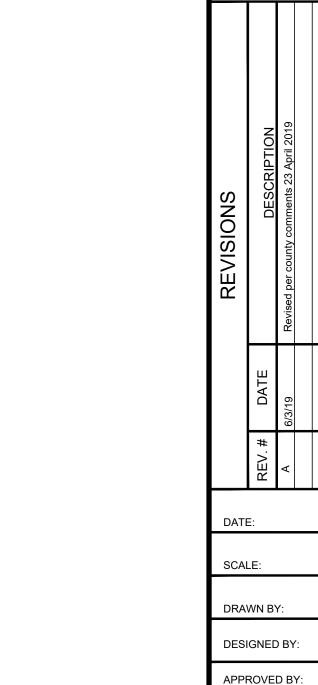


Construction Notes:

- Prior to installation, clear bedding area of obstructions including rocks or debris larger than 1 inch and fill in any sharp depression areas.
- If socks are prepared on-site, fill the sock fabric using a pneumatic blower so that the logs are rigid and do not deform. Terminate at the desired length.
- 3. For trenched applications, excavate 2 to 4 inches below grade along the width and length of the
- Install the compost filter logs perpendicular to the flow direction and parallel to the slope with the beginning and end of the installation pointing up the slope a minimum of 1 foot elevation difference. On sites where this is not possible, upturn at a minimum length of 10' at a 30 degree angle to prevent runoff bypass.
- For untrenched applications, blow or hand pack soil, mulch, or compost on the upslope side of the log, filling the bottom void area.
- Stake the filled log every 10 feet maximum through the center of the sock for trenched applications, or every 8 feet for untrenched. The stake shall be a 2" by 2" hardwood. It should extend 12" below grade and protrude at least 3" above the top of the sock. If located on a slope greater than 8:1, the stake shall be angled downslope at a 45 degree angle to prevent the force of the water from dislodging to log.
- When the length of the compost filter log needed exceeds the available compost filter sock length, the next sock shall be overlapped a minimum of 12" before being filled, and a stake placed through both socks at the overlap.
- Remove accumulated sediment when it has reached half of the effective height of the log.
- Inspect weekly and after rain event. If sock is degrading or the sock is failing, vegetate to secure the compost, replace the log, or reinforce with an additional log. If the log has been crushed due to construction equipment, it can be "fluffed" back to its effective height. If the effective height can no longer be restored, the log shall be replaced or reinforced with an additional compost filter log.

Source:	Symbol:	Detail No.
Adapted from MD Stds & Specs for ESC & Filtrexx™ International	CFL	DE-ESC-3.1.7 Sheet 2 of 2 Effective FEB 2019





CONSTRUCTION SEQUENCE

- 1. NOTIFY THE SUSSEX CONSERVATION DISTRICT IN WRITING AT LEAST FIVE (5) DAYS PRIOR TO THE START OF CONSTRUCTION. FAILURE TO DO SO CONSTITUENTS A VIOLATION OF THE APPROVED SEDIMENT AND STORMWATER MANAGEMENT PLAN.
- PRIOR TO ANY CLEARING, INSTALLATION OF SEDIMENT CONTROL MEASURES, OR GRADING, SCHEDULE AND CONDUCT A PRE-CONSTRUCTION MEETING WITH THE AGENCY CONSTRUCTION SITE REVIEWER. THE LANDOWNER/DEVELOPER REPRESENTATIVE, SITE CONTRACTOR, AND CERTIFIED CONSTRUCTION REVIEWER ARE REQUIRED TO BE IN ATTENDANCE AT THE PRE-CONSTRUCTION MEETING; THE SITE DESIGNER IS RECOMMENDED TO ATTEND.

PERIMETER CONTROLS

- 3. INSTALL THE ENTRANCEWAY CULVERTS AND STABILIZED CONSTRUCTION ENTRANCE(S) AS INDICATED ON THE PLAN, FOLLOWED BY THE PERIMETER CONTROLS (I.E., BERMS, SILT FENCE, COMPOST LOGS) AND INLET PROTECTION ON ANY EXISTING INLETS. MARK THE LIMITS OF SENSITIVE AREAS, SUCH AS PRESERVED TREES, INFILTRATION AREAS, AND OTHER SECTIONS THAT ARE NOT TO BE DISTURBED WITH A PHYSICAL BARRIER. ONLY CLEAR WOODS THAT ARE NEEDED TO INSTALL THE PERIMETER CONTROLS (AS NEEDED).
- 4. SCHEDULE A PERIMETER CONTROL REVIEW WITH THE AGENCY CONSTRUCTION SITE REVIEWER.
- 5. THE CONTRACTOR SHOULD AT ALL TIMES PROTECT AGAINST SEDIMENT OR DEBRIS LADEN RUNOFF OR WIND FROM LEAVING THE SITE. ALL PERIMETER CONTROLS ARE TO BE REVIEWED BY THE AGENCY CONSTRUCTION SITE REVIEWER AND APPROVED PRIOR TO PROCEEDING WITH FURTHER SITE DISTURBANCE OR CONSTRUCTION.
- CHECK PERIMETER CONTROLS DAILY AND ADJUST AND/OR REPAIR TO FULLY CONTAIN AND CONTROL SEDIMENT FROM LEAVING THE SITE. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT HAS REACHED HALF OF THE EFFECTIVE CAPACITY OF THE CONTROL. ADJUST OR ALTER MEASURES IN TIMES OF ADVERSE WEATHER CONDITIONS, AS NEEDED OR AS DIRECTED BY THE AGENCY CONSTRUCTION SITE REVIEWER.

PRE-CONSTRUCTION SITE STORMWATER MANAGEMENT PLAN

PRE-CONSTRUCTION SITE STORMWATER MANAGMENT PLAN FOR SEQUENCING.

SEE THE DETAILED CONSTRUCTION SEQUENCE ON THE

- 8. CLEAR AND GRUB THE SEDIMENT TRAP AND BASIN AREA(S).
- 9. NOTIFY THE PERSON RESPONSIBLE FOR STORMWATER SYSTEM CONSTRUCTION REVIEW AT LEAST THREE (3) DAYS PRIOR TO THE START OF THE STORMWATER SYSTEM CONSTRUCTION; STORMWATER FACILITIES MUST BE REVIEWED THROUGHOUT THEIR CONSTRUCTION.
- 10. CONSTRUCT THE SEDIMENT TRAP AND/OR BASIN(S), STARTING WITH THE OUTLET DEVICE AND DISCHARGE PIPE, AND STABILIZE IMMEDIATELY WITH SEED AND MULCH. FOR BASIN CONSTRUCTION, SEE THE SPECIFIC CONSTRUCTION SEQUENCE FOR THE FACILITY.
- 11. STOCKPILE TOPSOIL AND EXCAVATED SUBSOILS. STOCKPILES SHOULD BE SURROUNDED WITH A PERIMETER CONTROL, LOCATED ON LAND WITH SLIGHT TO NO SLOPE, AND STABILIZED ONCE INACTIVE.
- 12. CONSTRUCT TEMPORARY EARTH DIKES, BERMS, AND/OR SWALES NEEDED FOR SEDIMENT AND EROSION CONTROL AS INDICATED ON THE PLAN AND STABILIZE IMMEDIATELY AS PER THE VEGETATION SPECIFICATIONS. ALL CONVEYANCE AREAS, AND SLOPES, REQUIRE SEEDING AND MATTING AT A MINIMUM.
- 13. PERFORM ANY DEMOLITION WORK, AND CLEAR, GRUB, AND ROUGH GRADE THE SITE'S ROADWAYS. STOCKPILE APPROPRIATELY.
- 14. CONSTRUCT ROADSIDE DITCHES AND TEMPORARILY STABILIZE. STONE CHECK DAMS SHALL BE INSTALLED AT THIS TIME IF APPLICABLE.
- 15. CLEAR AND GRUB REMAINING AREAS WITHIN THE LIMITS OF

DISTURBANCE FOR THE PHASE OF CONSTRUCTION.STOCKPILE

CONSTRUCTION SITE STORMWATER MANAGEMENT PLAN

- 16. SEE THE DETAILED CONSTRUCTION SEQUENCE ON THE CONSTRUCTION SITE STORMWATER MANAGMENT PLAN FOR SEQUENCING.
- 17. CONSTRUCT THE STORMWATER CONVEYANCE SYSTEM, STARTING AT THE LOWEST ELEVATION WITHIN THE NETWORK ANDWORKING UPWARDS. INSTALL NECESSARY INLET PROTECTION AS SHOWN ON PLAN. INSTALL ANY REMAINING STORMWATER MANAGEMENT FACILITIES, FOLLOWING ITS RESPECTIVE SEQUENCE OF CONSTRUCTION.[ADDITIONAL SEQUENCE OF CONSTRUCTION TO BE PROVIDED FOR ALL STORMWATER FACILITIES.]
- 18. INSTALL REMAINING ROADWAY UTILITIES. [AS APPLICABLE]
- 19. INSTALL THE CURB AND GUTTER [AS APPLICABLE], FOLLOWED BY THE SUB-BASE AND BASE COURSE SECTIONS OF THE ROADWAYS TO DESIGN GRADES. INSTALL ANY SIDEWALKS.
- 20. ROUGH GRADE LOT OR BUILDING AREAS AND INDIVIDUAL UTILITY CONNECTIONS. TEMPORARY STABILIZATION IS TO BE APPLIED IN ACCORDANCE WITH THE STABILIZATION NOTES AND DETAILS.
- 21. COMMENCE BUILDING CONSTRUCTION. [AS APPLICABLE]
- 22. FINAL GRADE SWALES OR DITCHES, AND APPLY PERMANENT STABILIZATION AS SOON AS FINAL GRADE IS ACHIEVED.
- 23. FINAL GRADE LOT OR BUILDING AREAS AND APPLY PERMANENT STABILIZATION
- 24. FLUSH OUT THE STORMWATER PIPES FOR ANY ACCUMULATED SEDIMENT, AND REMOVE SEDIMENT FROM WITHIN ANY FOREBAYS, WITH INSPECTION BY THE CERTIFIED CONSTRUCTION REVIEWER AND/OR THE AGENCY CONSTRUCTION SITE REVIEWER. UPON APPROVAL OF THE AGENCY CONSTRUCTION SITE REVIEWER, CONVERT THE SEDIMENT BASIN TO FINAL POND DESIGN AS SPECIFIED ON THE POND CONVERSION NOTES, AND/OR FILL IN ANY SEDIMENT TRAPS. PROVIDE PERMANENT STABILIZATION AS PER THE VEGETATION SPECIFICATIONS.

POST CONSTRUCTION SITE STORMWATER MANAGEMENT PLAN

- 25. SEE THE DETAILED CONSTRUCTION SEQUENCE ON THE POST CONSTRUCTION SITE STORMWATER MANAGMENT PLAN FOR SEQUENCING.
- 26. WHEN ALL UPSTREAM CONTRIBUTING AREAS HAVE BEEN STABILIZED, COMMENCE CONSTRUCTION OF ANY ADDITIONAL STORMWATER MANAGEMENT FACILITIES. CARE SHOULD BE TAKEN TO PROHIBIT COMPACTION OF THE UNDERLYING SOILS DURING CONSTRUCTION. PROVIDE PERMANENT STABILIZATION. [AS APPLICABLE]
- 27. INSTALL TOPCOAT TO THE ROADWAYS.
- 28. THE EROSION AND SEDIMENT CONTROL DEVICES SHOULD BE REMOVED ONLY AFTER WORK IN AN AREA HAS BEEN COMPLETED AND STABILIZED, WITH WRITTEN APPROVAL FROM THE AGENCY CONSTRUCTION SITE REVIEWER. [REQUIRED SEQUENCE ITEM] COORDINATE THE INSPECTION, AND AFTER THE WRITTEN APPROVAL, REMOVE THE REMAINING CONSTRUCTION SITE CONTROLS.
- 29. IF ANY INDIVIDUAL LOTS ARE SOLD AND DEVELOPED, PROVIDE ON-SITE EROSION AND SEDIMENT CONTROLS FOR THAT LOT (PERIMETER CONTROLS AND A STABILIZED CONSTRUCTION ENTRANCE, AT A MINIMUM). [AS APPLICABLE]
- 30. PRIOR TO COMMENCING A NEW PHASE OF CONSTRUCTION, RECEIVE WRITTEN APPROVAL FROM THE AGENCY CONSTRUCTION SITE REVIEWER THAT THE PREVIOUS PHASE HAS BEEN SUFFICIENTLY STABILIZED. [REQUIRED SEQUENCE ITEM, AS APPLICABLE]
- 1. TERMINATE COVERAGE OF THE CONSTRUCTION GENERAL PERMIT, WHICH REQUIRES SUBMISSION AND ACCEPTANCE OF THE POST CONSTRUCTION VERIFICATION DOCUMENTS, INCLUDING FINAL STABILIZATION THROUGHOUT THE SITE, ALL ELEMENTS OF THE SEDIMENT AND STORMWATER MANAGEMENT PLAN IMPLEMENTED, ACCEPTANCE OF THE FINAL OPERATION AND MAINTENANCE PLAN, AND SUBMITTAL OF THE NOTICE OF TERMINATION. [REQUIRED SEQUENCE ITEM]

	Summary Table for Sub-Areas Draining to a Common Point of Interest (POI) ⁽¹⁾											
		POI:										
Ref.#	Sub-Area ID ⁽²⁾	Contributing Area (ac)	RPv Runoff Reduction Shortfall(+) or Credit(-) (cu.ft.) ⁽³⁾	Adjusted RPv CN after all reductions (4)	Cv RCN for H&H Modeling ⁽⁴⁾	Fv RCN for H&H Modeling ⁽⁴⁾	TN Pollutant Load (lb/yr)	TP Pollutant Load (lb/yr)	TSS Pollutant Load (lb/yr)			
1	PR-1-A	1.81	0	47.79	47.79	47.79	2.48	0.33	99.05			
2	PR-1-SW	16.65	-4511	54.01	62.78	65.22	34.95	4.72	1397.97			
3	PR-2	0.02	0	80.00	80.00	80.00	0.17	0.02	6.64			
4	PR-3	2.85	45	55.33	55.33	55.33	6.51	0.88	260.50			
5	PR-4B	0.96	1112	68.35	68.35	68.35	4.59	0.62	183.80			
Totals to Common POI 22.29 ac -3354 cu		-3354 cu.ft.	54.31	60.87	62.69	48.70 lb/yr	6.57 lb/yr	1947.96 lb/yr				
R	Pv Runoff Reduc	tion Goal Met?	YES									
If Not	, Total Offset Vo	lume Required	N/A									

Notes:

1. As long as the site lies within the same watershed, all sub-areas within the site can be tallied to reflect global site conditions; or, the summary table can be used to show conditions to a specific POI.

2. Only the most downstream sub-area information should be entered for a series of sub-areas that drain directly into each other, as the upstream areas will already be accounted for in the DURMM computations.

3. A RPv runoff reduction shortfall should be entered as a positive number, as it is the runoff volume still needed to be reduced. A RPv credit should be entered as a negative number, as it indicates the additional volume that was reduced past the requirement.

4. To portray an accurate total weighted CN value for the RPv, Cv and Fv events, an entry must be made for every defined sub-area. If a sub-area's contributing drainage acreage is entered, but not its corresponding CN value, then the total weighted CN will be skewed.

NOTE: TO SEE SUB AREAS AND POI REFER TO SHEET CB006-C-SW013

CONSTRUCTION SITE NOTES

- THE RESPONSIBILITY TO OPERATE AND MAINTAIN EACH POST CONSTRUCTION STORMWATER MANAGEMENT FACILITY SHALL BE ON THE CONTRACTOR DOING CONSTRUCTION AND THE OWNER POST CONSTRUCTION.
- 2. THE DNREC SEDIMENT AND STORMWATER PROGRAM AND/OR THE SUSSEX COUNTY SOIL CONSERVATION DISTRICT RESERVES THE RIGHT TO ENTER PRIVATE PROPERTY FOR PURPOSES OF PERIODIC SITE REVIEWS.
- 3. THE DNREC SEDIMENT AND STORMWATER PROGRAM [OR THE SUSSEX COUNTY SOIL CONSERVATION DISTRICT] SHOULD BE NOTIFIED WITH 30 BUSINESS DAYS IF THE PROPERTY OWNERSHIP IS TRANSFERRED TO A NEW PERSON OR ENTITY.
- 4. THE DNREC SEDIMENT AND STORMWATER PROGRAM AND/OR THE SUSSEX COUNTY SOIL CONSERVATION DISTRICT MAY SEEK ENFORCEMENT ACTION AGAINST ANY OWNER DEEMED NEGLIGENT IN FULFILLING THE OPERATIO AND MAINTENANCE REQUIREMENTS OF THE DELAWARE SEDIMENT AND STORMWATER REGULATIONS.
- 5. THE DNREC SEDIMENT AND STORMWATER PROGRAM [OR, THE SUSSEX COUNTY SOIL CONSERVATION DISTRICT] SHOULD BE CONTACTED IF A CONCERN ARISES REGARDING A STORMWATER MANAGEMENT FACILITY, BEFORE ANY NON-ROUTINE MAINTENANCE, OR IF MODIFICATIONS TO THE FACILITY ARE DESIRED.
- 6. ANY DESIGN MODIFICATIONS MADE TO THE STORMWATER SYSTEM SHALL REQUIRE THE CREATION OF A NEW POST CONSTRUCTION STORMWATER MANAGEMENT PLAN AND/OR OPERATIONS AND MAINTENANCE PLAN, WITH APPROVAL OF THE PLAN(S) BY THE DNREC SEDIMENT AND STORMWATER PROGRAM [OR THE SUSSEX COUNTY SOIL CONSERVATION DISTRICT].
- 7. FOR ALL STORMWATER EASEMENT AREAS (I.E., ACCESS, MAINTENANCE, OR OFFSITE) AND THE MINIMUM 10-FOOT WIDE ACCESSWAYS TO ALL STORMWATER FACILITIES AND THEIR STRUCTURAL COMPONENTS, REGULAR MOWING SHOULD BE PERFORMED TO KEEP THE GRASS 6 INCHES OR LESS; NO TREES OR SHRUBS SHOULD BE PLANTED, AND ANY FOUND GROWING SHOULD BE REMOVED; AND NO PERMANENT STRUCTURES, SUCH AS FENCES OR SHEDS, SHOULD BE LOCATED WITHIN THE EASEMENT OR ACCESSWAY.
- TREES SHOULD NOT BE PLANTED, AND SHOULD BE REMOVED IF FOUND GROWING, ON AND WITHIN 15 FEET OF ALL POND EMBANKMENTS, ON POND SLOPES OR SAFETY BENCHES, AND WITHIN 10 FEET OF STRUCTURAL COMPONENTS, SUCH AS PIPE INLETS.
- WHEN THE FACILITY IS EXCAVATED TO REMOVE ACCUMULATED SEDIMENT, THE DISPOSAL AREA SHALL BE PERMANENTLY STABILIZED SO THAT IT DOES NOT RECREATE AN EROSION PROBLEM. ANY MATERIAL TAKEN OFFSITE SHALL STILL BE USED OR DISPOSED OF IN AN APPROVED DNREC MANNER.
- 10. BEFORE ANY EARTHWORK OR EXCAVATION TAKES PLACE, THE CONTRACTOR SHOULD CALL MISS UTILITY AT 811 OR 1-800-282-8555 AT LEAST 48 HOURS PRIOR TO CONSTRUCTION, TO HAVE ALL EXISTING UTILITIES MARKED ONSITE.
- 11. ANY FACILITY SPECIFIC ROUTINE OR NON-ROUTINE MAINTENANCE, AND/OR OPERATIONAL REQUIREMENTS NOT LISTED IN THE ABOVE-MENTIONED STANDARD REQUIREMENTS FOR THE TYPE OF FACILITY. MAY INCLUDE, BUT IS NOT LIMITED TO ANY MOWING, SEDIMENT REMOVAL, PIPE INSPECTIONS, WATERING, RESEEDING/PLANTING, TRASH REMOVAL, ETC

VOLUME OF CUT: 20,178 CU-YD VOLUME OF FILL: 23,540 CU-YD

ng drainage acreage is

evelopment services



pproved by me, and that I are duly licensed rofessional engineer under pelaws of the State of th

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DATE: 26/FEB/19

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DESIGNED BY: WJR

APPROVED BY: X

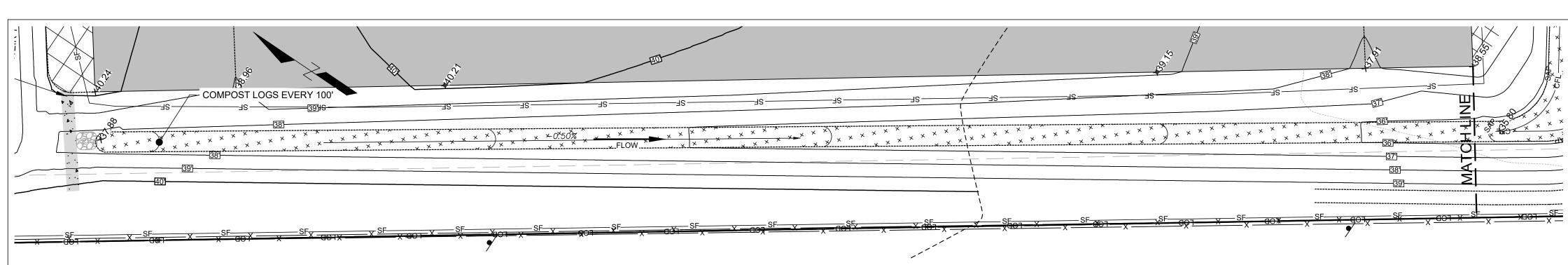
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FOR

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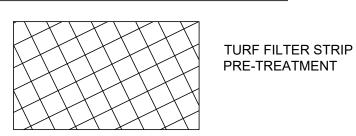
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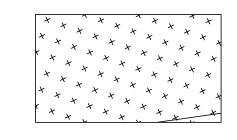
SCALE AS SHOWN



SW INFILTRATION BASIN INLET SWALE PLAN

SCALE 1"=20' FLAT BOTTOM WIDTH VARIES 4:1 SIDE -SLOPES - SEED, TOPSOIL AND MAT SIDE SLOPES





INFILTATION BED AREA TO BE SEEDED WITH ERNST SEED MIXES AND TEMPORARY STABILIZATION MIX

BASIN SEEDING SCHEDULE

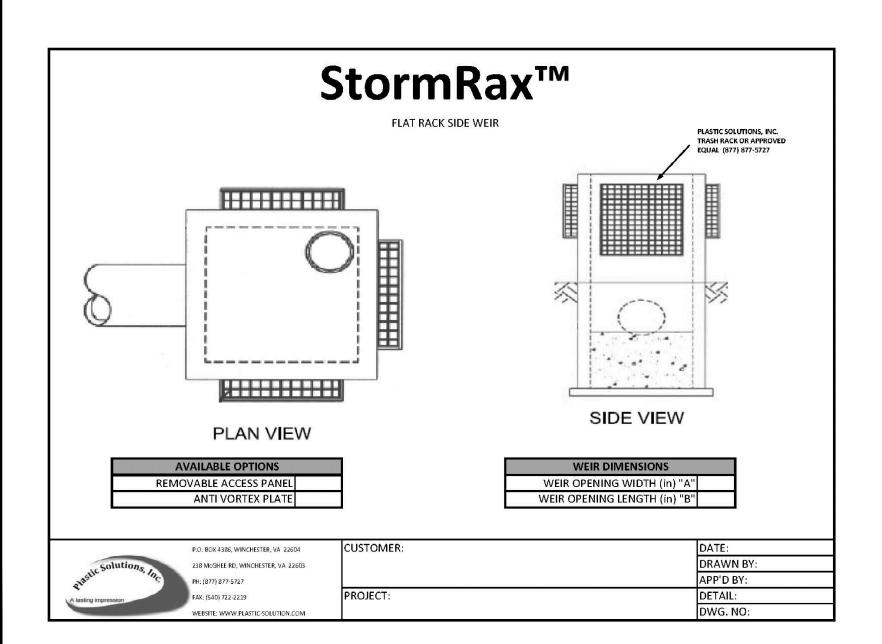
SEED BASES OF INFILTRATION BASIN WITH

- 50% ERNMX-180-2 SOUTH EASTERN RAIN GARDEN MIX @ 0.5lb/1000 SQ-FT
- 50% ERNMX-183 NATIVE DETENTION AREA MIX @ 0.5lb/1000 SQ-FT

ALL OTHER AREAS SHALL BE STABILIZED WITH TOPSOIL AND PERMANENT SEEDING AS PER ESC STANDARDS

SEEDING RATES							
BASIN	AREA SQ-FT	TOTAL WT OF SEEL					
NE	3789	8					
NW	11523	22					
SE	13356	26					
SW	34977	68					
TOTAL		124					

STANDARD SWALE DETAIL



SEQUENCE OF CONSTRUCTION FOR INFILTRATION BASINS PONDS

SEE CB06-C-SW025 FOR FULL SEQUENCE

PRE-CONSTRUCTION PHASE

CLEARLY MARK OUT EXTENTS OF BASINS, VEGETATED SWALES AND OUTFALL STRUCTURES.

INSTALL OUTLET STRUCTURE AND PIPE STARTING AT THE DOWNSTREAM MOST POINT AND WORKING UPSTREAM

EXCAVATE BASINS TO FINAL GRADE AND AMEND SOIL WITH BACKFILL WITH 4" TOPSOIL SALVAGED FROM ON SITE AND 2" WOOD CHIPS AND THOROUGHLY MIX INTO THE TOP 18" OF THE BED. USE LOW GROUND PRESSURE EQUIPMENT WHERE TRAVERSING THE INFILTRATION BED AND KEEP MOVEMENT ACROSS THE BED TO A MINIMUM.

PLACE FILL/TOPOSIL WON FROM THE BASIN AT DESIGNATED STOCKPILE AREAS AND IMMEDIATELY STABILIZE ALL EXPOSED SOIL

PERMANENTLY STABILIZE THE INFILTRATION BASIN BED WITH THE SEED MIXES SPECIFIED IN THE BASIN SEEDING SCHEDULE, AND TEMPORARY SEED MIXTURE AS SPECIFIED IN DE-ESC-3.4.3

INSTALL COMPOST FILTER LOGS, PERMANENTLY STABILIZE SIDE SLOPES WITH TOPSOIL, SEED AND EROSION CONTROL MATTING, INSTALL SUPER SILT FENCE ALONG TOP OF SLOPES. INSTALL ALL ADDITIONAL ASSOCIATED SEDIMENT CONTROL DEVICES

TEST THE INFILTRATION BED TO ENSURE THAT THE SOIL INFILTRATION RATE IS WITHIN THE SPECIFIED RANGE ACCORDING TO TABLE 1.2 IN THE DELAWARE POST CONSTRUCTION STORMWATER BMP STANDARDS AND SPECS.

CONSTRUCTION PHASE

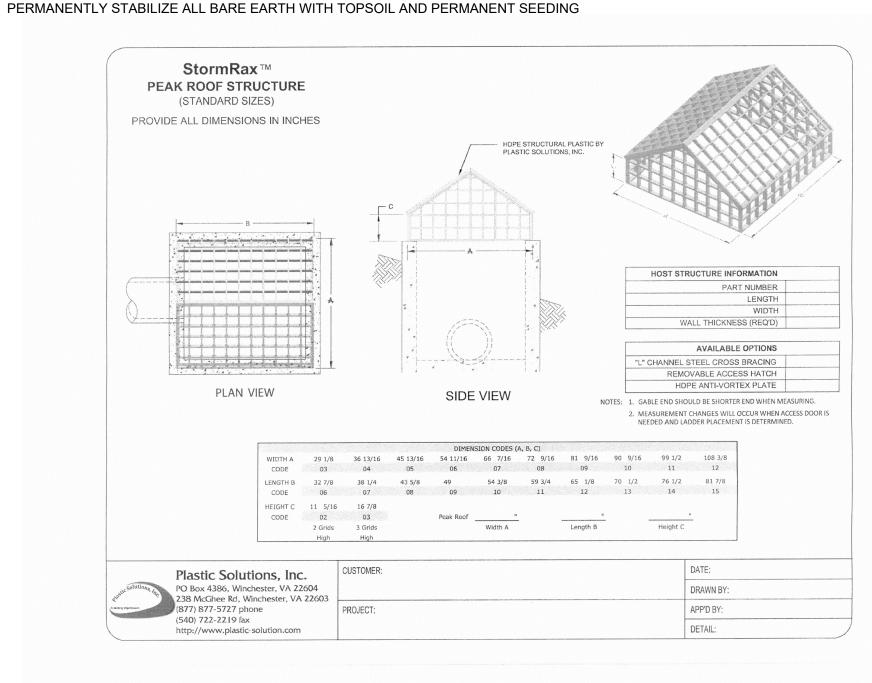
MAINTAIN ALL SEDIMENT CONTROL DEVICES. IMMEDIATELY REMOVE ANY SEDIMENT THAT BUILDS UP IN FOREBAYS OR INSIDE THE INFILTRATION BASIN.

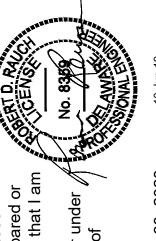
POST CONSTRUCTION PHASE

AFTER THE ASSOCIATED DRAINAGE AREA HAS BEEN COMPLETELY STABILIZED, INFILTRATION BASINS SHALL BE COMPLETED.

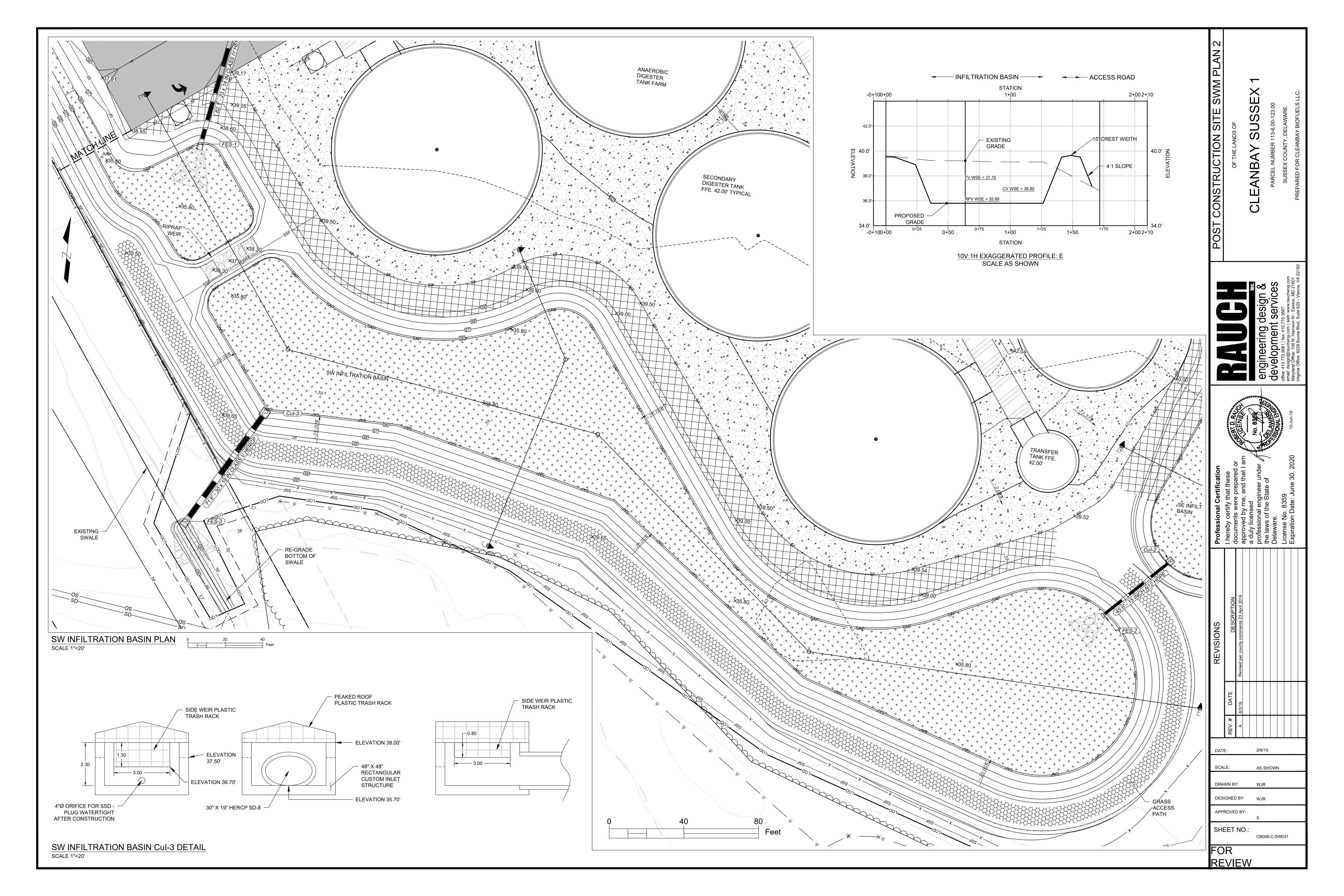
CONSTRUCT ANY FOREBAYS OR WEIRS, AT THIS TIME ALL EFFORTS SHALL BE UNDERTAKEN TO MINIMIZE COMPACTION TO THE INFILTRATION BED. RE-TEST INFILTRATION RATES TO ENSURE THAT THE SOIL INFILTRATION RATE IS WITHIN THE SPECIFIED RANGE ACCORDING TO TABLE 1.2 IN THE DELAWARE POST CONSTRUCTION STORMWATER BMP STANDARDS AND SPECS AND SHOWS NO SIGN OF CLOGGING FROM SEDIMENT THAT HAS WASHED INTO THE BASIN DURING THE CONSTRUCTION PHASE OF THE PROJECT. USE LOW GROUND PRESSURE EQUIPMENT AND MINIMIZE COMPACTION TO THE INFILTRATION BASIN FLOOR.

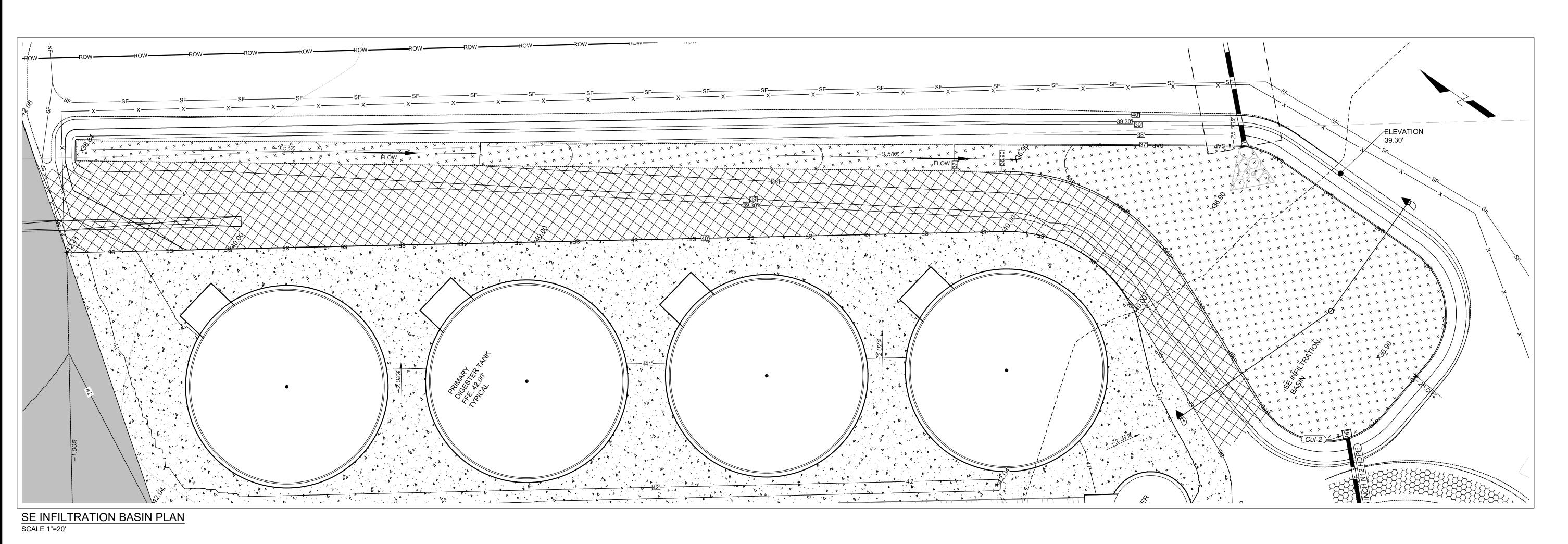
THE INFILTRATION BED SHALL BE TESTED BY A QUALIFIED PROFESSIONAL AS PER APPENDIX 1 IN THE DELAWARE POST CONSTRUCTION STORMWATER BMP STANDARDS AND SPECIFICATIONS.





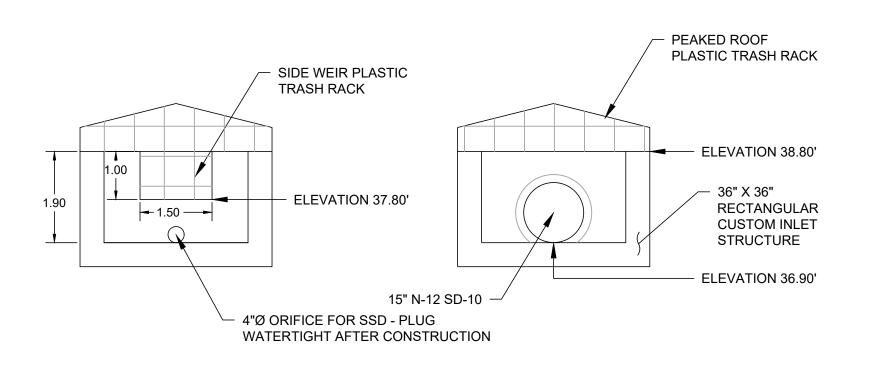
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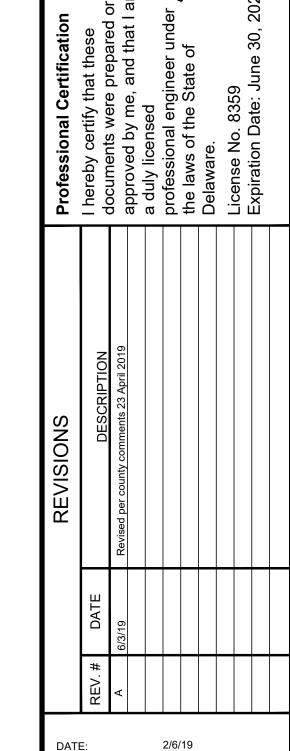


✓ INFILTRATION BASIN — ➤ STATION -0+100+00 - EXISTING GRADE **≥** 40.0' FV WSE = 38.86 CV WSE = 38.0 RPV WSE = 37.45 - 4:1 SLOPE 36.0'-PROPOSED-GRADE -0+100+00 1+501+60 1+00 STATION

10V:1H EXAGGERATED PROFILE: E SCALE AS SHOWN



SE INFILTRATION BASIN Cul-2 DETAIL SCALE 1"=20'



SCALE:

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APPROVED BY:

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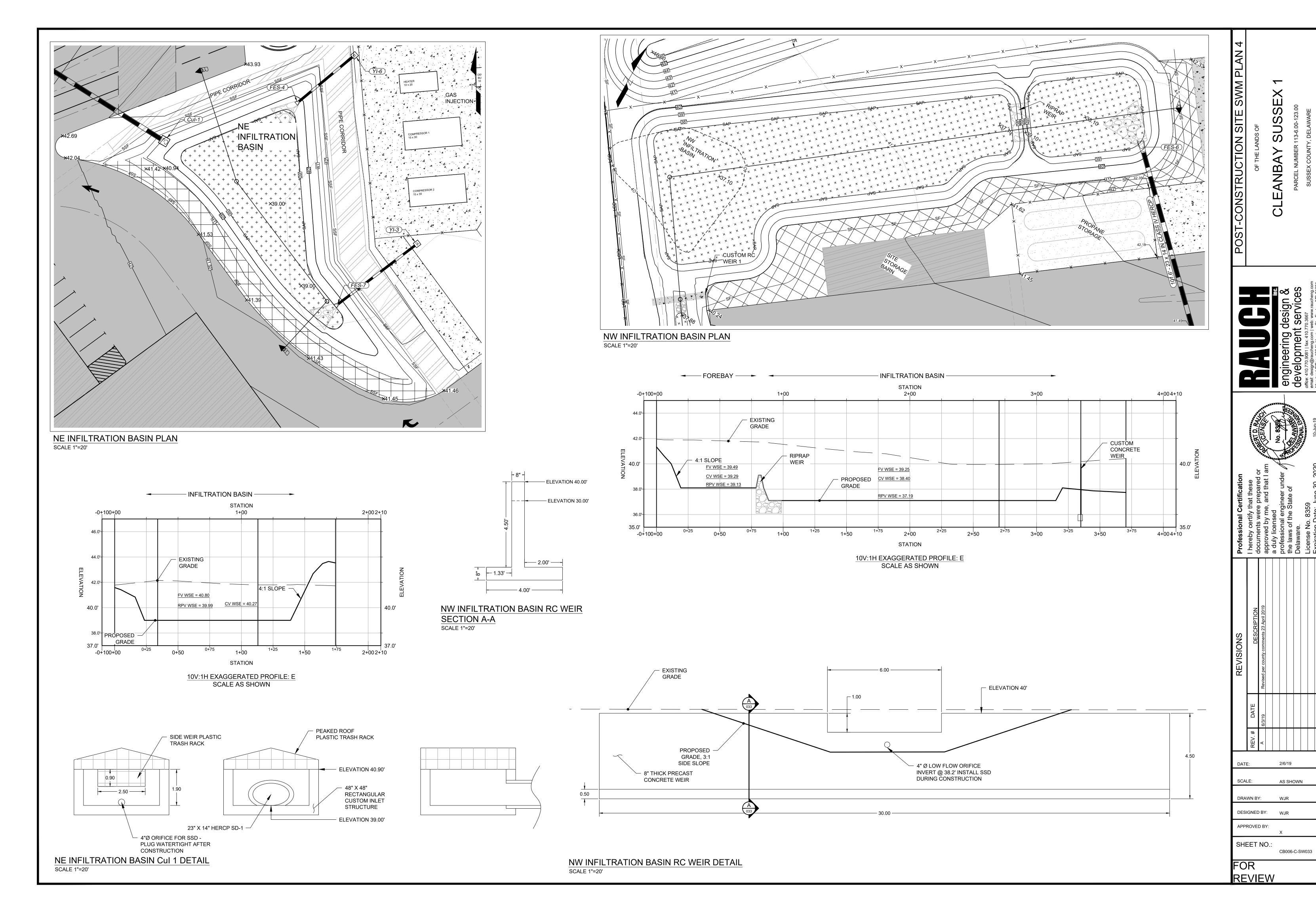
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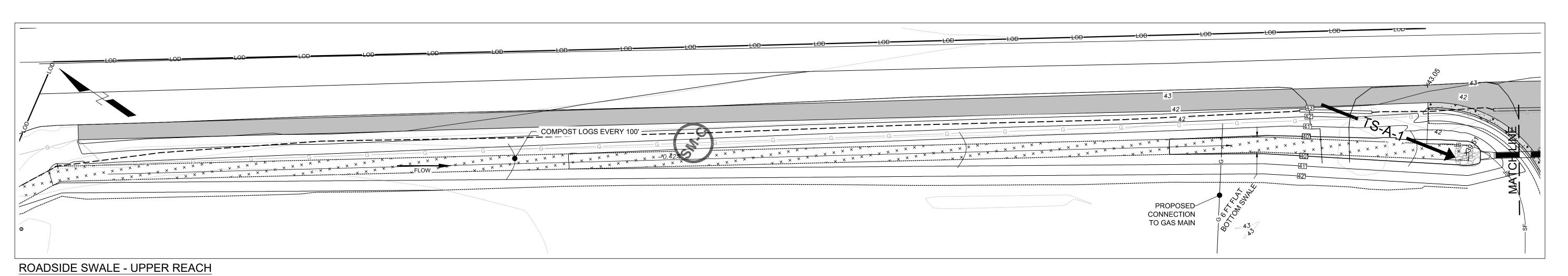
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DESIGNED BY: WJR

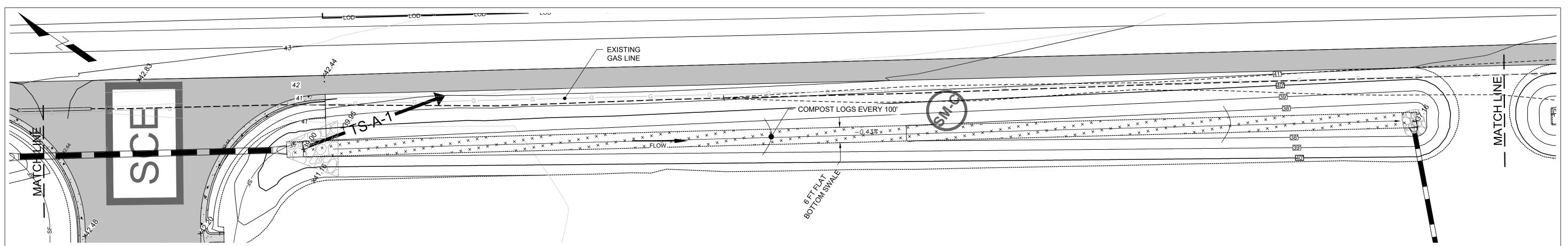
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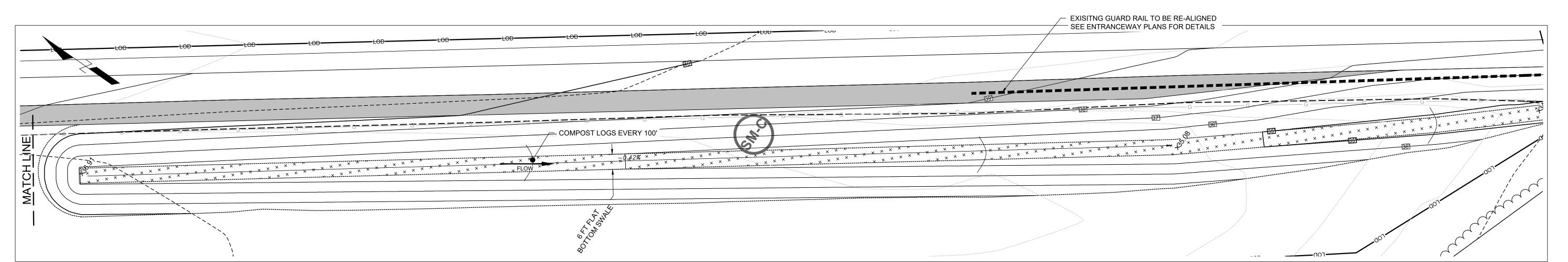




SCALE 1"=20'



ROADSIDE SWALE - MIDDLE REACH SCALE 1"=20'



ROADSIDE SWALE - BOTTOM REACH SCALE 1"=20'

NOTE: GAS UTILITIES ARE PRESENT ON SITE. CHECK WITH MISS UTILITY BEFORE EXCAVATION

Professional Certif	I hereby certify that t	docaliferits were pre	a duly licensed	professional enginee	the laws of the State	Delaware.	Licelise No. 0339	Expiration Date: Jun	
REVISIONS	DESCRIPTION	Revised per county comments 23 April 2019							
	DATE	6/3/19							
	REV.#	А							

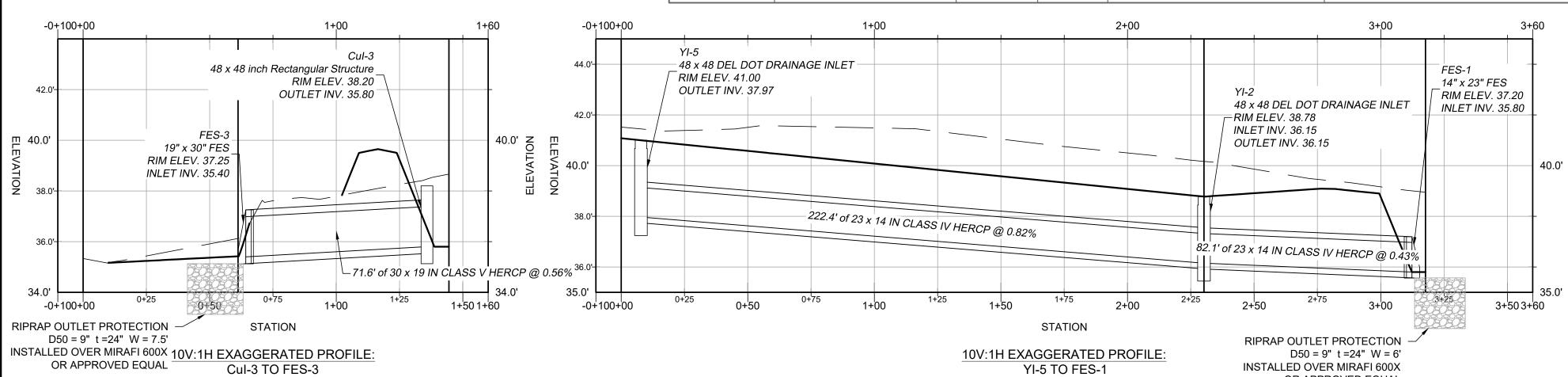
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CB006-C-SW034

SHEET NO.:

PIPE SCHEDULE SITE DRAINAGE

PIPE NAME	SIZE AND MATERIAL	LENGTH	SLOPE	UPSTREAM STRUCTURE AND INVERT	DOWNSTREAM STRUCTURE AND INVERT
SD-2	23 x 14 IN CLASS IV HERCP	101.58'	0.45%	YI-1 INV. 38.56'	FES-6 INV. 38.10'
SD-4	23 x 14 IN CLASS IV HERCP	222.38'	0.82%	YI-5 INV. 37.97'	YI-2 INV. 36.15'
SD-5	23 x 14 IN CLASS IV HERCP	82.13'	0.43%	YI-2 INV. 36.15'	FES-1 INV. 35.80'
SD-8	30 x 19 IN CLASS V HERCP	71.60'	0.56%	Cul-3 INV. 35.80'	FES-3 INV. 35.40'
SD-9	12 INCH N12 HDPE	38.85'	0.42%	YI-6 INV. 40.16'	FES-4 INV. 40.00'
SD-3	12 INCH N12 HDPE	53.08'	0.47%	YI-3 INV. 39.75'	FES-7 INV. 39.50'
SD-10	15 INCH N12 HDPE	45.62'	2.41%	Cul-2 INV. 36.90'	FES-2 INV. 35.80'
SD-1	23 x 14 IN CLASS IV HERCP	86.77'	0.51%	Cul-1 INV. 39.00'	YI-1 INV. 38.56'



STRUCTURE SCHEDULE SITE DRAINAGE

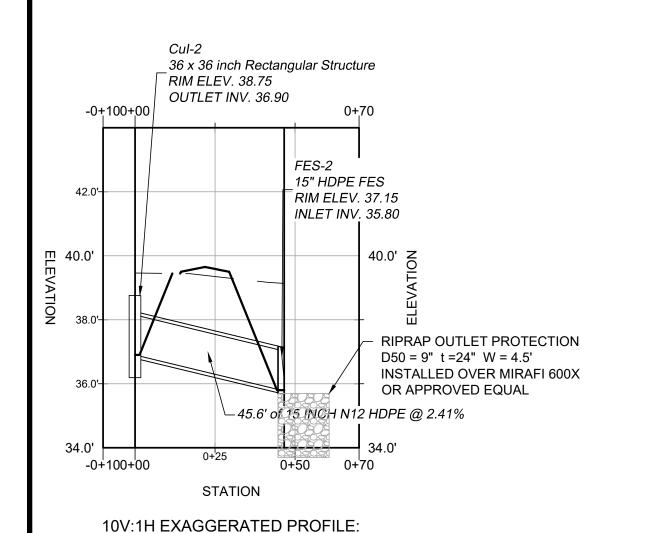
STRUCTURE NAME	STRUCTURE TYPE AND SIZE	CONNECTED PIPES	RIM ELEVATION
Cul-1	48 x 48 inch Rectangular Structure	SD-1 INV OUT = 39.000	40.90
Cul-2	36 x 36 inch Rectangular Structure	SD-10 INV OUT = 36.900	38.75
Cul-3	48 x 48 inch Rectangular Structure	SD-8 INV OUT = 35.800	38.20
FES-1	14" x 23" FES	SD-5 INV IN = 35.800	37.20
FES-2	15" HDPE FES	SD-10 INV IN = 35.800	37.15
FES-3	19" x 30" FES	SD-8 INV IN = 35.400	37.25
FES-4	12" HDPE FES	SD-9 INV IN = 40.000	41.11
FES-6	14" x 23" FES	SD-2 INV IN = 38.100	39.50
FES-7	12" HDPE FES	SD-3 INV IN = 39.500	40.61
YI-1	48 x 48 DEL DOT DRAINAGE INLET	SD-1 INV IN = 38.561 SD-2 INV OUT = 38.561	41.49
YI-2	48 x 48 DEL DOT DRAINAGE INLET	SD-4 INV IN = 36.150 SD-5 INV OUT = 36.150	38.78
YI-3	24 x 24 DEL DOT DRAINAGE INLET	SD-3 INV OUT = 39.750	41.61
YI-5	48 x 48 DEL DOT DRAINAGE INLET	SD-4 INV OUT = 37.967	41.00
YI-6	24 x 24 DEL DOT DRAINAGE INLET	SD-9 INV OUT = 40.163	42.02

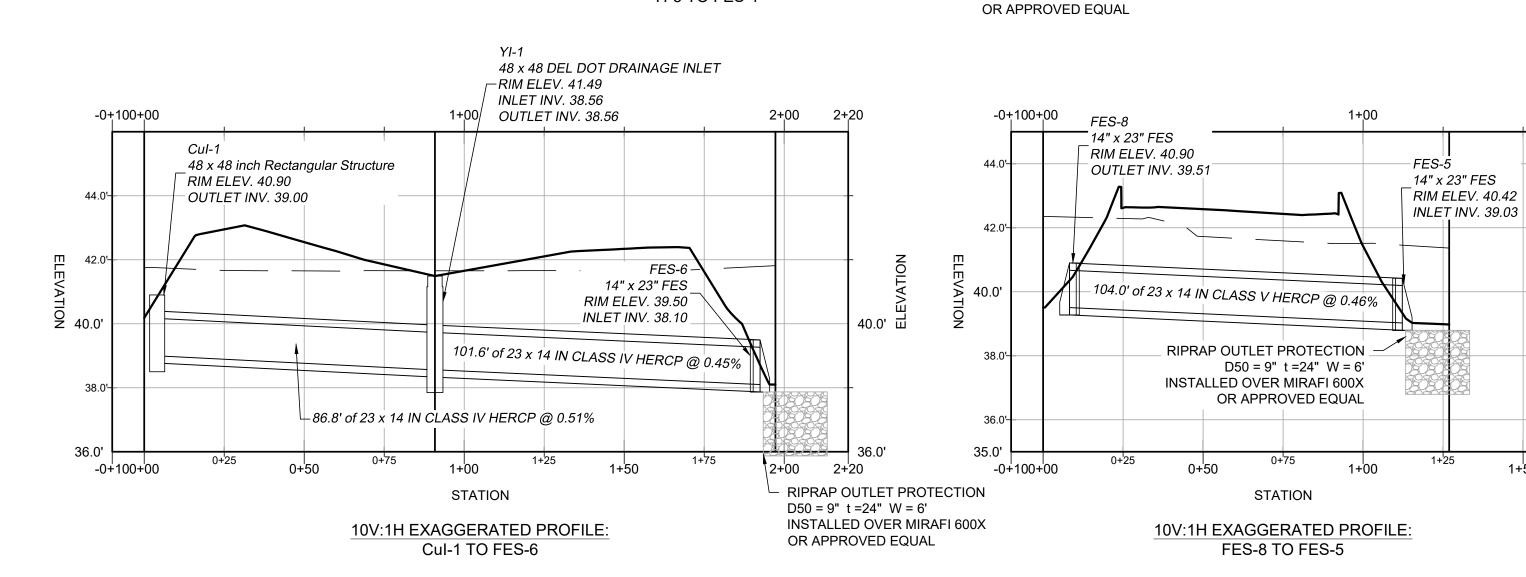
NOTE: FOR CUSTOM OUTLET STRUCTURES AND WEIRS SEE STORMWATER DRAWINGS

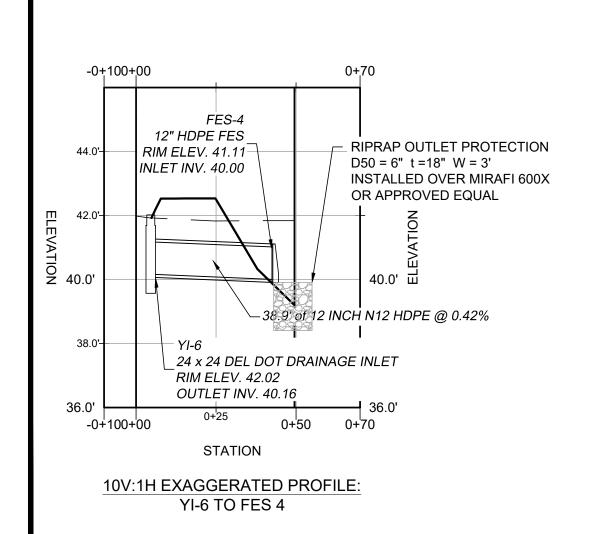
FOR STORMWATER PRACTICE SECTIONS SEE STORMWATER DRAWINGS

40.0' ≤

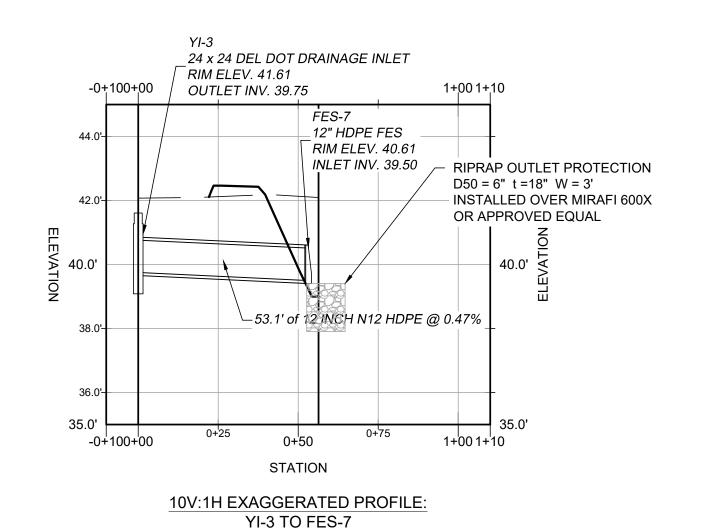
1+501+60

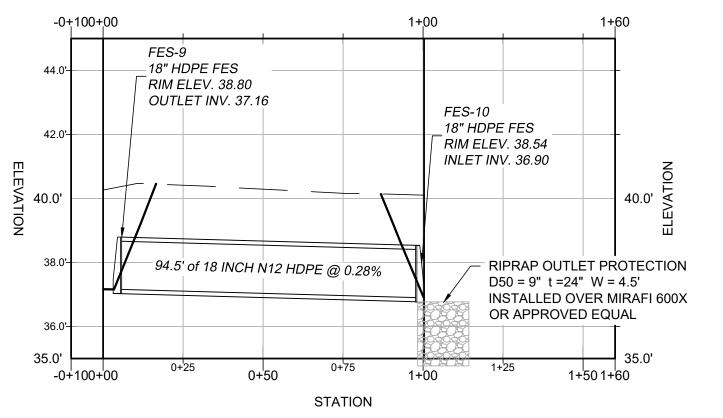






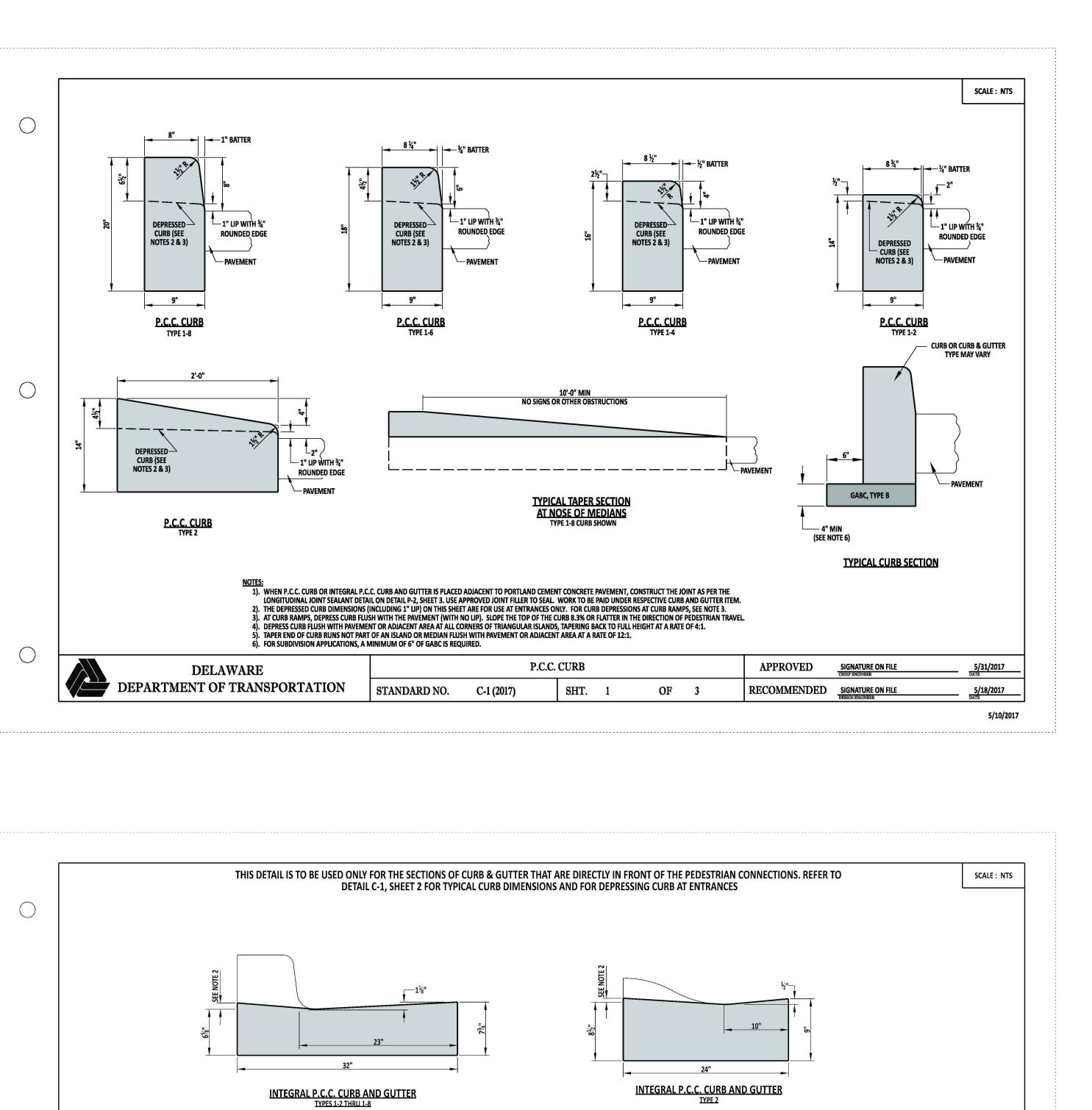
Cul-2 TO FES-2





10V:1H EXAGGERATED PROFILE: FES-9 TO FES-10

DATE: SCALE: AS SHOWN DESIGNED BY: WJR APPROVED BY: SHEET NO.: CB006-C-SW040



1). WHEN P.C.C. CURB OR INTEGRAL P.C.C. CURB AND GUTTER IS PLACED ADJACENT TO PORTLAND CEMENT CONCRETE PAVEMENT, CONSTRUCT THE JOINT AS PER THE LONGITUDINAL JOINT SEALANT DETAIL ON DETAIL P-2, SHEET 3. USE APPROVED

JOINT FILLER TO SEAL. WORK TO BE PAID UNDER RESPECTIVE CURB AND GUTTER ITEM.

2). DEPRESS CURB FLUSH WITH PAVEMENT (WITH NO LIP). SLOPE THE TOP OF THE CURB TO MATCH THE RUNNING SLOPE OF THE ADJACENT PEDESTRIAN CONNECTION. THE MAXIMUM RUNNING SLOPE IS 8.3%. THE MAXIMUM SLOPE OF THE GUTTER PAN AT THE PEDESTRIAN CONNECTION IS 5%.

3). SEE TYPICAL CURB SECTION DETAIL AND NOTE 6 ON DETAIL C-1, SHEET 1 FOR PLACEMENT

OF GABC UNDER CURB AND GUTTER.
4). TRANSITION FROM STANDARD GUTTER SLOPE TO SLOPE SHOWN ON THIS DETAIL OVER

APPROVED

RECOMMENDED SIGNATURE ON FILE

SIGNATURE ON FILE

1/04/2019

1/7/2019

A DISTANCE OF 5'-0".

OF

INTEGRAL P.C.C. CURB & GUTTER

(FOR USE AT PEDESTRIAN CONNECTIONS ONLY)

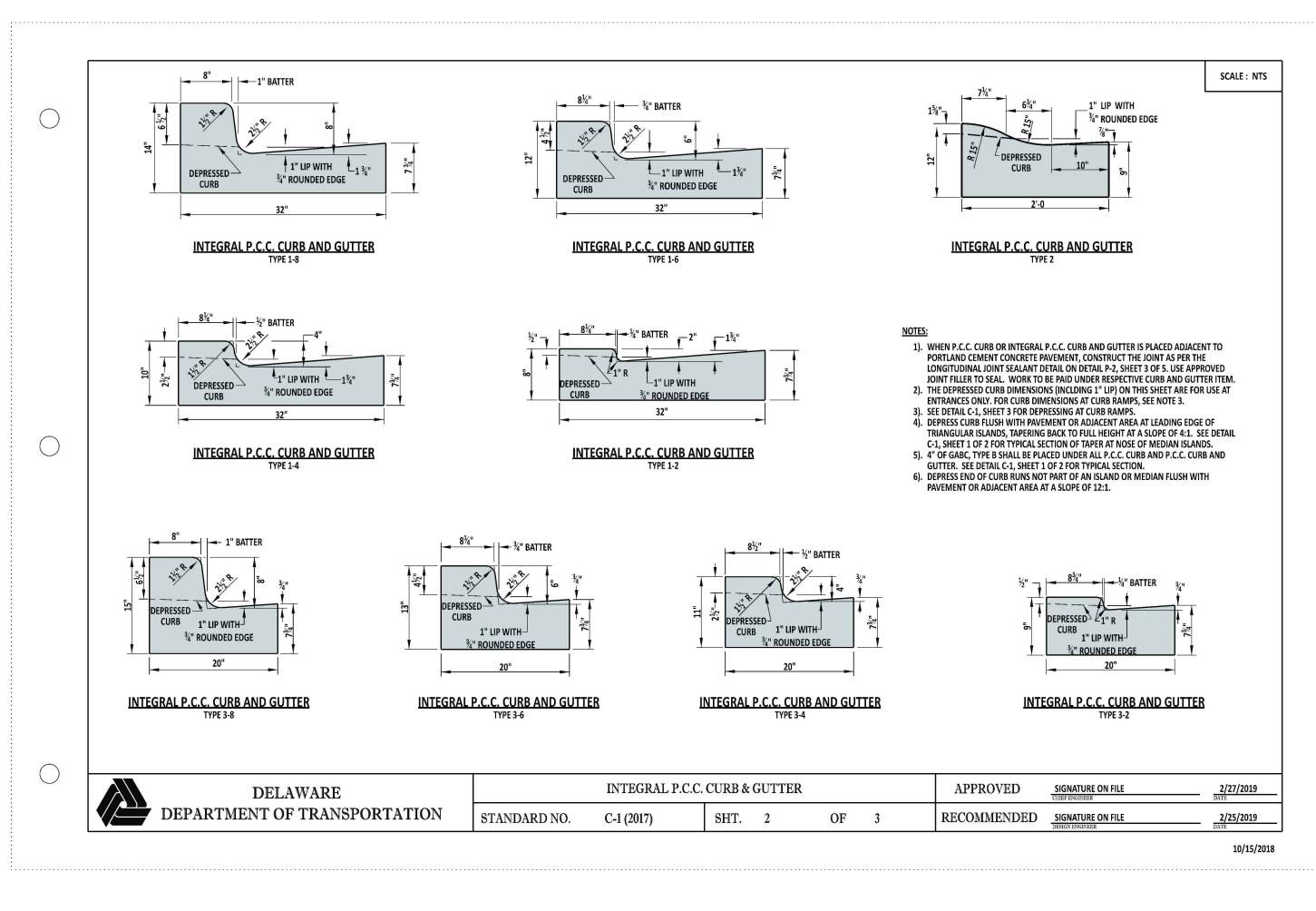
C-1 (2018)

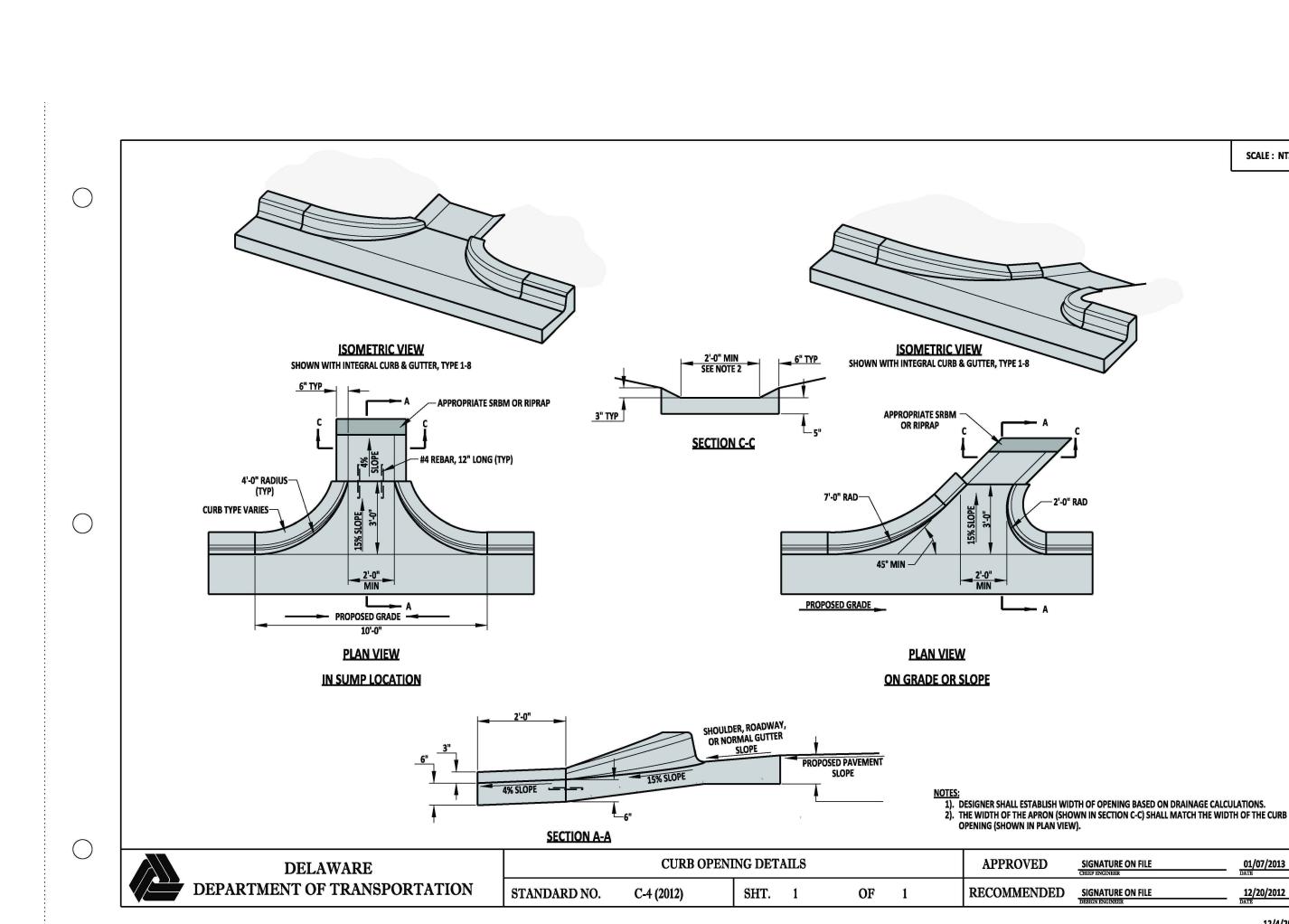
INTEGRAL P.C.C. CURB AND GUTTER

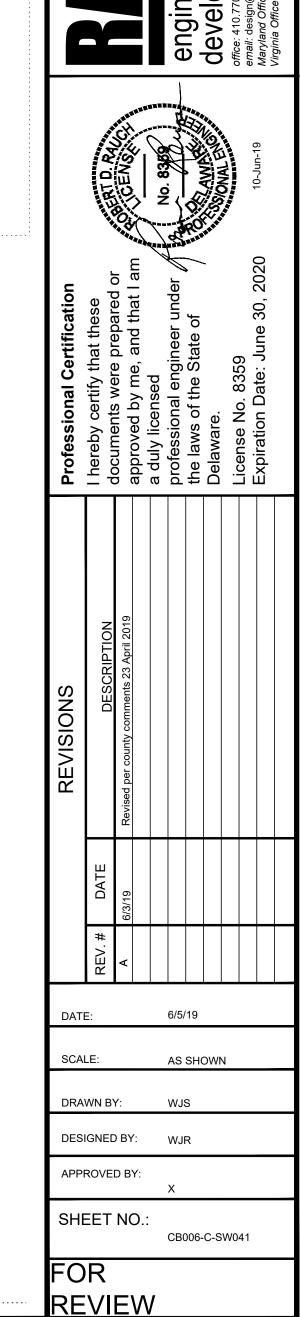
STANDARD NO.

DELAWARE

DEPARTMENT OF TRANSPORTATION

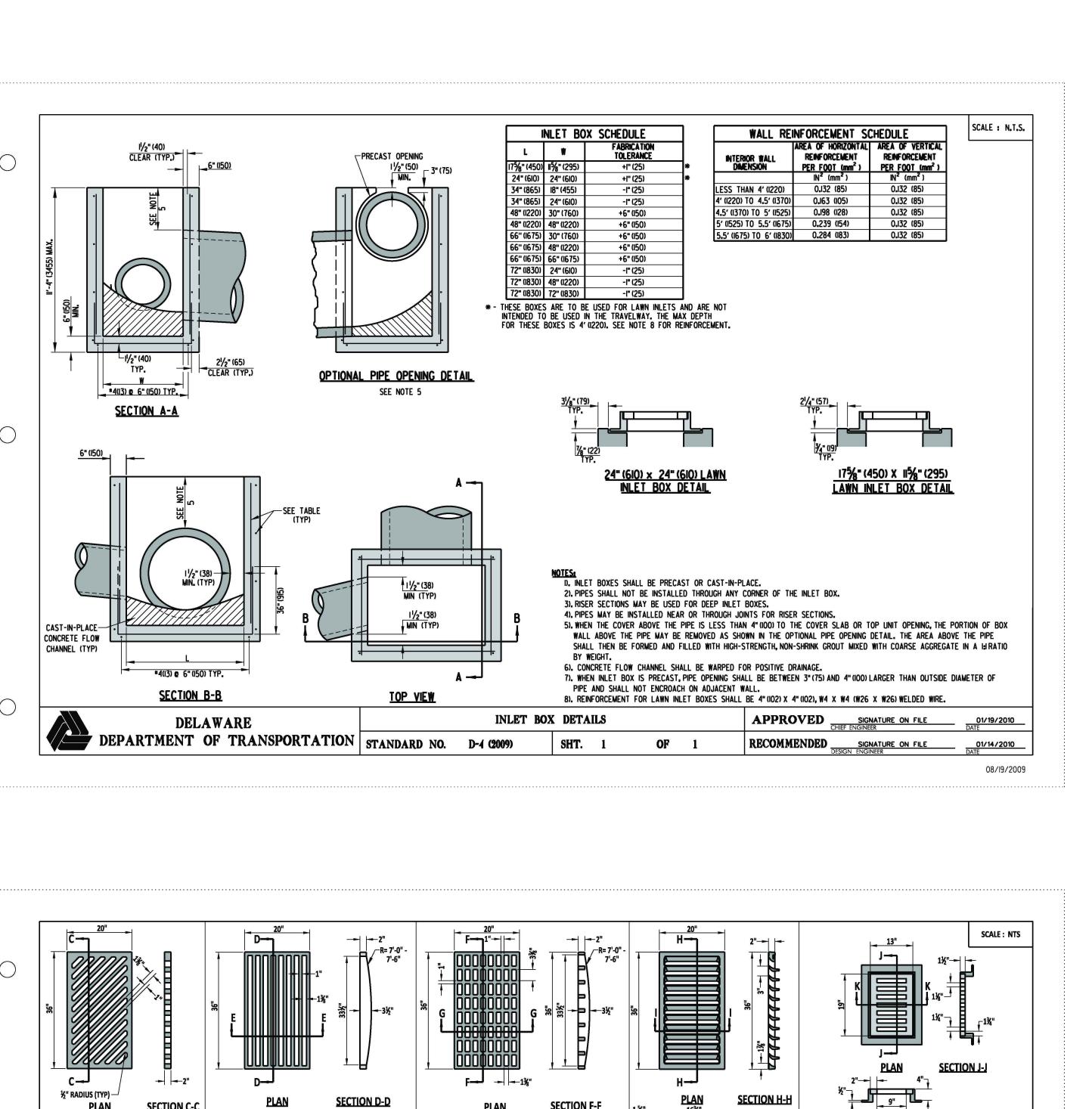


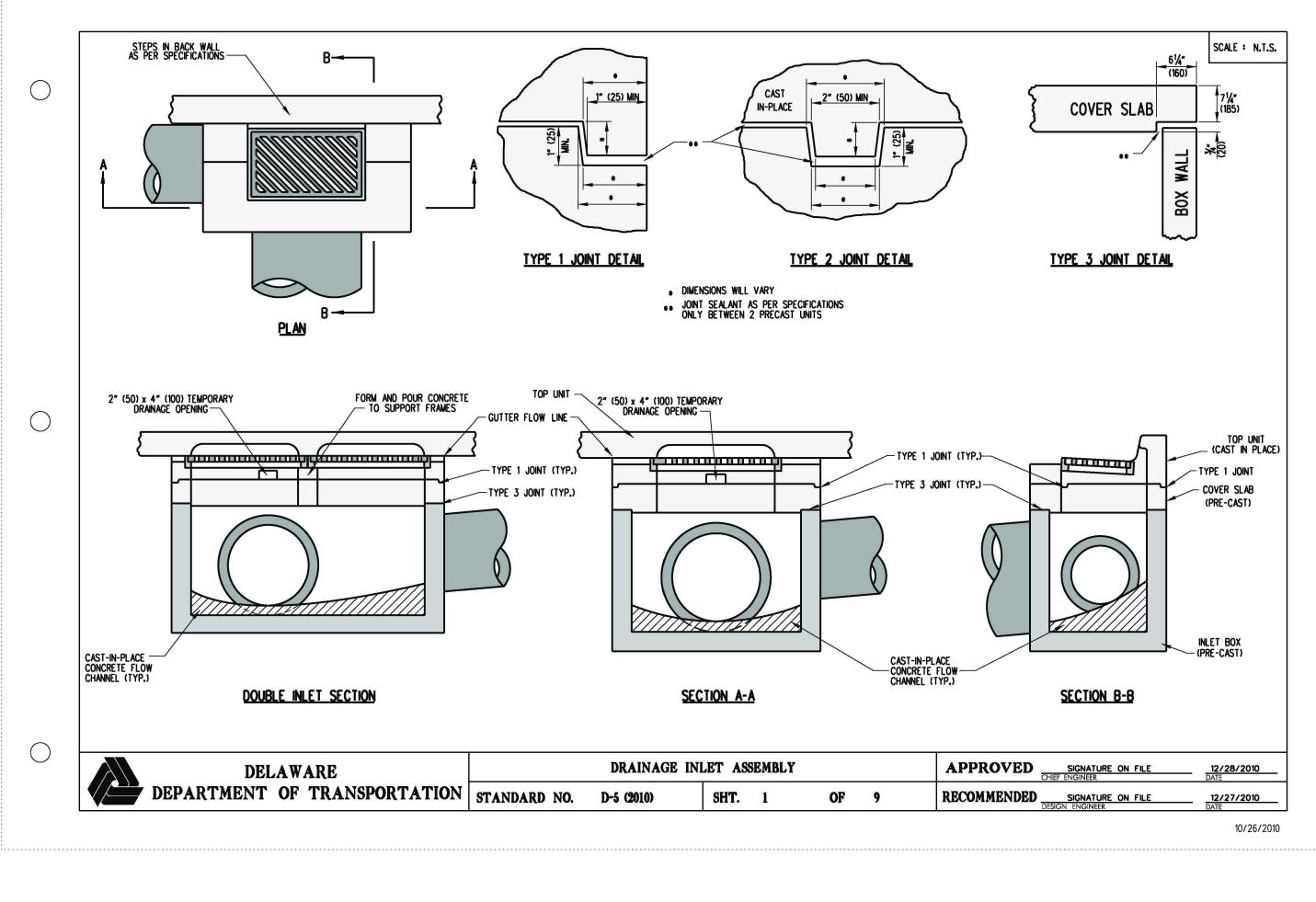


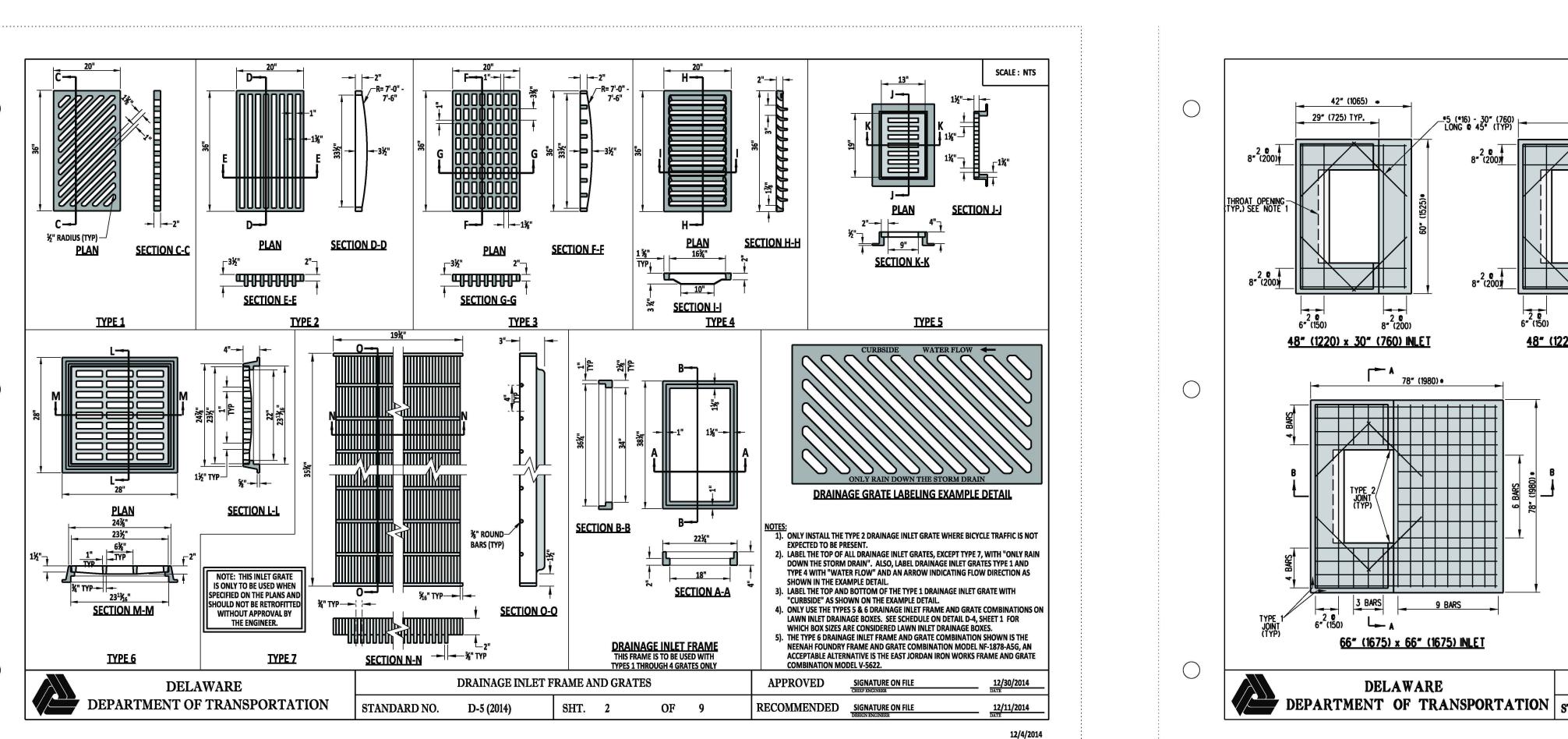


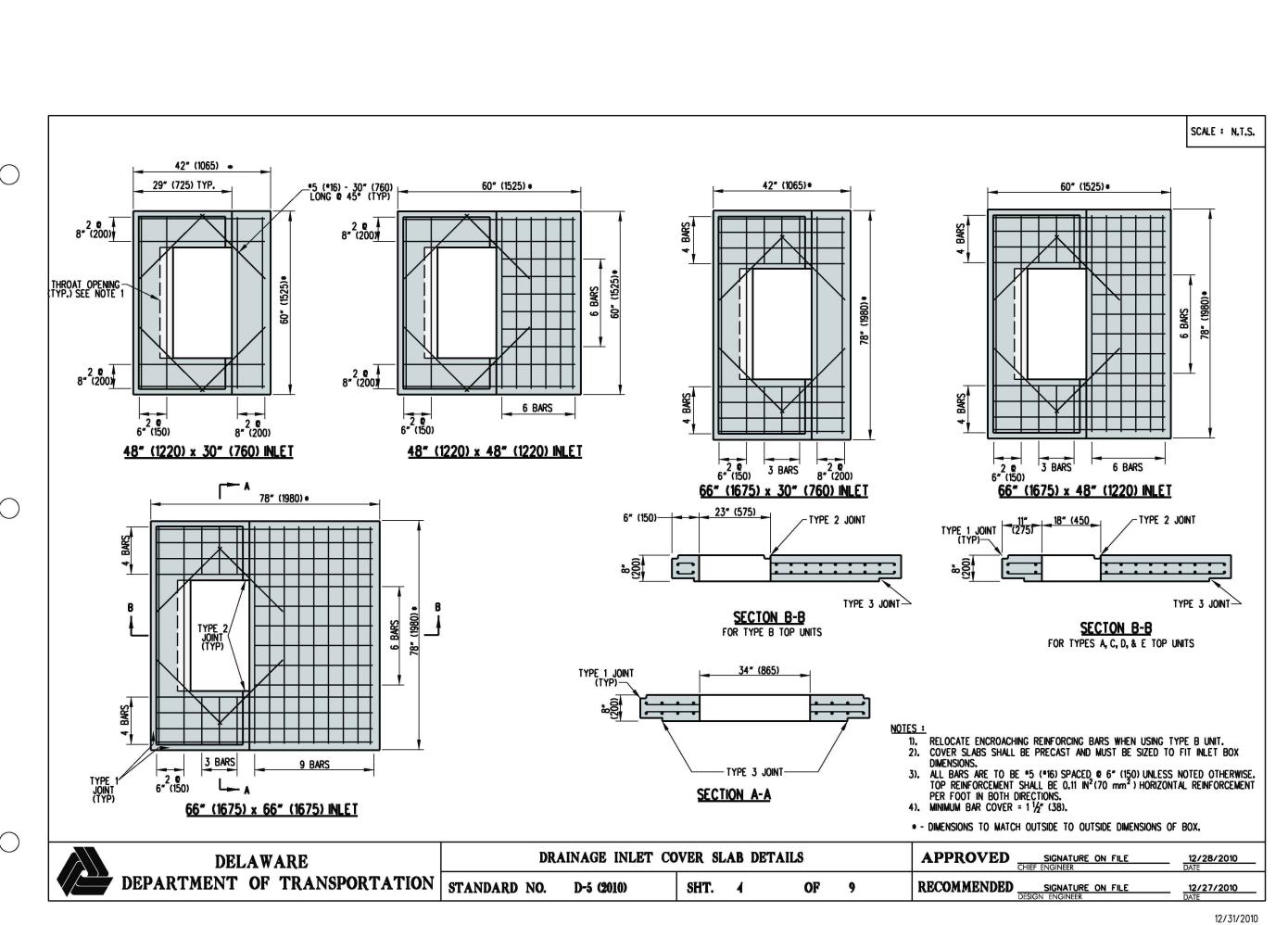
12/4/2012

ANB,











6/5/19

WJS

AS SHOWN

CB006-C-SW042

DATE:

SCALE:

DRAWN BY:

APPROVED BY:

SHEET NO.:

REVIEW

FOR

DESIGNED BY: WJR

